# **Technical Summary**

March 2009

Pit Name: Crescent Spur

Provincial Pit #: 1811

**Location:** The pit is located at the end of Prospect Road, approximately 5km down Crescent Spur Road (off Hwy 16), approximately 50km Northwest of McBride (Figure 1).

**Legal Land Description:** The site is currently a section 16 Map Reserve held by the Ministry of Transportation and Infrastructure. The legal land description of the reserve is the "Crown Land covering that Part of D.L. 8043, Cariboo District, as shown on the attached map and containing 13.2 hectares, more or less" (Figure 2).

**2009 Investigation:** The 2009 investigation consisted of 13 test pits with depths ranging from 3.1 to 5.1m. Based on the results of this investigation, Area A has been delineated as an area containing usable granular materials (Figure 3).

#### **Material Gradation**

Table 1 displays the gradation as percent by weight of the fines (silts and clays), sand and gravel component as well as the Unified Soil Classification (USC) for the samples from Area A.

Table 1: Deposit Gradation

Test Pit	Depth (m)	Fines (%)	Sand (%)	Gravel (%)	USC
TP 09-02	0.05-1.2	1.2	54.5	44.3	SW
TP 09-03	1-4.5	3.8	39.3	56.9	GW
TP 09-04	0.3-5.1	5.1	32.8	62.1	GW-GM
TP 09-07	0.7-2.8	2.7	24.2	73.1	GP
TP 09-08	2.3-4.1	2.9	36.2	60.9	GW
TP 09-12	0-1.9	4.1	42.4	53.5	GW
Average		3.3	38.2	58.5	GW

Table 2 includes the percent by weight of oversized rocks as well as the corrected percentages of fines, sand and gravel. The gravel is also divided into fine and coarse portions.

Table 2: Pit Run Gradation with Oversized Estimates

	% Fines	% Sand	% G	ravel	% Small	% Large
Test Pit	<0.075mm	0.075-	Fine 4.75-	Coarse 25-	Boulders	Boulders
		4.75mm	25 mm	75 mm	75-300 mm	>300mm
09-02	1.1	50.1	13.4	27.4	8	0
09-03	3.0	31.4	18.5	27.1	20	0
09-04	4.4	28.5	22.4	31.7	13	0
09-07	2.4	21.3	20.7	43.6	10	2
09-08	2.3	28.2	19.3	28.2	22	0
09-12	3.9	40.7	31.9	19.5	4	0
Average	2.9	33.4	21.0	29.6	12.8	0.3

### **Material Durability**

Durability tests were performed on the samples to verify that the aggregate is of sufficient quality to meet the specifications needed for various highway construction and maintenance products. They were tested for loss from abrasion (micro-deval), presence of plastic fines (sand equivalent), as well as absorption and relative density.

Table 3: Durability Test Results

Area A	Sand	Microdeval	Absor	rption	Relative Density				
Area A	Equivalent	(% loss)	Coarse	Fine	Coarse	Fine			
TP 09-02	87.6	11.52							
TP 09-03			1.39	1.31	2.600	2.617			
TP 09-04	65.8	19.34							
TP 09-07			1.09	1.21	2.600	2.636			
TRAN Standard Specs	≥ 20 Sub- base aggregate ≥40 base coarse & asphalt mix aggregates	≤ 30 sub-base aggregate ≤ 25 base course aggregate ≤ 17 asphalt mix aggregates	< 2.0 for paving aggregate <1.0 for graded aggregate seal	<1.5 for graded aggregate seal	~2.65	~2.65			

### **Material Suitability**

Table 4 summarizes the potential aggregate products that can be produced from Area A.

Table 4: Suitability

	Pit Run	Crush
Suitability	Bridge end fill SGSB	Base course aggregate Asphalt mix aggregate Graded aggregate seal

For pit run, Bridge End Fill and SGSB, the material will need to be screened to remove the larger oversize rocks. The absorption test results on the fine portion meets the requirements for Graded Aggregate Seal aggregates but are slightly higher on the coarse portion. However, it is anticipated that crushing of the oversized rocks with the gravel will improve the durability characteristics and that the crushed material should be suitable for a variety of aggregate products.

#### **Volume Estimates**

Based on depths encountered during the subsurface investigation, Table 5 consists of estimates of the volumes that can be expected for the top soil, overburden and aggregate from Area A that measures approximately 4.45 ha.

Table 5: Volume Estimates

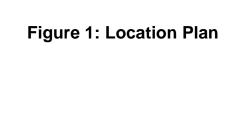
	Top Soil	Overburden	Aggregate
Average Layer Thickness (m)	0.1	0.4	4.0
Volume of material in development area (m <sup>3</sup> )	4,450	17,800	178,000

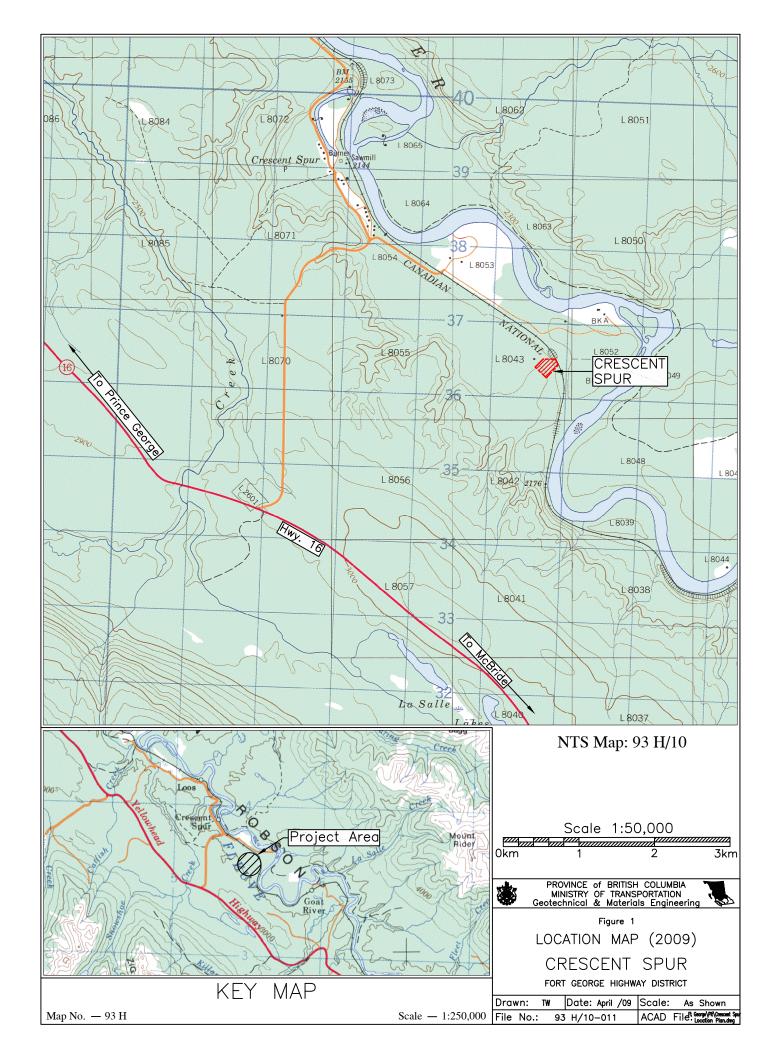
There is a potential for granular materials in the east/northeast portion of the reserve. Test Pit 09-13 excavated in this area encountered approximately 3.6m thick gravel layer and bottomed out in gravel. It is recommended that further exploration be carried out in this area to assess the potential.

## **Pit Development Notes**

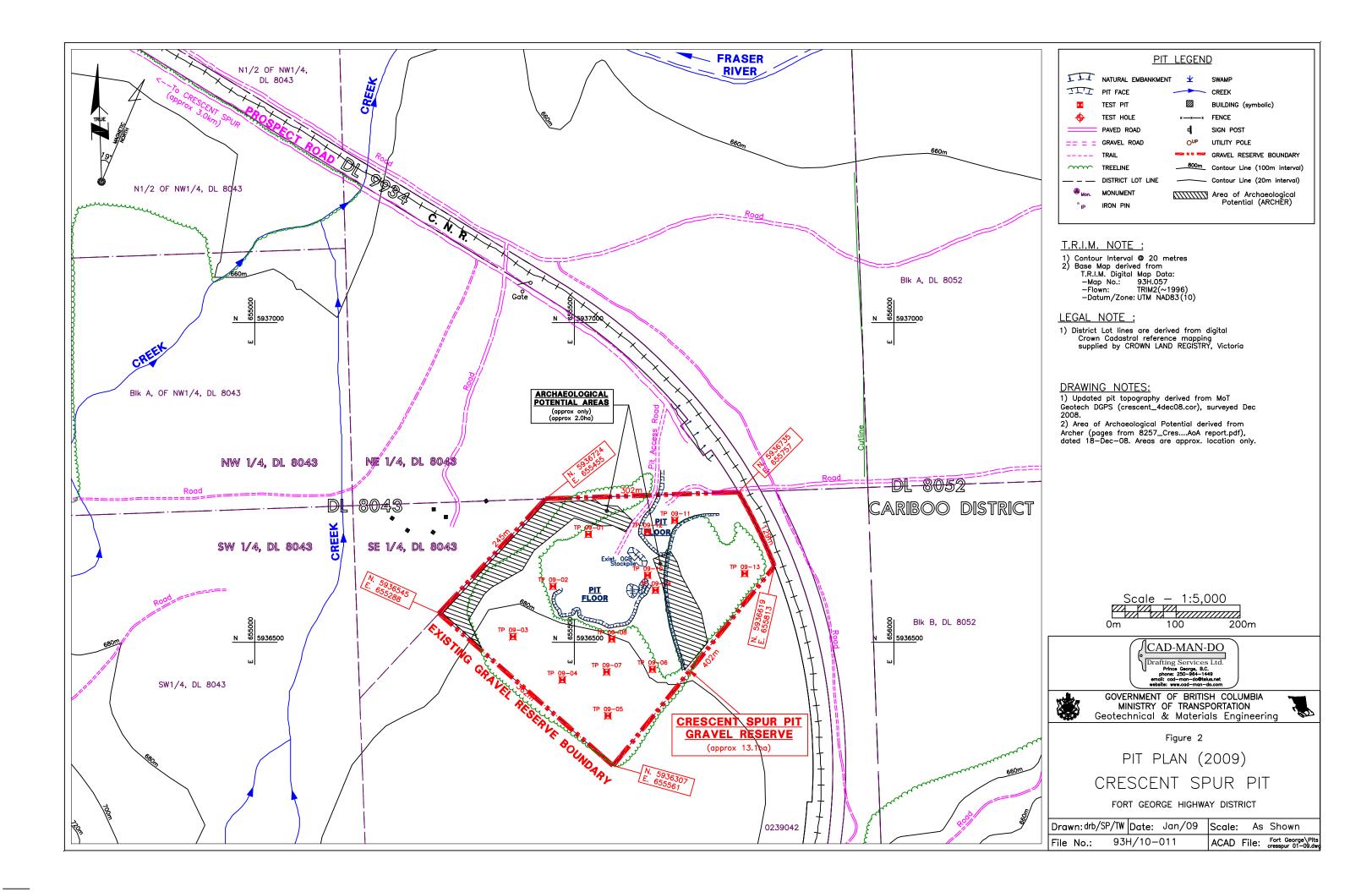
- All development must be carried out in accordance with the Health, Safety, and reclamation Code for Mines in British Columbia, BC Ministry of Energy and Mines, as well as the Standard Specifications for Highway Construction, BC Ministry of Transportation and Infrastructure.
- Development of the pit should continue from the existing pit face towards the south, as shown on the development plan. Area A can be accessed through the existing access road.
- There are two areas marked out in both Figure 2 and Figure 3 that are areas of high archaeological potential according to an Archaeological Overview Assessment (AOA). These areas are not to be entered unless an Archaeological Impact Assessment (AIA) is carried out and finds no archaeological artefacts.
- Groundwater was not encountered in any of the test pits.
- Vegetation consists of mature, but not large, coniferous and deciduous trees. For the most part they are spread out. The pit area has been previously logged so there is a tree line of mature trees that is the pit boundary. Area A will need to be cleared, but the timber may not be merchantable.

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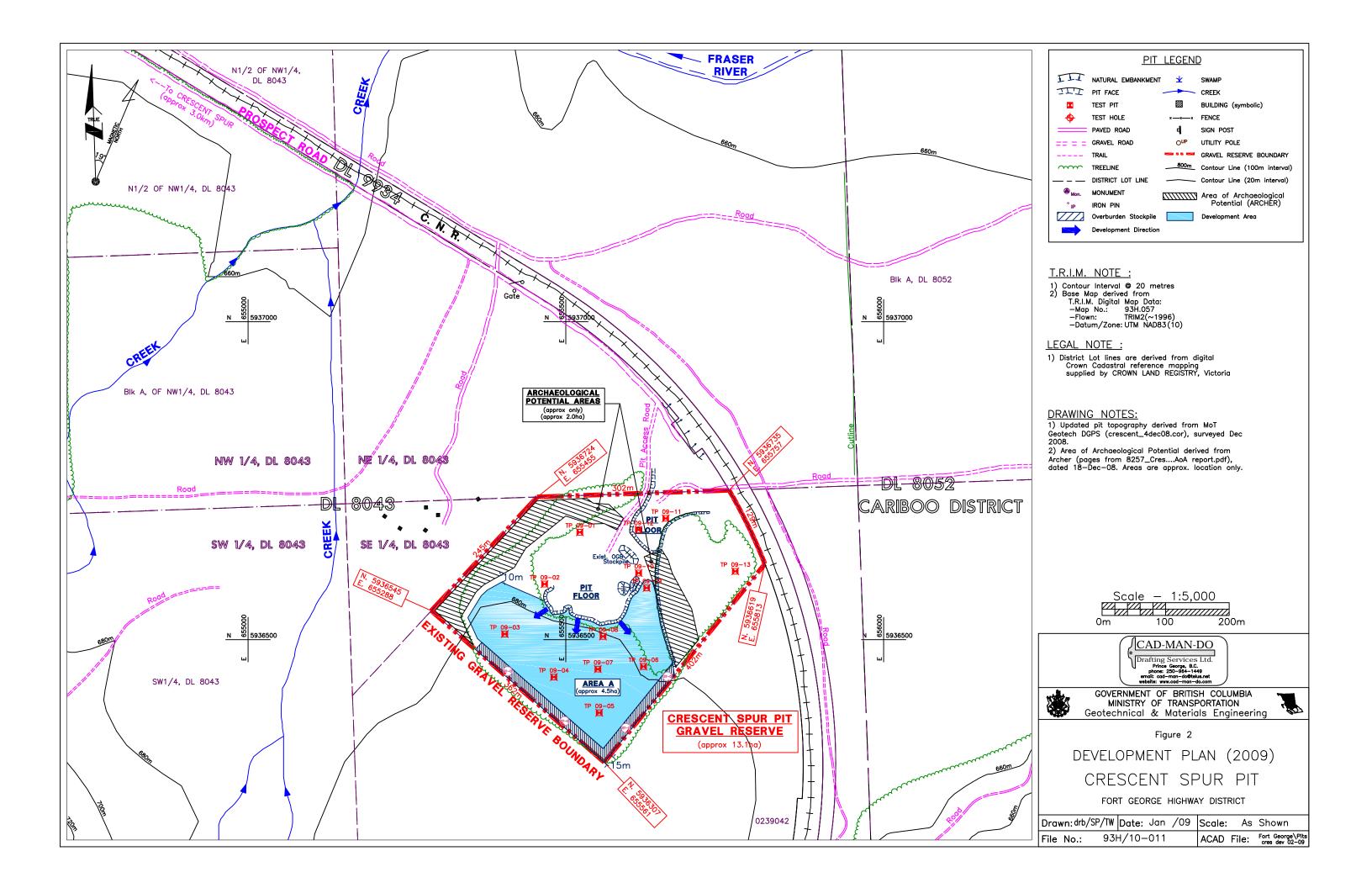


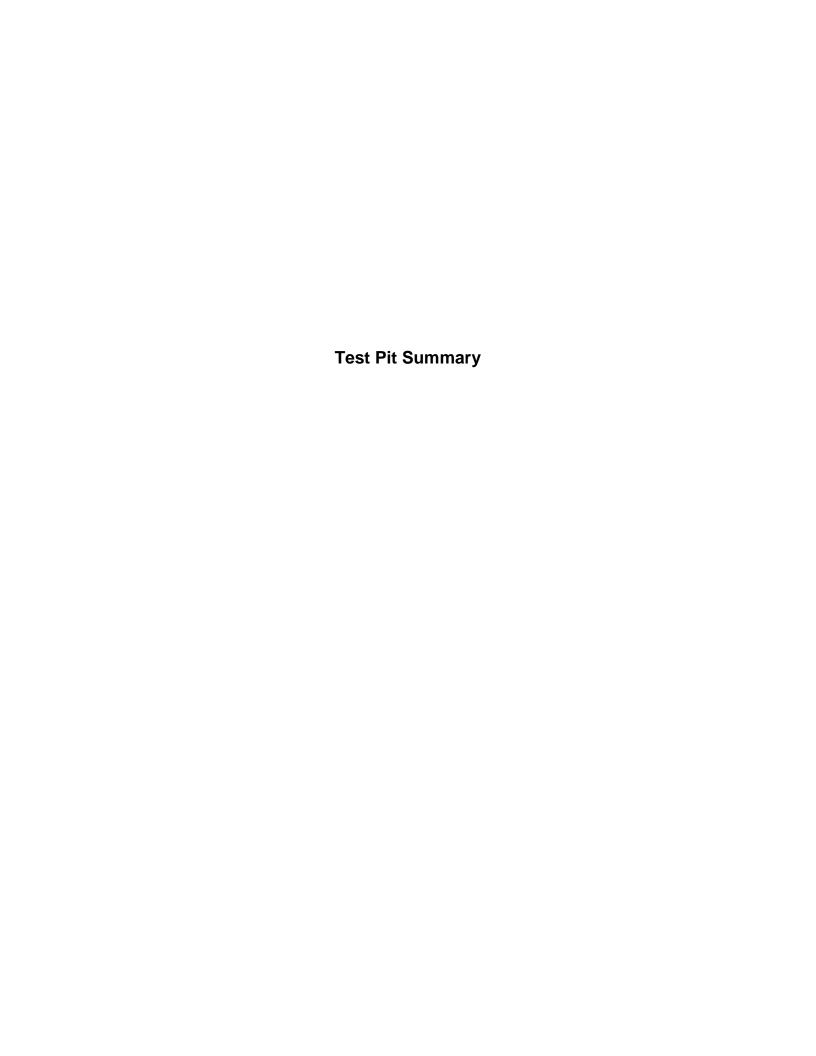






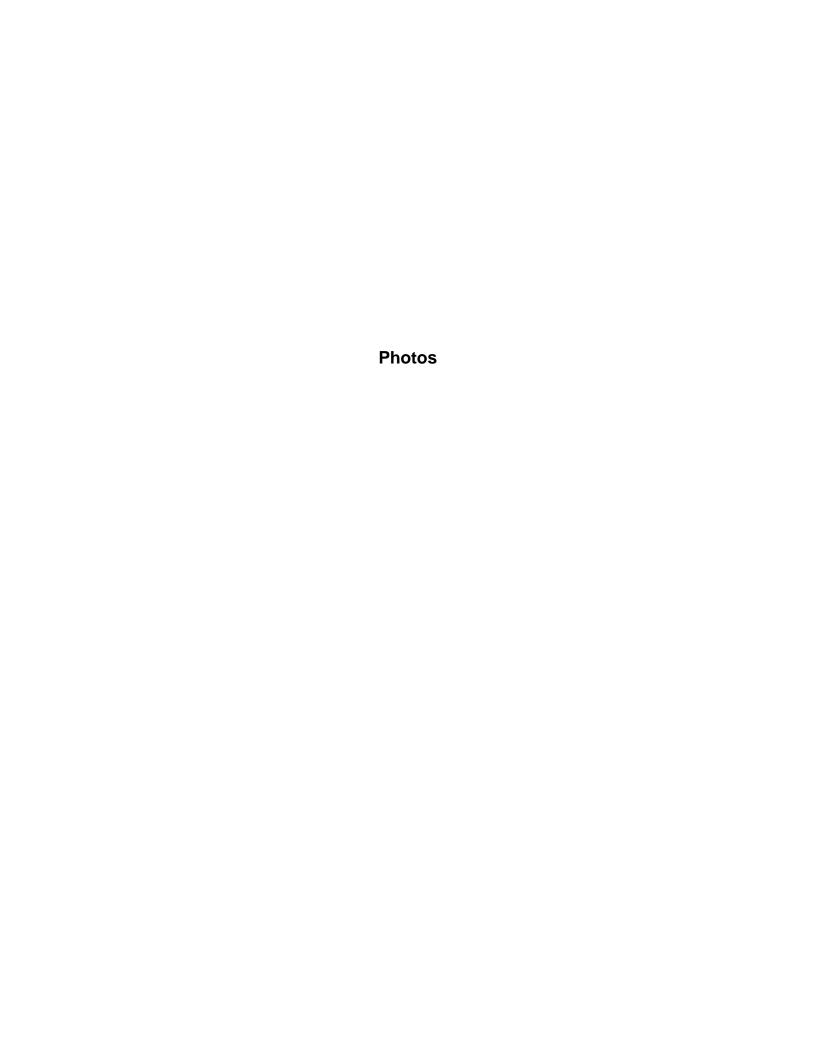






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TEST PIT NUMBER	SAMPLE NUMBER	From	(m) H H H H H H H H H H H H H H H H H H H	LAYER THICKNESS	SOIL	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	>300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	FRACTURE A/B	B.R.D coarse/fines	ABSORPTION %coarse/%fines
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09-01		0.0	0.1	0.1	TS OB																						
SA01	48689	0.2	1.2	1.0	GW	2	35	63	1			75															
SA02	36833	1.2	4.3	3.1	SP	4	94	2																			
00.00		0.0	0.05	0.4	TO	-																					
09-02 SA01	32554	0.05	0.05 1.2	0.1 1.2	TS GW	2	43	55	5	3	-	300		SW	1	55	44				87.6	11.52					
SA02	30235	1.2	4.3	3.1	SM	6	93	1	Ŭ			000			H	-00					07.0	11.02					
09-03		0.0	0.1	0.1	TS			10																			
SA01	1306	0.1 1.0	1.0 4.5	0.9 3.5	OB GW	8	80 30	12 66	15	5		300		GW	4	39	57									2.600/2.617	1 30/1 31
OA01	1300	1.0	4.5	3.3	GVV	-	30	00	13			300		OVV	_	33	31									2.000/2.017	1.53/1.51
09-04		0.0	0.1	0.1	TS																						
		0.1	0.3	0.2	OB																						
SA01	B0258	0.3	5.1	4.8	GWGM	5	30	65	10	3		250		GP-GM	5	33	62				65.8	19.34					
09-05		0.0	0.1	0.1	TS																						
		0.1	0.4	0.3	ОВ																						
	B0878	0.4	4.3	3.9	GMGW	6	40	54				0.5.7															
SA02	1343	4.3	5.1	0.8	SW	4	80	16	7	2		225		ML	52	24	24										
09-06		0.0	0.1	0.1	TS																						
33 00		0.1	0.4	0.3	OB																						
SA01	39927	0.4	4.4	4.0	GWGM	5	30	65																			
00.07		0.0	0.4	0.4	TO	-																					
09-07		0.0	0.1	0.1	TS OB	$\vdash$																					
SA01	38449	0.7	2.8	2.1	GW	4	36	60	5	5	2	400		GP	3	24	73									2.600/2.636	1.09/1.21
	32822	2.8	4.5	1.7	GP	3	27	70	10	7		200															
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TEST PIT NUMBER	SAMPLE NUMBER		DEP H (m)	AYER THICKNESS	SOIL	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	-300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND EQUIVALENT	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	FRACTURE A/B	B.R.D coarse/fines	ABSORPTION %coarse/%fines
		From	То	Ź	ŭ				75	15(	٨	¥	M	CLA							ш	₹		%	FR		A A
09-08		0.0	0.1	0.1	TS																						
		0.1	1.1	1.0	GW	4	41	55																			
		1.1	2.3	1.2	SP	4	86	10																			
SA01	50279	2.3	4.1	1.8	GP	3	40	57	15	7		225		GW	3	36	61										
00.00		0.0	0.05	0.1	TS								-														
09-09		0.05	0.05	0.1	OB																						
SA01	B0875	0.4	2.6	2.2	GW	2	38	60	8	8		300															
SA02	36975	2.6	4.4	1.8	SP	4	70	26	4	2		300		SM2	27	50	23										
09-10		0.0	3.1	3.1	SP	2	75	23	2		20	1100															
09-11		0.0	0.1	0.1	TS							0.50															
		0.1	3.2	3.1	SM2	20	55	25	3	2		250															
		3.2	4.3	1.1	SC1	18	77	5	1			100															
09-12	B0778	0.0	1.9	1.9	GW	1	45	54						GW	4	42	54										
00 12	50,70	1.9	4.4	2.5	SM2	25	65	10	3	1		280			-т	74	07										
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09-13		0.0	0.1	0.1	TS																						
		0.1	0.9	8.0	SP	3	87	10																			
SA01	32904	0.9	4.5	3.6	GWGM	5	35	60	10	5	3	310															





TP09-02





TP09-04



TP09-05



TP09-06



TP09-07



TP09-08





TP09-13