

# Appendix A

## Capacity Analyses

## Malahat Capacity Calculations

### Highway Characteristics

Highway Classification		Rural Arterial Divided
Design Speed		80 km/h
Posted Speed		70 km/h (assume ave speed = 75 km/h)
Lane Widths		3.5 m
Distance of Obstructions to Edge of Travelled Way		1 m
Rural or Suburban Environment		rural
Commuter/Regular Users vs Recreation/Non-regular Users		Summer: non-regular
Grades	Goldstream to Tunnel Hill	6 km between 0% and 7% (3 km between 3% and 7%)
	Tunnel Hill to Summit	8 km between 0% and 5% (2 km between 3% and 5%)
	Summit to Mill Bay	11 km between 0% and 6.8% (2 km of 6.8%)
Proportion of Heavy Trucks		1.8%
Proportion of Buses		0.2%
Proportion of RV's		0.1%

## 1 Existing Highway - 2 lane Divided Rural Highway

Use FHWA Rural One-Lane Capacity

$$\text{One Lane Capacity} = (1600\text{pch} \times \text{PHF} \times f_G \times f_{HV}) - V_{NP}$$

### Step 1: Determine PHF (Peak Hour Factor)

At capacity PHF = 1.0

### Step 2: Determine $f_G$ (Grade Adjustment Factor)

At capacity two way flow rate will be > 1200 pch  
Mountainous Terrain

From FHWA Table 6:  $f_G = 0.99$

### Step 3: Determine $f_{HV}$ (Heavy Vehicle Factor)

$$f_{HV} = 1 / (1 + P_T(E_T - 1))$$

Where  $P_T = 2\%$  trucks and buses

$E_T = 7.2$  (from FHWA Table 7)

$$f_{HV} = 0.89$$

### Step 4: Determine $V_{NP}$ (Volume Adjustment for No Passing Zones)

$$V_{NP} = f_{NP} / 0.00776$$

to get  $f_{NP}$  we need two way demand flow rate  $V_P$

$$V_P = \frac{\text{Volume}_{\text{at capacity}}}{\text{PHF} \times f_G \times f_{HV}} = \sim 2400 / (.88 \times .99 \times .89) = 3095$$

From FHWA Table 8 @ 100% no passing,  $f_{NP} = 0.8$

$$V_{NP} = 0.8 / 0.00776 = 103$$

$$\text{One Lane Capacity} = (1600 \text{ pch} \times \text{PHF} \times f_G \times f_{HV}) - V_{NP}$$

$$= (1600 \times 1.0 \times 0.99 \times 0.89) - 103$$

$$= 1307 \text{ pch in one direction}$$

## 2 Multilane Highway – 4-lane divided

### Step 1: Calculate Free Flow Speed

$$\begin{aligned} \text{FFS} &= \text{BFFS} - f_{\text{LW}} - f_{\text{LC}} - f_{\text{M}} - f_{\text{A}} \\ \text{BFFS} &= \text{speed limit} + 11 \text{ for speed limit } 70 \text{ km/h} = 81 \text{ km/h} \\ f_{\text{LW}} &= 1.0 \text{ km/h (from Exhibit 21-4)} \\ f_{\text{LC}} &= 2.0 \text{ km/h (from Exhibit 21-5)} \\ f_{\text{M}} &= 0.0 \text{ km/h (from Exhibit 21-6)} \\ f_{\text{A}} &= 0.0 \text{ km/h (from Exhibit 21-7)} \\ \text{FFS} &= 78 \text{ km/h (49 mph)} \end{aligned}$$

### Step 2: Calculate Base Capacity (BaseCap)

$$\begin{aligned} \text{BaseCap} &= 1000 + 20 \times \text{FFS}; \text{ for FFS} \leq 60 \text{ mph} \\ &= 2200 \text{ for FFS} > 60 \text{ mph} \\ \text{BaseCap} &= 1000 + 20 \times 49 \\ &= 1980 \text{ pcphpl} \end{aligned}$$

### Step 3: Determine Peak Capacity (PeakCap)

$$\text{PeakCap} = \text{BaseCap} \times \text{PHF} \times N \times f_{\text{HV}} \times f_{\text{P}}$$

$$\begin{aligned} \text{PHF} &= \text{Peak Hour Factor} = 0.95 \text{ @ rural capacity (Table 5 FHWA)} \\ N &= 2 \text{ for 4-Lane Divided} \\ &= 3 \text{ for 4-Lane Divided with Climbing Lane} \\ f_{\text{P}} &= 0.95 \text{ for mixture of regular and non-regular users (summer)} \\ f_{\text{HV}} &= 1 / (1 + P_{\text{T}}(E_{\text{T}} - 1)) \\ P_{\text{T}} &= \text{Proportion of trucks and buses (RV's ignored)} \\ &= 2\% \\ E_{\text{T}} &= \text{grade dependent} \end{aligned}$$

### Peak Capacity per Direction:

4 Lane Divided			
Grade	$E_{\text{T}}$	$f_{\text{HV}}$	PeakCap (vph)
1%	4	0.94	3372
3%	10	0.85	3029
5%	14	0.79	2836
7%	17	0.76	2708

4 Lane Divided with Passing Lane			
Grade	$E_{\text{T}}$	$f_{\text{HV}}$	PeakCap (vph)
1%	4	0.94	5057
3%	10	0.85	4543
5%	14	0.79	4255
7%	17	0.76	4061