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### 300 ALIGNMENT CHAPTER

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## 330 HORIZONTAL AND VERTICAL ALIGNMENT

### 330.01 CIRCULAR CURVES

BC adopts and concurs with the engineering principles and discussion throughout Section 3.2 of TAC. However, we wish to supplement the Tables provided with MoTI specific procedures and recommended guidelines.

#### Maximum Superelevation

##### Rural Areas: Design Domain

As stated in TAC, 0.06 m/m is the preferred maximum superelevation. The following guidelines indicate the MoTI recommendations in the selection of Design  $e_{max}$  values.

- Rural Ambient Designs Match existing  $e_{max}$
- All other Rural Roads 0.06 m/m

Where a Rural Ambient Project is reconstructing a significant length of highway, the designer should consider using 0.06 m/m as maximum superelevation.

#### Minimum Radius

##### Urban Areas: Design Domain

As per discussion in TAC Section 3.2.2.4

#### Minimum Radius

##### Rural Areas: Design Domain

The following table is provided for Minimum Radii for Rural Design. This is a supplement to TAC Table 3.2.4

**Table 330.A Minimum Radii for Rural Designs**

Design Speed (km/h)	Minimum Radius (m)				
	Normal <sup>2</sup> (-0.02 m/m)	Crown Section		Superelevated Section	
		Reverse <sup>3</sup> (0.02 m/m)		Maximum Rate <sup>1</sup>	
		$e_{max} + 0.06$	$e_{max} + 0.08$	+0.06 m/m	+0.08 m/m
40	700	475	525	55	50
50	1100	745	820	90	80
60	1600	1080	1190	130	120
70	2150	1470	1615	190	170
80	2800	1950	2120	250	230
90	3550	2470	2700	340	300
100	4380	3070	3350	440	390
110	5300	3780	4100	600	530
120	6300	4535	4920	750	670

Notes		
1.	On downgrades in excess of 3%, the minimum horizontal radius should be increased. The method to calculate the increase is described on the following page.	
2.	To determine the minimum radius for normal crown, the (e+f) value is set at 0.018* in TAC Eqn 3.2.1: $e+f = V^2/127R$ . (*Referenced from 1994 BC MoTH <i>Highway Engineering Design Manual</i> which stated "Both TAC and Ontario have selected this value ...")	
3.	The minimum radius reverse crown is solved by re-arranging the basic equation for superelevation (TAC Eqn. 3.2.1) and solving for R when $e = +0.02$ . Superelevation distribution is non-linear, resulting in different minimum radius values at Reverse Crown for $e_{max}$ 0.06 and $e_{max}$ 0.08. The method of distributing "e" and "f" is described in more detail on the following page.	
4.	All values are based on Max. Lateral Friction values from TAC Table 3.2.1	

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**Minimum Radius on Downgrades**

The minimum horizontal radius should be increased on steep downgrades to enhance road safety.

The minimum curve radius should be increased by 10% for each 1% increase in grade over 3%.

$$R_{g (min)} = R_{(min)} ( 1+ (G-3)/10)$$

Where:

$R_{(min)}$  = minimum radii from Table 330.A

G = grade (%)

$R_{g (min)}$  = minimum radius on grade (m)

Example: Design Speed = 100 km/h; e=0.06; G=6%  
 $R_{(min)}$  = 440 m from Table 330.A.

$$R_{g (min)} = 440(1+(6-3)/10) = 572 \text{ m or } 570 \text{ m (rounded)}$$

[Note: Rounding should be to the nearest 10 m increment.]

The applied superelevation rate shall be selected from the appropriate table of superelevations (**Table 330.D or 330.E**) for the adjusted value of  $R_{g (min)}$ .

**Rural and High Speed Urban Design – Superelevation Distribution:**

The general formula for the relationship of speed, radius, superelevation and friction is given by TAC Equation 3.2.1 as:  $e+f = V^2/127R$

For rural and high speed urban roadways the method used for distributing e and f is referred to as “Method 5” in the AASHTO publication. The formula for calculating e is as follows:

For any radius R :

$$e = \frac{V^2}{127R + V^2} \left[ \frac{1}{e_{max}} - \frac{1}{(e+f)_{max}} \right] \quad \text{330.01.01}$$

Where  $e_{max}$  is 0.06 or 0.08 and  $(e+f)_{max}$  is  $e_{max}$  plus  $f_{max}$  which is taken from TAC Table 3.2.1

For clarity, let’s call the bracketed part of the denominator ‘z’

$$e = \frac{V^2}{127R + V^2z} \quad \text{330.01.02}$$

The ‘z’ value is a function of design speed and maximum superelevation. It is a constant for each design speed and maximum superelevation as shown in the following table.

The designer can now calculate the superelevation for any radius that may be desired.

**Table 330.B Superelevation Calculation Factors**

Speed (km/h)	“z” for Max. Super of:	
	0.06 m/m	0.08 m/m
40	12.319	8.500
50	12.121	8.333
60	11.905	8.152
70	11.905	8.152
80	11.667	7.955
90	11.404	7.738
100	11.111	7.500
110	10.417	6.944
120	10.000	6.618
130	9.524	6.250

The resultant friction f is solved as  $V^2/127R$  minus the solved e from **Equation 330.01.02**. The friction can be used as the entry point into the Barrier Warrant Index Nomograph, **Figure 610.A**.

TAC Tables 3.2.6 and 3.2.7 have an insufficient number of design radii that are often necessary to deal with the challenges of horizontal alignment in British Columbia.

In order to facilitate design, MoTI has developed superelevation tables that cover a greater number of design radii. On the following pages are two tables: **Table 330.D** for  $e_{max} = 0.06$  m/m and **Table 330.E** for  $e_{max} = 0.08$  m/m.

These tables also indicate design values for Spiral Lengths (see Section 330.02) and Tangent Runout.

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### Maximum Superelevation for Auxiliary Truck Climbing Lanes

For auxiliary truck climbing lanes, the designer should use the value obtained from the appropriate superelevation table (**Table 330.D** or **330.E** as applicable) or the value from **Table 330.C**, whichever is lower.

**Table 330.C  $e_{\max}$  on Auxiliary Truck Climbing Lanes**

Gradient	$e_{\max}$
4% <sup>(*)</sup>	0.070
5% <sup>(*)</sup>	0.065
6%	0.055
7%	0.045
8%	0.040
9%	0.040
10%	0.040
11%	0.040
12%	0.040

(\*) These values are used only when **Table 330.E** is applied.

#### Notes:

- 1)  $e_{\max}$  in this table should also be applied to the auxiliary slow moving vehicle pullout and the outside auxiliary passing lane, in the uphill direction only.
- 2) For adjacent through lanes, the designer should use the values obtained from the applicable superelevation table (**Table 330.D** or **330.E**).





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### 330.02 SPIRAL CURVES

BC uses **Spiral Length** as opposed to Spiral Parameter; as shown on **Table 330.D** and **Table 330.E**. The lengths are based upon the same rationale as used in TAC; the formulae are converted below to express the Spiral as a Spiral Length “Ls” rather than a Spiral Parameter “A”.

For **Comfort**:  $L_s = \frac{V^3}{28R}$  **330.02.01**

For **Superelevation**  $L_s = \frac{100we}{2s}$  **330.02.02**

For **Aesthetics**:  $L_s = \frac{V}{1.8}$  **330.02.03**

#### Segmental Spirals

It is preferable to use a connecting or segmental spiral between two curves of different radii and it is mandatory when the radius of the flatter curve is more than 50% greater than the radius of the sharper curve.

There are two distinct cases where a segmental spiral would be used. First, where the spiral is needed to adjust the superelevation between the two curves. The second case is where the segmental spiral is used for a speed-change facility, as between a highway curve and an interchange loop.

#### Case 1

80 km/h;  $e_{max} = 0.08$ ;  $R_1 = 600$  m;  $R_2 = 230$  m. What is the La Length? From Table 330.E, Min Ls for R 230 = 80 m

$$\text{Min Segmental } La = L_s * \frac{R_1 - R_2}{R_1} = 49.333 \text{ m}$$

Use La = 50 m

Whenever a solved La is rounded, the Ls generated by the La needs to be determined for detailed calculations of the segmental spiral data.

$$\text{Resultant } L_s = La * \frac{R_1}{R_1 - R_2} = 81.081 \text{ m}$$

#### Case 2:

135 m of Segmental spiral is needed to decelerate from a highway curve of  $R_1$  250 m at 70 km/h to an interchange loop of  $R_2$  50 m at 40 km/h. What is the length of the total spiral?

$$L_s = La * \frac{R_1}{R_1 - R_2} = 135 * \frac{250}{250 - 50} = 168.750 \text{ m}$$



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### 330.03 CREST VERTICAL CURVE

(Ref. BC MoTH Technical Bulletin DS96004)

The design speed shall be used to determine the minimum design rate of vertical curvature (K).

Taillight height shall be used for all roads other than Low-Volume Roads. The additional 1.0 second perception reaction time is NOT required for

taillight height designs. This represents the minimum and should be exceeded where possible.

The use of rock as object height is only required for low volume roads; the additional perception reaction time is also required. This represents the minimum and should be exceeded where possible.

**Table 330.F Minimum K Factors to Provide Stopping Sight Distance on Crest Curves**

Design Speed (km/h)	Minimum SSD (m)		Minimum K for Crest Curve	
	Rock (150 mm)	Taillight (600 mm)**	Rock	Taillight
40	50	50	7	4
50	65	65	11	7
60	85	85	18	11
70	105	105	28	17
80	130	130	42	26
90	185*	160	85*	39
100	215*	185	114*	52
110	250*	220	154*	74
120	285*	250	200*	95

\* Represents 1 second of additional perception/reaction time (based on interpretation of 1976 RTAC *Geometric Design Guide for Canadian Roads and Streets*, Section B.2.5).

\*\* SSD based on 2017 TAC Table 3.3.2

There is no maximum K value for open shoulder designs with unimpeded flows into the ditch. For curbed designs, K values greater than 50 may have poor drainage near the flat points (grade less than 0.3%).

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### 330.04 VERTICAL AND HORIZONTAL ALIGNMENTS NEAR AND ON BRIDGES

#### Alignment Constraints

While it may be aesthetically pleasing to place a bridge on a reversing curve with spirals, this often will introduce logistical complexities in the design and construction of the structure. The introduction of a superelevation transition such as tangent runout can add substantially to the design calculations and construction efforts resulting in a higher the final cost for bridges.

Bridges over fish-bearing streams often have special drainage requirements. In cases where the grade is insufficient to carry water across a bridge or at the bottom of a sag curve, water will pond unless special and very costly drainage works are constructed on both sides of the bridge deck to meet environmental regulations. Many jurisdictions have established grade requirements for bridges to minimize the risk of water accumulation on the deck.

In selecting the roadway's horizontal and vertical alignments near and at bridge crossings, the highway designer should take into account the above constraints on the design of the structure.

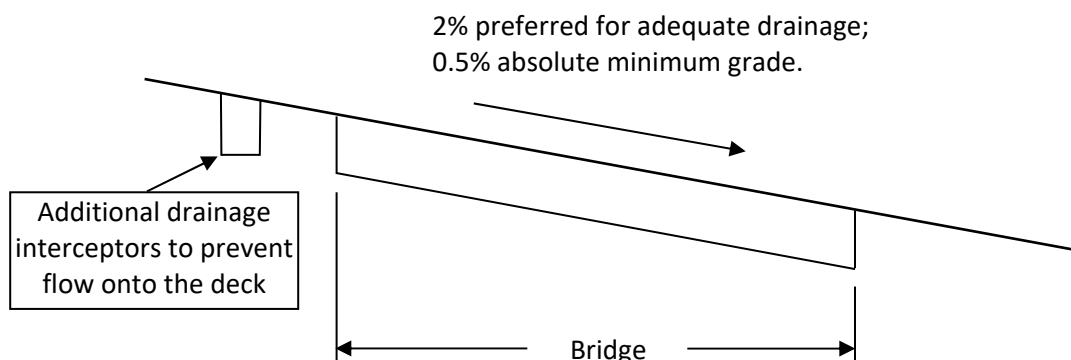
#### Recommendations

The Bridge Engineering Section and the Geometric Standards and Design Section have developed the following guidelines for use by Highway Design Staff and Consultants.

- Bridge Section and/or the Bridge Design Consultant should be part of the preliminary design process to address the following concerns and to balance needs of both the grading and structural design;
- Desirable Grade on Bridges is 2%. Absolute Minimum Grade is 0.5% based on extreme topographical hardship;
- Avoid bridges in the bottom of Sag Vertical Curves;
- Because of our winter conditions and the ease with which bridge decks can freeze, additional drainage pickups should be standard for the downgrade (upstream) approach to bridges;
- Bridges should be located on tangent and outside of tangent runout of the nearest curve or located completely within the circular curve portion.

#### Reference

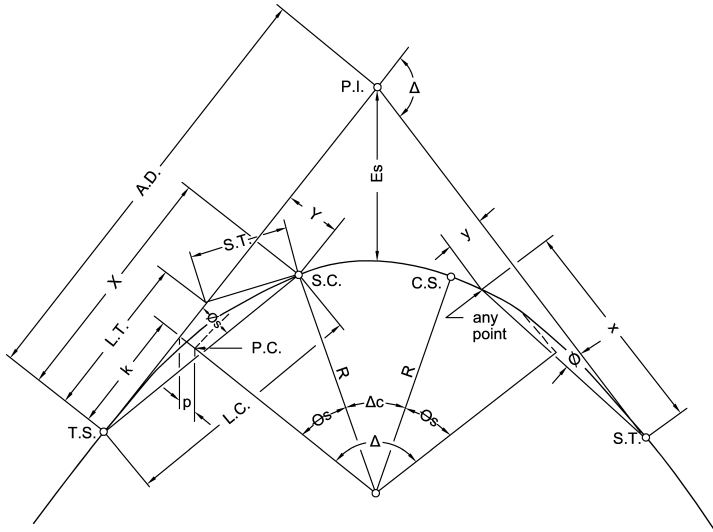
*Rural Road Design, A Guide to the Geometric Design of Rural Roads*, AUSTRROADS, Sydney 2003.



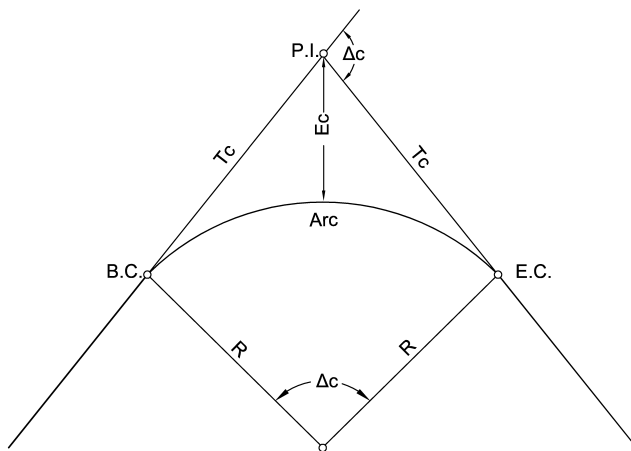
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# 340 GEOMETRIC DESIGN AIDS

Figure 340.A Spiral and Circular Curve Nomenclature



**CURVE WITH TRANSITION BOTH ENDS**  
**Figure (a)**



**CIRCULAR CURVE**  
**Figure (b)**

- P.I. Point of intersection of the main tangents
- T.S. Tangent to Spiral: common point of tangent and spiral - beginning of spiral
- S.C. Spiral to Curve: common point of spiral and circular curve - beginning of circular curve
- C.S. Curve to Spiral: common point of circular curve and spiral - end of circular curve
- S.T. Spiral to Tangent: common point of spiral and tangent - end of spiral
- S.C.S. Mid-point of a curve which is transitional throughout
- R Radius of the circular curve
- r Radius of a curve at any length on the spiral
- Ls Length of spiral between T.S. and S.C.
- ℓ Length between any two points on the spiral
- A.D. Tangent distance P.I. to T.S. or S.T.; apex distance
- Es External distance from P.I. to centre of circular curve portion or to S.C.S. of a curve transitional throughout
- Arc Length of circular curve from S.C. to C.S.
- Δ Intersection angle between the tangents of the entire curve
- Δc Intersection angle between tangents at the S.C. and at the C.S. or the central angle of a circular curve
- Θs Spiral Angle: The intersection angle between the tangent of the complete curve and the tangent at the S.C.
- Θ Intersection angle between tangent of complete curve and tangent at any other point on the spiral
- Θs Deflection angle from tangent at T.S. to S.C.
- Ø Deflection angle from tangent at any point on spiral to any other point on spiral
- L.T. Long tangent distance of spiral only
- S.T. Short tangent distance of spiral only
- L.C. Long chord of the spiral curve; distance from T.S. to S.C.
- P Offset distance from the tangent of P.C. of circular curve produced
- k Distance from T.S. to point on tangent opposite the P.C. of the circular curve produced
- X,Y Coordinates of S.C. from T.S.
- x,y Coordinates of any other point on spiral from the T.S.
- Tc Tangent distance P.I. to B.C. or E.C.
- B.C. Beginning of curve
- E.C. End of curve
- Arc Length of curve from B.C. to E.C.
- Δc Intersection angle between the tangents
- Ec External distance from P.I. to centre of curve

} Circular Curve only

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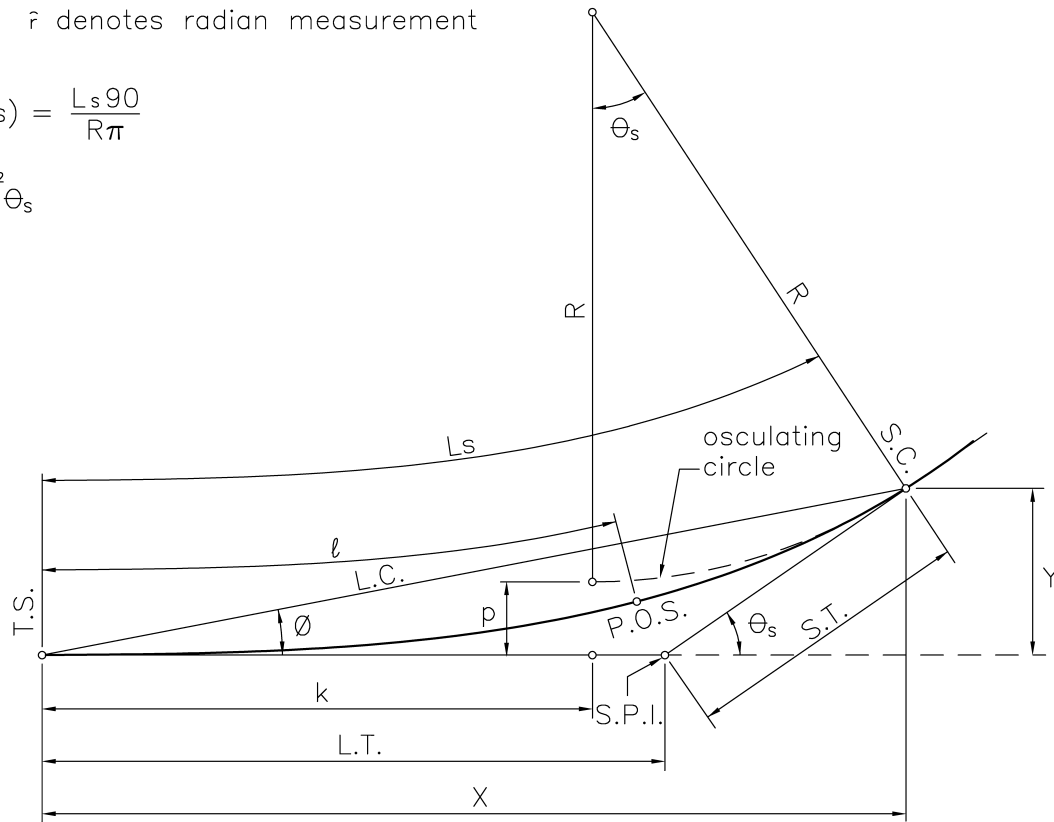
Figure 340.B Spiral Formulae

$$\theta_{sr} = \frac{L_s}{2R} \quad \hat{r} \text{ denotes radian measurement}$$

$$\theta_s \text{ (degrees)} = \frac{L_s 90}{R\pi}$$

$$\theta_l = \left(\frac{l}{L_s}\right)^2 \theta_s$$

$$r = \frac{L_s R}{l}$$



$$X = L_s \left( 1 - \frac{\theta_{sr}^2}{5 \times 2!} + \frac{\theta_{sr}^4}{9 \times 4!} - \frac{\theta_{sr}^6}{13 \times 6!} + \dots \right)$$

For any point on the spiral substitute:

①  $l$  for  $L_s$

②  $\theta_{lr}$  for  $\theta_{sr}$

$$Y = L_s \left( \frac{\theta_{sr}}{3 \times 1!} - \frac{\theta_{sr}^3}{7 \times 3!} + \frac{\theta_{sr}^5}{11 \times 5!} - \frac{\theta_{sr}^7}{15 \times 7!} + \dots \right)$$

$$k = \frac{L_s}{2} \left( 1 - \frac{\theta_{sr}^2}{5 \times 3!} + \frac{\theta_{sr}^4}{9 \times 5!} - \frac{\theta_{sr}^6}{13 \times 7!} + \dots \right) = X - R \sin \theta_s$$

$$L.T. = X - \frac{Y}{\tan \theta_s}$$

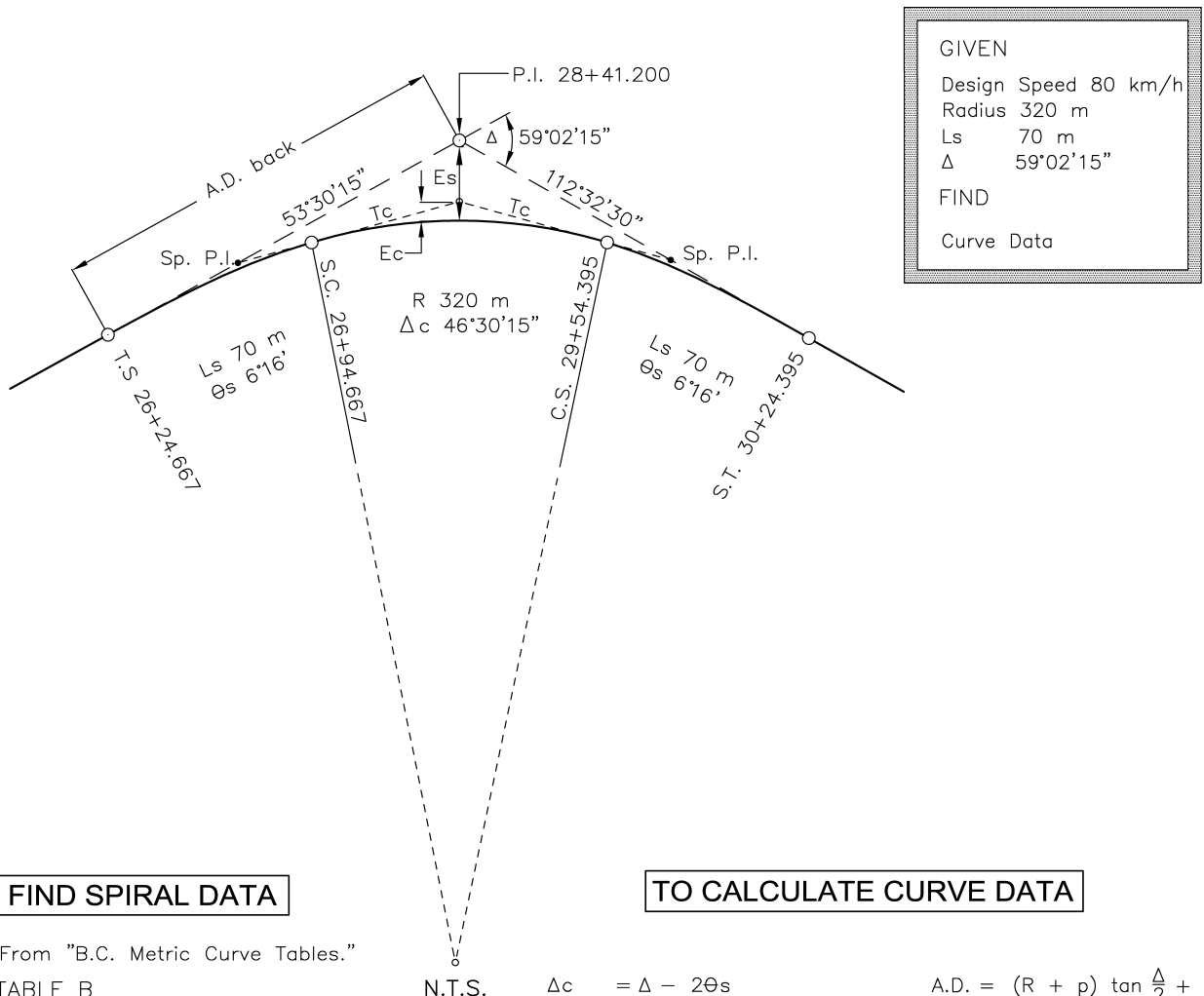
$$p = \frac{L_s}{2} \left( \frac{\theta_{sr}}{3 \times 2!} - \frac{\theta_{sr}^3}{7 \times 4!} + \frac{\theta_{sr}^5}{11 \times 6!} - \frac{\theta_{sr}^7}{15 \times 8!} + \dots \right) = Y - R + R \cos \theta_s$$

$$S.T. = \frac{Y}{\sin \theta_s}$$

$$L.C. = \sqrt{X_s^2 + Y_s^2} \quad \theta_s = \arctan \left( \frac{Y_s}{X_s} \right) = \frac{\theta_s}{3} - C_s \quad \text{where } C_s \text{ (in seconds)} = 0.0031 \theta_s^3$$

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Figure 340.C Circular Curve with Equal Spirals



GIVEN	
Design Speed	80 km/h
Radius	320 m
Ls	70 m
Δ	59°02'15"
FIND	
Curve Data	

**TO FIND SPIRAL DATA**

1. From "B.C. Metric Curve Tables."  
TABLE B

Ls 70 m	X = 69.916
R 320 m	Y = 2.550
	k = 34.986
	p = 0.638
	L.T. = 46.696
Theta (Θs) = 6°16'	S.T. = 23.360
Phi (Øs) = 2°05'19"	L.C. = 69.963

$$\text{Theta } (\Theta_s) = \frac{L_s}{2R} \left( \frac{180}{\pi} \right)$$

OR 
$$(\Theta_s) = \frac{L_s}{R} \left( \frac{90}{\pi} \right)$$

$$= \frac{70}{320} (28.6479) = 6.26673^\circ$$

$$\Theta_s = 6'16''$$

**TO CALCULATE CURVE DATA**

N.T.S.

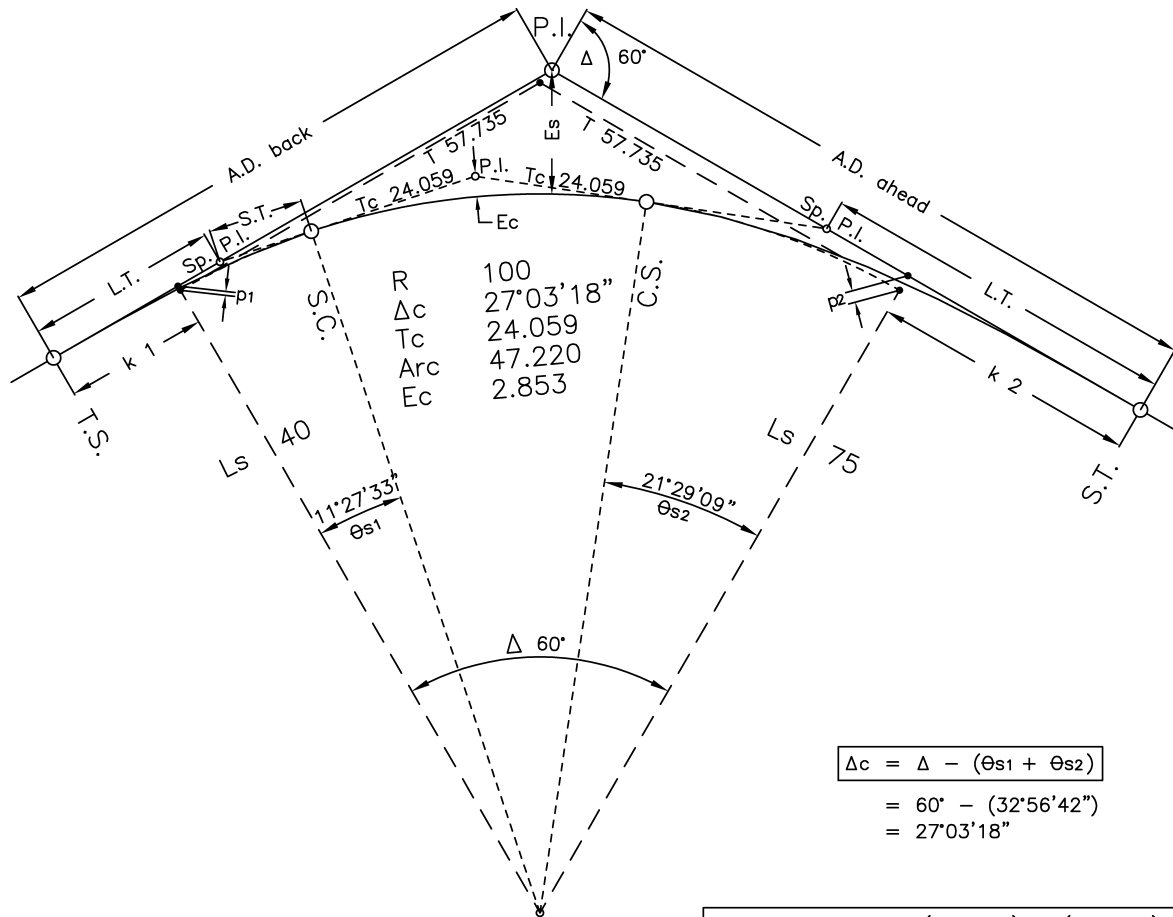
$$\begin{aligned} \Delta_c &= \Delta - 2\Theta_s \\ &= 59^\circ02'15'' - (2 \times 6'16'') \\ \Delta_c &= 46^\circ30'15'' \\ \text{Arc} &= R \times \text{radian } 46^\circ30'15'' * \\ &= 320 \times 0.8116508 \\ \text{Arc} &= 259.728 \\ T_c &= R \tan \frac{\Delta_c}{2} \\ &= 320 \times 0.429677 \\ T_c &= 137.497 \\ E_c &= \frac{R}{\cos \frac{\Delta_c}{2}} - R \\ &= \frac{320}{0.918777} - 320 \\ E_c &= 28.289 \end{aligned}$$

$$\begin{aligned} \text{A.D.} &= (R + p) \tan \frac{\Delta}{2} + k \\ \text{A.D.} &= 216.533 \\ E_s &= \frac{R + p}{\cos \frac{\Delta}{2}} - R \\ &= \frac{320.638}{0.870195} - 320 \\ E_s &= 48.467 \end{aligned}$$

\* NOTE  
1 degree =  $\frac{\pi}{180}$  radian  
 $\therefore 46^\circ30'15'' = 46.5041667 \left( \frac{\pi}{180} \right)$   
 $= 0.8116508$  radian

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Figure 340.D Circular Curve with Unequal Spirals



R	100
Δc	27°03'18"
Tc	24.059
Arc	47.220
Ec	2.853

$$\Delta_c = \Delta - (\Theta_{s1} + \Theta_{s2})$$

$$= 60^\circ - (32^\circ 56' 42'')$$

$$= 27^\circ 03' 18''$$

GIVEN:	R 100	From B.C. Metric Curve Tables
	Δ 60°	
	Ls1 40	T.5. → L.S. 40, R 100
	Ls2 75	k1 19.973
		p1 0.666
FIND:	A.D. ahd.	L.T. 26.723
	A.D. back	S.T. 13.384
	Curve data	Θs1 11°27'33"

$$\text{A.D. back} = k_1 + \frac{(R + p_2) - (R + p_1) \cos \Delta}{\sin \Delta}$$

$$= 19.973 + \frac{102.332 - 100.666 \cos 60^\circ}{\sin 60^\circ}$$

A.D. back = 80.016

$$\text{A.D. ahead} = k_2 + \frac{(R + p_1) - (R + p_2) \cos \Delta}{\sin \Delta}$$

$$= 37.325 + \frac{100.666 - 102.332 \cos 60^\circ}{\sin 60^\circ}$$

A.D. ahead = 94.483

Ls 75 not given in Tables

$$\Theta_s = \frac{L_s}{R} \left( \frac{90}{\pi} \right) \therefore \Theta_{s2} = \frac{75}{100} 28.6479$$

$$= 21.485917^\circ$$

From Table C (Unit Spiral) = 21°29'09"

- T.71 → Theta 21.486° (interpolate)
- p2 0.031094 x 75 = 2.332
- k2 0.497666 x 75 = 37.325
- L.T. 0.671645 x 75 = 50.373
- S.T. 0.337863 x 75 = 25.340

$$E_s = \sqrt{(\text{A.D. back} - k_1)^2 + (R + p_1)^2} - R$$

$$= \sqrt{(80.016 - 19.973)^2 + (100 + 0.666)^2} - 100$$

Es = 17.213

(Formula valid only if Es is on a Circular Curve)

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Figure 340.E Three Transition Compound Curve – General Layout

EXAMPLE 1

Proceeding from flatter to sharper curve.

- GIVEN  $\Delta I = 42^\circ 03'$   
 $R_1 = 600$   
 $R_2 = 230$   
 $Ls_1 = 50$   
 $Ls_2 = 80$   
 $La = 50$

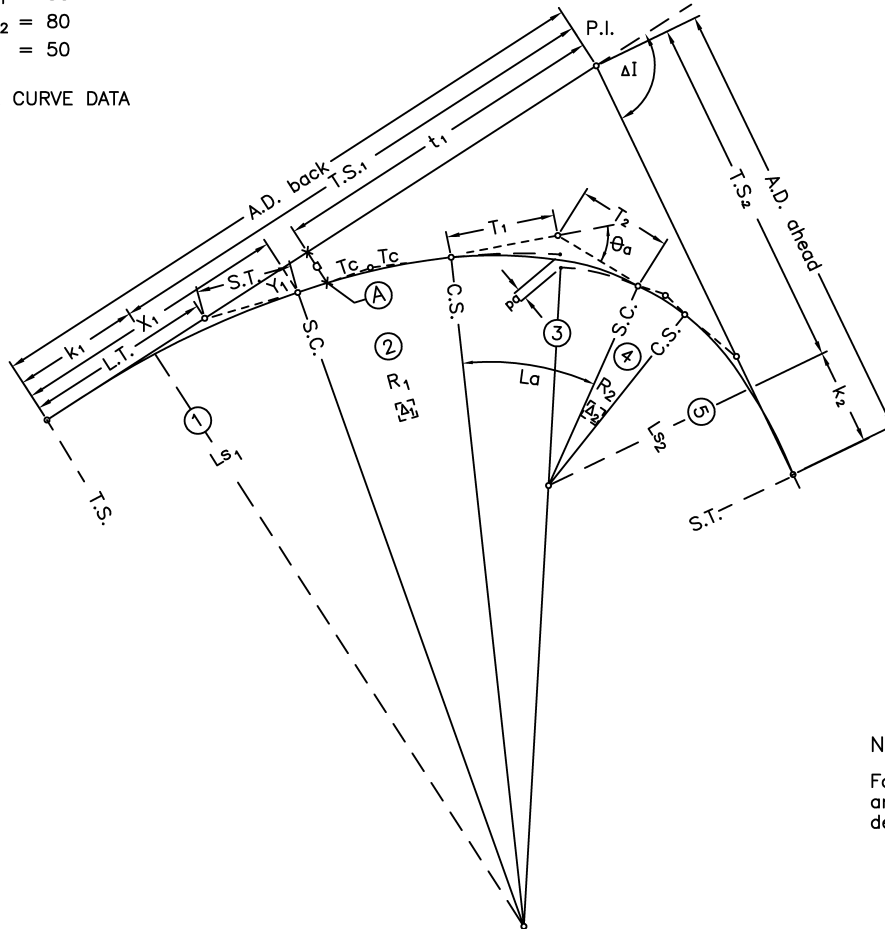
FIND ALL CURVE DATA

CASE 1

$\Delta_1$  and  $\Delta_2$  are not given.

From a preliminary plan showing P.I.,  $\Delta I$  and bearings, it was determined that curve ② (R 600) has to pass through point (A);

$\therefore$  T.S.<sub>1</sub> is fixed.



NOTE:

For graphical reasons, scale and proportions are distorted deliberately.

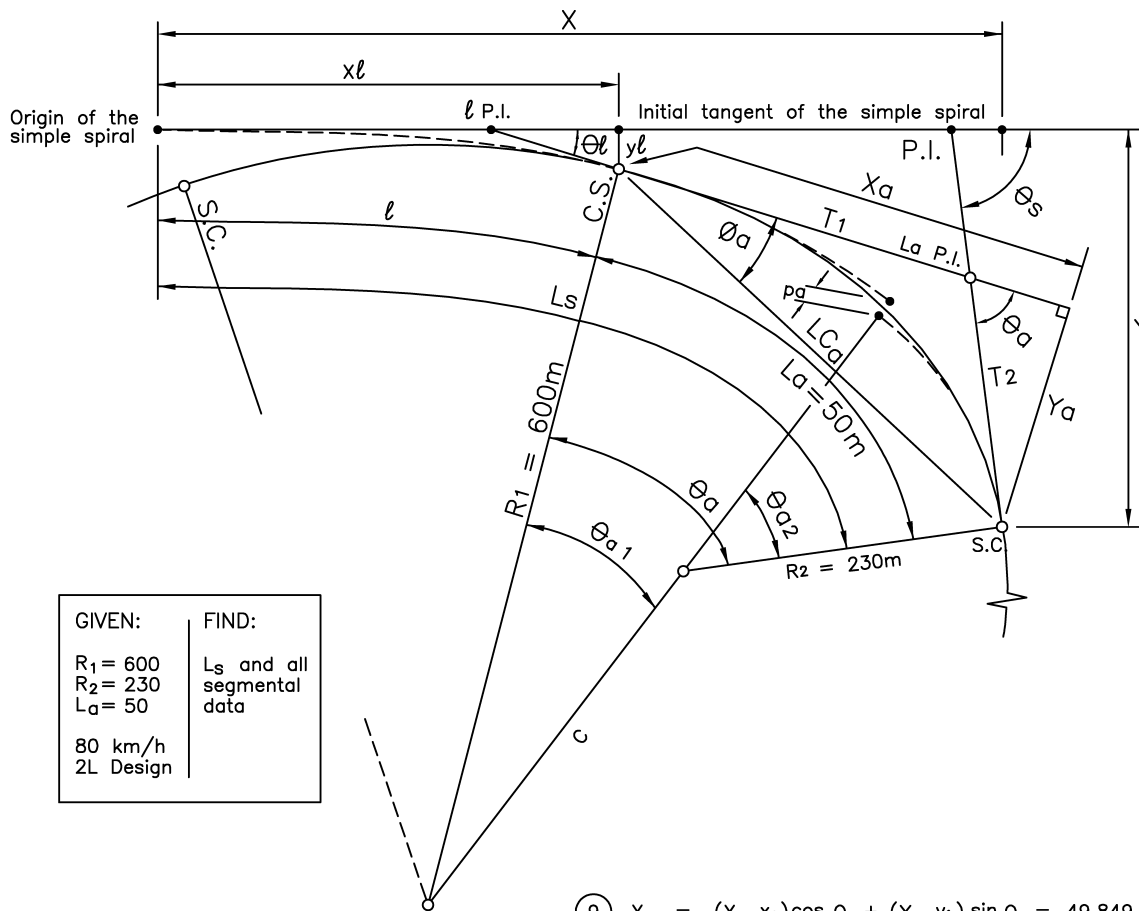
CALCULATED CURVE DATA

1	$Ls_1$ 50	3	$La$ 50	4	$R_2$ 230
X	49.991	$\theta_a$	8° 36' 54"	$\Delta$	12° 01' 26"
Y	0.694	$\theta_{a1}$	2° 23' 12"	$T_c$	24.222
k	24.999	$\theta_{a2}$	6° 13' 42"	Arc	48.267
p	0.174	pa	0.279	Ec	1.272
L.T.	33.336	Xa	49.850	Ch	48.178
S.T.	16.669	Ya	3.195		
L.C.	49.996	T1	28.761	5	$Ls_2$ 80
$\theta_s$	2° 23' 14"	T2	21.329	X	79.758
$\phi$	0° 47' 45"	$\phi_a$	3° 40' 02"	Y	4.628
				k	39.960
2	$R_1$ 600			p	1.158
$\Delta$	9° 03' 34"	P.I. N.	1000.000	L.T.	53.418
$T_c$	47.534	E.	500.000	S.T.	26.744
Arc	94.870	A N.	924.199	L.C.	79.893
Ec	1.880	E.	507.000	$\theta_{s2}$	9° 57' 52"
Ch	94.771			$\phi_2$	3° 19' 14"

- USE B.C. METRIC CURVE TABLES TO FIND SPIRAL DATA FOR  $Ls_1$  AND  $Ls_2$ .
- SOLVE SEGMENTAL SPIRAL –  $La$  50, USING PROCEDURE SHOWN ON FIGURE 340.F
- CALCULATE CURVE DATA, USING PROCEDURE SHOWN ON FIGURE 340.G

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Figure 340.F Three Transition Compound Curve – Segmental Spiral



<b>GIVEN:</b>	<b>FIND:</b>
R <sub>1</sub> = 600	L <sub>s</sub> and all segmental data
R <sub>2</sub> = 230	
L <sub>a</sub> = 50	
80 km/h 2L Design	

- ① L<sub>a</sub> = 50 (see 330.02)
- ② L<sub>s</sub> = L<sub>a</sub> ( (R<sub>1</sub> / (R<sub>1</sub> - R<sub>2</sub>)) ) = 81.081
- ③ θ<sub>s</sub> = (L<sub>s</sub> / R<sub>2</sub>) × (90 / π) = 10.099° = 10°05'57"
- ④ θ<sub>a</sub> = L<sub>a</sub> ( (1 / R<sub>2</sub>) + (1 / R<sub>1</sub>) ) (90 / π) = 8.615° = 8°36'54"
- ⑤ l = L<sub>s</sub> - L<sub>a</sub> = 31.081
- ⑥ θ<sub>l</sub> = (l / R<sub>1</sub>) × (90 / π) = 1.484° = 1°29'02"
- ⑦ From Unit Spiral Tables θ<sub>s</sub> = 10.099° interpolate (or use spiral formulae 340.B)  
X = 80.829      Y = 4.753
- ⑧ From Unit Spiral Tables θ<sub>l</sub> = 1.484° interpolate (or use spiral formulae 340.B)  
x<sub>l</sub> = 31.079      y<sub>l</sub> = 0.268
- ⑨ X<sub>a</sub> = (X - x<sub>l</sub>) cos θ<sub>l</sub> + (Y - y<sub>l</sub>) sin θ<sub>l</sub> = 49.849 5\*
- ⑩ Y<sub>a</sub> = (Y - y<sub>l</sub>) cos θ<sub>l</sub> - (X - x<sub>l</sub>) sin θ<sub>l</sub> = 3.195 1\*
- ⑪ θ<sub>a1</sub> = arctan ( (X<sub>a</sub> - (R<sub>2</sub> sin θ<sub>a</sub>) / (R<sub>1</sub> - [Y<sub>a</sub> + (R<sub>2</sub> cos θ<sub>a</sub>)] ) ) = 2°23'12"
- ⑫ θ<sub>a2</sub> = θ<sub>a</sub> - θ<sub>a1</sub> = 6°13'42"
- ⑬ C = R<sub>1</sub> - R<sub>2</sub> - p<sub>a</sub>  
= √ [X<sub>a</sub> - (R<sub>2</sub> sin θ<sub>a</sub>) ]<sup>2</sup> + {R<sub>1</sub> - [Y<sub>a</sub> + (R<sub>2</sub> cos θ<sub>a</sub>)] }<sup>2</sup>  
= 369.721
- ⑭ T<sub>1</sub> = X<sub>a</sub> - (Y<sub>a</sub> / tan θ<sub>a</sub>) = 28.760
- ⑮ T<sub>2</sub> = (Y<sub>a</sub> / sin θ<sub>a</sub>) = 21.330
- ⑯ φ<sub>a</sub> = arctan ( (Y<sub>a</sub> / X<sub>a</sub>) ) = 3°40'02"
- ⑰ LC<sub>a</sub> = √ (X<sub>a</sub><sup>2</sup> + Y<sub>a</sub><sup>2</sup>) = 49.952

\* For sufficient accuracy of T<sub>1</sub> and T<sub>2</sub>, use at least 4 decimal places

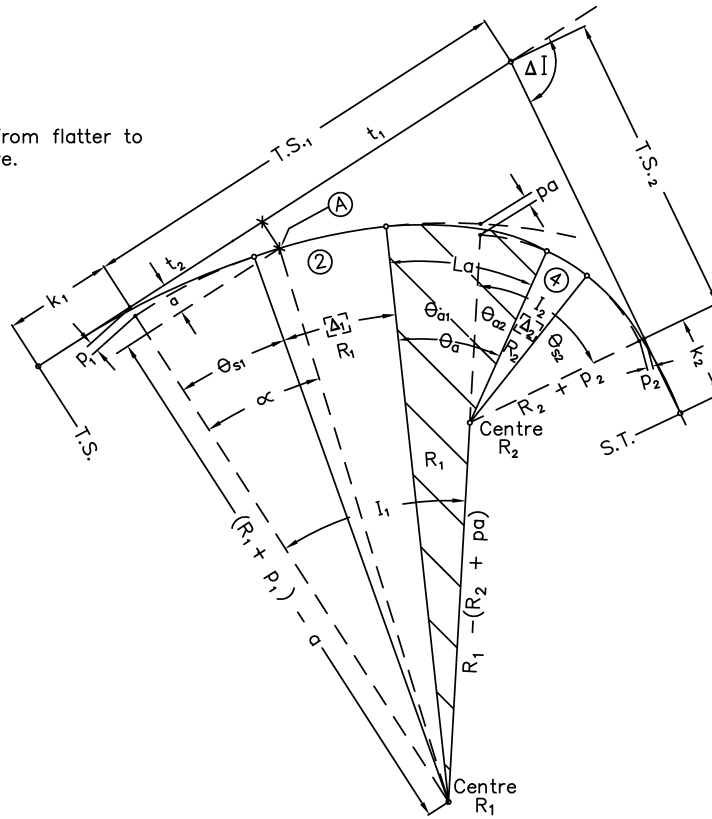


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Figure 340.G Three Transition Compound Curve Calculation

EXAMPLE 1

Proceeding from flatter to sharper curve.



GIVEN  $\Delta I = 42^\circ 03'$   
 $R_1 = 600$   
 $R_2 = 230$   
 $Ls_1 = 50$   
 $Ls_2 = 80$   
 $La = 50$

FIND ALL CURVE DATA

FIGURE 2

CALCULATION PROCEDURE

Solve curve data for  $La$  (see Figure 340.F)

From coordinates of (A) :  $a = 7.014$   $t_1 = 75.801$   
 Centre of  $R_1$  and T.S.<sub>1</sub> become fixed.

$$\cos \alpha = \frac{(R_1 + p_1) - a}{R_1} = \frac{600.174 - 7.014}{600} \therefore \alpha = 8^\circ 39' 35''$$

$$t_2 = R_1 \sin \alpha = 600 \sin 8^\circ 39' 35'' = 90.340$$

$$\begin{aligned} \text{T.S.}_1 &= t_1 \dots\dots 75.801 \\ &+ t_2 \dots\dots 90.340 \\ &\hline &166.141 \end{aligned}$$

$$\begin{aligned} \cos I_2 &= \frac{\text{T.S.}_1 \sin \Delta I - (R_2 + p_2) + (R_1 + p_1) \cos \Delta I}{R_1 - R_2 - pa} \\ &= \frac{166.141 \sin 42^\circ 03' - 231.158 + 600.174 \cos 42^\circ 03'}{369.721} \\ I_2 &= 28^\circ 13' 00'' \end{aligned}$$

$$\begin{aligned} \Delta_2 &= I_2 - (\theta_{a2} + \theta_{s2}) \\ &= 28^\circ 13' 00'' - (6^\circ 13' 42'' + 9^\circ 57' 52'') \\ &= 12^\circ 01' 26'' \end{aligned}$$

$$\begin{aligned} I_1 &= \Delta I - I_2 \\ &= 42^\circ 03' 00'' - 28^\circ 13' 00'' \\ &= 13^\circ 50' 00'' \end{aligned}$$

$$\begin{aligned} \Delta_1 &= I_1 - (\theta_{a1} + \theta_{s1}) \\ &= 13^\circ 50' 00'' - (2^\circ 23' 12'' + 2^\circ 23' 14'') \\ &= 9^\circ 03' 34'' \end{aligned}$$

$$\begin{aligned} \text{T.S.}_2 &= \frac{(R_1 + p_1) - (R_2 + p_2) \cos \Delta I - (R_1 - R_2 - pa) \cos I_1}{\sin \Delta I} \\ &= \frac{600.174 - 231.158 \cos 42^\circ 03' - 369.721 \cos 13^\circ 50'}{\sin 42^\circ 03'} \\ &= 103.807 \end{aligned}$$

$$\text{AD back} = \text{T.S.}_1 + k_1 = 166.141 + 24.999 = 191.140$$

$$\text{AD ahead} = \text{T.S.}_2 + k_2 = 103.807 + 39.960 = 143.767$$

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