# Business Case <br> Highway 97 <br> Bentley Road to Deep Creek <br> 4-Laning 



Prepared for:
Ministry of Transportation
Southern Interior Region

Project No. MOT-65
18 May, 2004


## Executive Summary Highway 97-4-Laning Summerland to Peachland

The overall project involves 4-laning 15 km of Highway 97 between Summerland and Peachland, making it consistent with the highway cross section to the north and south. The intention of the project is to increase capacity and safety consistent with travel demand in the corridor.

2003 AADT is estimated to be 11,700 with $2 \%$ linear growth, The existing geometry of Highway 97 between Summerland and Peachland is constrained by steep terrain along the west shore of Lake Okanagan and has frequent speed advisories. Traffic volume is high for a 2-lane facility and combined with the curvilinear alignment along the lake shore, results in negligible passing opportunity during peak periods. Peak period speeds are often $10 \mathrm{~km} / \mathrm{hr}$ below the posted speed. Existing operation is LOS 'E' and projected is ' $F$ ' within the forecast period.

The existing accident rate and severity are close to expected values for a 2-lane rural highway but at this traffic volume, a 4 lane design, which potentially reduces fatal accidents by $32 \%$ and total accidents by $4 \%$, is justified. This amounts to saving one fatal accident every 6.5 years.

Average posted speed will increase $11 \%$ from 80 to $90 \mathrm{~km} / \mathrm{hr}$ which is more consistent with expectations for a high speed rural highway. Increases in travel speed during peak periods will be more pronounced with increases of 20 to $30 \mathrm{~km} / \mathrm{hr}$.

The planning estimate of staging and costs is summarized below.

| Staging | Section | Project Cost <br> (\$millions) |  |
| :---: | :--- | :---: | :---: |
| 1 | Okanagan Lake Park | $\$ 15.3$ |  |
| 2 | Bentley Road to Okanagan Lake Park | $\$ 22.8$ |  |
| 3 | Greata Ranch to Deep Creek | Total | $\$ 3.2$ |
|  |  | $\$ 51.4$ |  |

Overall, the project returns a benefit cost ratio of 1.22. $56 \%$ of benefits stem from time savings due to increased speed and capacity and some reduction in traveled distance. $39 \%$ of benefits stem from a reduction in accident severity and some reduction in accident rate.

Recommended project staging starts with the Okanagan Lake Park section which is already designed, is the most constructable and enjoys local support. The second stage Bentley Road to OK Lake section would make continuous the existing 4-laning from Penticton through to OK Lake Park. The third stage would complete 4-laning through to Peachland.

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# Highway 97 <br> Bentley Road to Greata Ranch - 4 Laning 

## 1 Introduction

### 1.1 Background

The overall objective is to increase capacity and safety consistent with travel demand ( $11,000+$ AADT) in the corridor over 15 km of existing 2-lane highway between Summerland and Peachland. Highway 97 to the north and south of this project is 4 lanes. The intention is to 4-lane the remaining 2-lane sections in stages. For staging purposes, the project is broken down into 5 sections and a business case is presented for each section as well as an overall summary for the project as a whole.

### 1.2 Project Description and Location

This project includes upgrading 14.98 km of rural Highway 97 (Segment 1115 LKI 31.51 to 46.49 ) between Peachland and Summerland from a 2 lane $80 / 90 \mathrm{~km} / \mathrm{hr}$ with frequent speed advisories to a 4-lane $100 \mathrm{~km} / \mathrm{hr}$ standard, posted $90 \mathrm{~km} / \mathrm{hr}$. The general location and staging sections are shown in Exhibit 1-1.

### 1.3 Previous Studies

Transportation in the Okanagan Valley has been investigated through a series of previous studies which have led up to the current project.

The Okanagan Valley Transportation Plan completed in 1997 provided a comprehensive review of travel characteristics and demand forecasts throughout the Okanagan Valley and an evaluation of broader corridor strategies evaluated at a system level. These concluded with the strategy to maintain and improve mobility of the existing highway corridor. Alternative routes were examined but were not recommended.

The more recent South Okanagan Corridor Management Plan ${ }^{1}$ (SOK CMP) identified the Summerland to Peachland section as the most congested part of the Highway 97 Corridor south of Peachland. Summerland to Peachland is a 2-lane highway bounded at either end by 4 lane highway to the south and north, leaving this 2-lane section as the remaining

[^0]choke point. It is operating at level of service 'E' and will decline to ' F ' during the forecast period. The Corridor Plan specifically recommends 4-laning this section.

Design work and cost estimates for part of the study area at Okanagan Lake Park have been prepared by McElhanney Engineering ${ }^{2}$ for a 4 lane, flush median, $100 \mathrm{~km} / \mathrm{hr}$ design with a total cost of $\$ 15.3$ million (excluding sunk costs).

## Exhibit 1-1 General Location



[^1]
### 1.4 Other Alternatives

Alternative routes were considered at the system level in the 1997 Okanagan Valley Transportation Plan but were not recommended. Other options such as selective passing lanes are possible but at current traffic levels, they will only provide limited relief during peak demand periods. The investment in passing lanes (3-lane sections) may be lost if it cannot be incorporated into a future 4-lane scenarios required during the forecast period. The proposed improvement is an isolated 4 lane section and acts as a long passing lane section providing relief within the treated section and to downstream 2 lane sections. The investment in this option can also be incorporated directly into the ultimate plan for continuous 4-laning.

## 2 Problem Statement

### 2.1 Corridor Role and Function

The Highway 97 corridor is a gateway for north-south international trade intended to provide an uncongested high quality route, with limited delay through urban areas. It links the United States to the northern half of British Columbia as well as regions east and west. The corridor serves development, recreation, and resource areas throughout the Okanagan. It links communities and urban centres including Osoyoos, Penticton, Kelowna, Vernon, Salmon Arm as well as serving commuter demands between rural areas and population centres. Traffic includes agriculture, tourism, recreation, forestry and mining.

Changes in demographic and economic trends are putting increasing pressure on the corridor. Population is projected to grow rapidly through the Okanagan Valley. This population growth will lead to further diversification of the economy through expansion in retail trade, construction and service industries. Spillover in recent growth from the Central Okanagan is now throughout the valley. Much of this growth has been fueled by the retirement industry. Continued expansion in the tourism sector is projected.

The heavy population growth in the Okanagan Valley is placing increasing traffic demands on Highway 97. Expansion of commuter sheds to more rural areas will increase conflicts between local and long distance travel, and increase the cost of moving freight. Expansion in tourism will add to congestion experienced, especially during peak summer periods.

### 2.2 Traffic Characteristics

Current and projected traffic volumes from the SOK CMP are:

## Exhibit 2-1 Traffic Volume

|  | 2000 | 2025 |
| :--- | :--- | :--- |
| AADT | 11,420 | 18,736 |
| SADT | 13,330 | 19,341 |
| DHV | 1,256 | 2,061 |

Traffic volume at counter 25-002 south of Peachland in 2001 was 11,359 AADT and 12,963 SADT with about $3.5 \%$ trucks and $10 \%$ recreational vehicles. Historical and projected traffic volumes based on 2001 counts are presented in Exhibit 2-2. Historical traffic is subsequent to the opening of the Coquihalla Connector in 1991.

## Exhibit 2-2 Historical and Projected AADT



### 2.3 Mobility

Highway 97 from Summerland to Peachland is the most congested section of Highway 97 south of Peachland. It presently operates at LOS 'E' and is projected to LOS 'F' in $2025^{3}$ For rural highways, the desirable level of service is 'C'. Posted speeds are presently 90 $\mathrm{km} / \mathrm{hr}$ and observed speeds are regularly travelling at $10 \mathrm{~km} / \mathrm{hr}$ below posted speeds. A high percentage of recreational traffic combined with limited passing opportunity due to opposing traffic volume and geometry result in heavy platooning in this section. There

[^2]are a number of speed advisories between Bentley Road and Deep Creek with reduced speed curves at the south end as low as 60 to $70 \mathrm{~km} / \mathrm{hr}$.

Average travel speed between Kaleden and Peachland is $80 \mathrm{~km} / \mathrm{hr}$ which is at the lower threshold of desired corridor speed. This is the result of reduced rural speeds and lack of passing opportunity in this section and delays in the urban Summerland area. The objective for this section is to safely attain consistent, high rural travel speeds at 90-100 $\mathrm{km} / \mathrm{hr}$.

## 3 Bentley Road to North Beach

### 3.1 Purpose

The purpose of this business case is to advance the project to preliminary design and costing. No detailed design has been completed for this section yet.

### 3.2 Project Description and Location

This project includes upgrading 3.29 km of rural Highway 97 from Bentley Road at the north end of Summerland to North Beach south of Okanagan Lake Park (Segment 1115 LKI 31.51 to 34.80 ) between Peachland and Summerland. The upgrade is from a 2 lane $90 \mathrm{~km} / \mathrm{hr}$ with $70 \mathrm{~km} / \mathrm{hr}$ advisories to a 4 lane $100 \mathrm{~km} / \mathrm{hr}$ standard. The general location is shown in Exhibit 3-1.

Exhibit 3-1 Bentley Road to North Beach


### 3.3 Mobility

This section is part of the overall Summerland to Peachland segment identified in the SOK CMP as deficient with LOS 'F" projected for 2025. Specific to this section are low truck speeds related to grades in both directions north of Bentley Road. There are also two $70 \mathrm{~km} / \mathrm{hr}$ advisories totaling 2.7 km out of the 3.3 km section length. The average posted speed is $74 \mathrm{~km} / \mathrm{hr}$. The existing posted speeds identified from the 1999 Intranet Photolog, Traversal H97 are summarized below. Proposed posted speeds are $90 \mathrm{~km} / \mathrm{hr}$ and design speed $100 \mathrm{~km} / \mathrm{hr}$.

## Exhibit 3-2 Posted Speed Advisories

|  | LKI | Length | Existing <br> Posted | Proposed <br> Posted |
| :--- | :---: | :---: | :---: | :---: |
| Bentley Road intersection | 31.51 | 0.226 | 70 | 90 |
| Start 2 lane NB | 31.74 | 0.163 | 70 | 90 |
| $70 \mathrm{~km} / \mathrm{h}$ Advisory Warning NB | 31.90 | 0.154 | 70 | 90 |
| $70 \mathrm{~km} / \mathrm{h}$ Advisory NB | 32.05 | 0.42 | 70 | 90 |
| End $70 \mathrm{~km} / \mathrm{h}$ Advisory | 32.47 | 0.624 | 70 | 90 |
| $90 \mathrm{~km} / \mathrm{h}$ Posted | 33.10 | 0.584 | 90 | 90 |
| $70 \mathrm{~km} / \mathrm{h}$ Advisory NB | 33.68 | 0.604 | 70 | 90 |
| Continued Advisory (no sign) | 34.29 | 0.515 | 70 | 90 |
| Continue 70 | 34.80 |  |  |  |

### 3.4 Safety Performance

The existing accident rate in this section is not above the expected rate for a 2 lane highway with this access density. The expected rate is $0.33 \mathrm{a} / \mathrm{mvk}$ and the observed rate is $0.28 \mathrm{a} / \mathrm{mvk}$. The accident histogram shows clustering at intersections and accesses and at the first reduced speed curve in the NB direction. Expected safety improvements stem from a reduction in severity with construction of a 4-lane cross section.

## Exhibit 3-3 Accident Histogram 1998 to 2002 - Bentley Road to North Beach



The SOK CMP did not identify any accident prone locations in this section based on 1991 to 1995 data. The accident analysis for the current business case report uses data for 5 years from 1998 to 2002. The estimated existing accident rate in this section is 0.28 $\mathrm{a} / \mathrm{mvk}$ which is below the expected rate of $0.33 \mathrm{a} / \mathrm{mvk}$ for this section. but the severity appears greater than expected. Fatal and injury accidents compose $72 \%$ of the 18 accidents between 1998 and 2002. Normal expectations for fatal and injury accidents would be $52 \%$ for this road class and intersection density. Nine of the 18 accidents between 1998 and 2002 in this segment are head-on and run-off-road accidents which is typical for winding rural 2-lane but can be improved considerable with better alignment and wider cross section. There are 3 accesses in this segment including Bentley Road, one viewpoint and one rest area.

Existing and proposed safety performance is summarized in Exhibit 3-4. The approach used to estimate accident rates is summarized in Appendix A.

## Exhibit 3-4 Accident Rates Used for Analysis

|  | Fat | Inj | PDO | at \& In | All |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Number of Accidents (5 years) |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 1 | 12 | 5 | 13 | 18 |
| Predicted Base Case Accidents (RAU2) | 0.59 | 10.5 | 10.2 | 11.1 | 21.3 |
| Base Case Estimate used for Analysis | 0.52 | 9.0 | 8.7 | 9.5 | 18.1 |
| Predicted Accidents for Proposed Case (RAU4) | 0.35 | 9.1 | 8.1 | 9.5 | 17.6 |
| Accident Reduction Potential (5 yrs) | 0.16 | -0.1 | 0.6 | 0.0 | 0.6 |
| Accident Rate (a/mvk) |  |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 0.015 | 0.18 | 0.08 | 0.20 | 0.28 |
| Predicted Base Case Accidents (RAU2) | 0.009 | 0.16 | 0.16 | 0.17 | 0.33 |
| Base Case Estimate used for Analysis | 0.008 | 0.14 | 0.13 | 0.15 | 0.28 |
| Predicted Accidents for Proposed Case (RAU4) | 0.005 | 0.14 | 0.12 | 0.15 | 0.27 |
| Accident Reduction Potential (5 yrs) | 32\% | -1\% | 6\% | 0\% | 3\% |

### 3.5 Summary of Deficiencies

| Criteria | Description |
| :--- | :--- |
| Safety | The accident rate is close to but below the expected value for a 2 <br> lane rural highway. Severity is close to the expected value. <br> Corridor Mobility was measured at $77.7 \mathrm{~km} / \mathrm{hr}$ during peak periods <br> while the desired rural corridor speed is $80 \mathrm{~km} / \mathrm{hr}$. Average posted <br> speed in this section is 73.6. Trucks speeds are low due to grades in <br> both directions. This section is 3.3 km long posted $90 \mathrm{~km} / \mathrm{hr}$ but <br> with two $70 \mathrm{~km} / \mathrm{hr}$ advisory curves totaling 2.7 km. |
| Geotechnical | Alignment is physically constrained by the steep slopes on the west <br> side of Lake Okanagan |
| Infrastructure | No substandard structures. <br> Local pavement condition is good with a PCR rating close to 8. |

### 3.6 Option Evaluation

The preferred option is a 4 lane undivided $100 \mathrm{~km} / \mathrm{hr}$ design consistent with the overall plan to upgrade Highway 97 between Peachland and Summerland. This will provide a level of service and safety consistent with projected traffic volumes for the long term.

The existing section is 3.29 km long. The estimated project costs for this section are $\$ 11.3$ million assuming 3.22 km of new construction at $\$ 3.5$ million $/ \mathrm{km}$ along a slightly shorter alignment.

Results of the Benefit Cost Analysis are presented in Exhibit 3-5 followed by an interpretation of the results.

## Exhibit 3-5 Benefit Cost Analysis Highway 97 - Bentley Road to North Beach

## Discounted Costs (2003 \$millions)

| Construction | $\$ 10.646$ |
| :--- | :---: |
| Salvage Value | $\$ 2.229$ |
| Mtce \& Rehabilitation | $\$ 0.318$ |
| Total Discounted Cost | $\$ 8.735$ |

Discounted Benefits

| Delay Reduction | $\$ 8.870$ |
| :--- | :---: |
| VOC Reduction | $\$ 0.665$ |
| Accident Reduction | $\$ 3.271$ |
| Total Discounted Benefits | $\$ 12.806$ |


| Net Present Value (NPV) | $\$ 4.07$ |
| :--- | :---: |
| NPV/Project Cost | 0.36 |
| B/C Ratio | 1.5 |
| MicroBencost File | BENTLEY |

The project returns a benefit cost ratio of 1.5 . Ratios greater than 1.0 are deemed to be economically beneficial to society. Benefits stem mainly first from time savings and second from reduced accident costs. Travel time savings derive from eliminating the 70 $\mathrm{km} / \mathrm{hr}$ advisories, increasing the capacity and shortening the traveled distance slightly.

Accident cost savings stem from a reduction in accident rate and severity associated with the higher design standard. There is an estimated $3 \%$ overall reduction in accidents but an estimated $32 \%$ reduction in fatal accidents. The wider cross section and higher design speed will help to reduce the number and severity of accidents.

### 3.7 Sensitivity Analysis

This is intended to show the impact of alternative project assumptions on the results. In this case, NPV remains positive over the range of input assumptions tested.

## Exhibit 3-6 Sensitivity Analysis

|  | Sensitivity Option |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Baseline NPV | $\begin{aligned} & \text { 4\% Discount } \\ & \text { Rate } \end{aligned}$ | $\begin{gathered} \text { 8\% } \\ \text { Discount } \\ \text { Rate } \end{gathered}$ | $+25 \%$ <br> Construction Cost | $\begin{array}{\|c\|} \hline-25 \% \\ \text { Construc- } \\ \text { tion Cost } \end{array}$ | Traffic Growth 2.2\% | Traffic Growth 1.2\% |
|  | Net Present Value (millions \$) |  |  |  |  |  |  |
| NPV | 4.1 | 7.2 | 1.8 | 1.2 | 6.9 | 4.8 | 3.4 |

### 3.8 Project Timing

The optimum timing is the implementation year that would maximize the net present value of the investment. It is measured as the year when the annual rate of return (annual benefits) exceeds the discount rate. A straight line interpolation of annual benefits suggests that this section is near its optimum timing.

Exhibit 3-7 Optimum Timing

| Discount <br> Rate | Optimum <br> Timing |
| :---: | :---: |
|  |  |
| $4 \%$ | 1979 |
| $6 \%$ | 1993 |
| $8 \%$ | 2007 |

### 3.9 Conclusions and Recommendations

The improvements have the potential reduce accidents by $3 \%$ and fatal accidents by $32 \%$. The added capacity will relieve congestion for the planning period and beyond and contribute to the corridor's role as a high speed north/south route consistent with long term planning objectives.

Based on the positive results of the benefit cost analysis, the growing congestion and bottleneck effect of the Peachland to Summerland section, the Bentley Road to North Beach Project should proceed within the context of an overall improvement program for Summerland to Peachland.

## 4 North Beach Project

### 4.1 Purpose

The purpose of this business case is to advance the project to design and engineering. No detailed design has been completed for this section.

### 4.2 Project Description and Location

This project includes upgrading 2.20 km of rural Highway 97 In the vicinity of North Beach Road on Highway 97 north of Summerland (Segment 1115 LKI 34.8 to 37.0) from a 2 lane $90 \mathrm{~km} / \mathrm{hr}$ with speed advisories as low as $60 \mathrm{~km} / \mathrm{hr}$ to a 4 lane $100 \mathrm{~km} / \mathrm{hr}$ standard. The general location is shown in Exhibit 4-1.

## Exhibit 4-1 North Beach Project



### 4.3 Mobility

This section is part of the overall Summerland to Peachland segment identified in the SOK CMP as deficient with LOS 'F" projected for 2025. Specific to this section are low truck speeds related to grades and frequent speed advisories. The average posted speed is 73 km/hr.

## Exhibit 4-2 Posted Speeds

|  | LKI | Length (km) | Existing | Proposed |
| :---: | :---: | :---: | :---: | :---: |
| Continued Advisory (no sign) | 34.80 | 0.377 | 70 | 90 |
| End of $70 \mathrm{~km} / \mathrm{h}$ Advisory (anticipated) | 35.18 | 0.512 | 90 | 90 |
| $80 \mathrm{~km} / \mathrm{h}$ Advisory NB | 35.69 | 0.246 | 80 | 90 |
| Continued Advisory (no sign) | 35.94 | 0.102 | 80 | 90 |
| Continued Advisory (no sign) | 36.04 | 0.072 | 80 | 90 |
| Start $60 \mathrm{~km} / \mathrm{h}$ Advisory NB | 36.11 | 0.583 | 60 | 90 |
| End 60 km/h Advisory (anticipated) | 36.69 | 0.163 | 70 | 90 |
| North Beach Road intersection | $\begin{aligned} & 36.86 \\ & 37.00 \\ & \hline \end{aligned}$ | 0.145 | 50 | 80 |
|  | Wtd. Avg. Posted |  | 72.6 | 89.34 |

### 4.4 Safety Performance

The existing accident rate in this section is not above the expected rate for a 2 lane highway with this access density. The expected rate is $0 . .31 \mathrm{a} / \mathrm{mvk}$ and the observed rate is $0.26 \mathrm{a} / \mathrm{mvk}$. The accident histogram shows clustering at North Beach Road and the 60 $\mathrm{km} / \mathrm{hr}$ advisory zone. The South Okanagan CMP did not identify any accident prone locations or in this section.

Exhibit 4-3 Accident Histogram 1998 to 2002 - Highway 97 North Beach


The SOK CMP did not identify any accident prone locations in this section based on 1991 to 1995 data. The accident analysis for the current business case report uses data for 5 years from 1998 to 2002. The estimated existing accident rate in this section is 0.26 $\mathrm{a} / \mathrm{mvk}$ which is below the expected rate of $0.28 \mathrm{a} / \mathrm{mvk}$, assuming approach volume on North Beach Road equivalent to 3\% of Highway 97 volume. While the accident rate is not higher than expected, 10 of the 11 accidents between 1998 and 2002 are run-off-road accidents potentially suggests some alignment issues. There has been 1 fatal accident in this section in the 5 year data period. Existing and proposed safety performance is estimated in Appendix A and summarized in Exhibit 4-4.

## Exhibit 4-4 Accident Rates Used for Analysis

|  | Fat | Inj | PDO | Fat \& In | All |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Number of Accidents (5 years) |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 1 | 5 | 5 | 6 | 11 |
| Predicted Base Case Accidents (RAU2) | 0.36 | 5.9 | 5.7 | 6.3 | 12.0 |
| Base Case Estimate used for Analysis | 0.31 | 5.5 | 5.3 | 5.8 | 11.1 |
| Predicted Accidents for Proposed Case (RAU4) | 0.20 | 4.9 | 4.4 | 5.1 | 9.5 |
| Accident Reduction Potential (5 yrs) | 0.11 | 0.6 | 0.9 | 0.7 | 1.6 |
| Accident Rate (a/mvk) |  |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 0.023 | 0.11 | 0.11 | 0.14 | 0.25 |
| Predicted Base Case Accidents (RAU2) | 0.008 | 0.14 | 0.13 | 0.14 | 0.28 |
| Base Case Estimate used for Analysis | 0.007 | 0.13 | 0.12 | 0.13 | 0.25 |
| Predicted Accidents for Proposed Case (RAU4) | 0.005 | 0.11 | 0.10 | 0.12 | 0.22 |
| Accident Reduction Potential (5 yrs) | 35\% | 11\% | 16\% | 12\% | 14\% |

### 4.5 Summary of Deficiencies

| Criteria | Description |
| :--- | :--- |
| Safety | The accident rate and severity is close to the expected value for a 2 <br> lane rural highway with 1 intersection. Run-off-road accidents are <br> over-represented. <br> Mobility <br> Geotechnical <br> Isth the speed advisories, the average posted speed in this section <br> is 72.6 km/hr compared to a desired 80 km/hr. Truck speeds are <br> low due to grades in both directions. |
| Infrastructure | Alignment is physically constrained by the steep slopes on the west <br> side of Lake Okanagan |
| No substandard structures. <br> Local pavement condition is good with a PCR rating close to 8. |  |

### 4.6 Option Evaluation

The preferred option is a 4 lane undivided $100 \mathrm{~km} / \mathrm{hr}$ design consistent with the overall plan to upgrade Highway 97 between Peachland and Summerland. This will provide a level of service and safety consistent with projected traffic volumes for the long term.

The existing section is 2.20 km long. The estimated project costs for this section are $\$ 7.5$ million assuming 2.16 km of new construction at $\$ 3.5$ million $/ \mathrm{km}$.

Results of the Benefit Cost Analysis are presented in Exhibit 4-5 followed by an interpretation of the results.

Exhibit 4-5 Benefit Cost Analysis Highway 97 - North Beach

Discounted Costs (2003 \$millions)

| Construction | $\$ 7.119$ |
| :--- | :--- |
| Salvage Value | $\$ 1.491$ |
| Mtce \& Rehab C | $\$ 0.016$ |
| Total Discounted Cost | $\$ 5.644$ |

## Discounted Benefits

| Delay Reduction | $\$ 6.239$ |
| :--- | :--- |
| VOC Reduction | $\$ 0.508$ |
| Accident Reduction | $\$ 2.329$ |
| Total Discounted Benefits | $\$ 9.076$ |


| Net Present Value (NPV) | $\$ 3.43$ |
| :--- | :---: |
| NPV/Project Cost | 0.45 |
| B/C Ratio | $\mathbf{1 . 6}$ |
| MicroBencost File | NBEACH |

The project returns a benefit cost ratio of 1.6. Ratios greater than 1.0 are deemed to be economically beneficial to society. Benefits stem mainly from time savings and secondly from reduced accident costs. Travel time savings derive from improving the alignment to eliminate advisory speed zones, from increasing the capacity and from shortening the traveled distance slightly.

Accident cost savings stem from a reduction in accident rate and severity associated with the higher design standard. There is an estimated $14 \%$ overall reduction in accidents and an estimated $35 \%$ reduction in fatal accidents. The wider cross section and higher design speed will help to reduce the severity of accidents while the improved alignment would help reduce the number.

### 4.7 Sensitivity Analysis

This is intended to show the impact of alternative project assumptions on the results. In this case, NPV remains positive over the range of input assumptions tested.

## Exhibit 4-6 Sensitivity Analysis

|  |  |  |  | sitivity Opti |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Baseline NPV | 4\% Discount Rate | $\begin{array}{\|c\|} \hline 8 \% \\ \text { Discount } \\ \text { Rate } \end{array}$ | $+25 \%$ <br> Construction Cost | $-25 \%$ Construction Cost | Traffic Growth +0.5\% | Traffic <br> Growth - <br> $.5 \%$ |
|  |  |  | Net Prese | t Value (mi | ons \$) |  |  |
| NPV | 3.4 | 5.7 | 1.8 | 1.5 | 5.3 | 3.9 | 3.0 |

### 4.8 Project Timing

The optimum timing is the implementation year that would maximize the net present value of the investment. It is measured as the year when the annual rate of return (annual benefits) exceeds the discount rate. A straight line interpolation of annual benefits suggests that this section is near its optimum timing.

## Exhibit 4-7 Optimum Timing

| Discount <br> Rate | Optimum <br> Timing |
| :---: | :---: |
|  |  |
| $4 \%$ | 1977 |
| $6 \%$ | 1991 |
| $8 \%$ | 2004 |

### 4.9 Conclusions and Recommendations

The improvements have the potential to reduce accidents by $14 \%$ and fatal accidents by $35 \%$. The added capacity will relieve congestion for the planning period and beyond and contribute to the corridor's role as a high speed north/south route consistent with long term planning objectives.

Based on the positive results of the benefit cost analysis, the growing congestion and bottleneck effect of the Peachland to Summerland section, the North Beach Project should proceed within the overall improvement program for Summerland to Peachland.

## 5 North Beach to Okanagan Lake Park

### 5.1 Purpose

The purpose of this business case is to advance the project to preliminary design and cost estimating. No detailed design has been completed for this section.

### 5.2 Project Description and Location

This project includes upgrading 1.17 km of rural Highway 97 south of Okanagan Lake Park. between Summerland and Peachland (Segment 1115 LKI 37.0 to 38.17) from a 2 lane $80 \mathrm{~km} / \mathrm{hr}$ with reduced speed curves to a 4 lane $100 \mathrm{~km} / \mathrm{hr}$ standard. The general location is shown in Exhibit 5-1.

Exhibit 5-1 North Beach to Okanagan Lake Park


### 5.3 Mobility

This section is part of the overall Summerland to Peachland segment identified in the SOK CMP as deficient with LOS 'F" projected for 2025. The average posted speed is $77.3 \mathrm{~km} / \mathrm{hr}$ assuming some influence from the $60 \mathrm{~km} / \mathrm{hr}$ at the north end of the section.

## Exhibit 5-2 Posted Speeds

Start of section
Advance Warning SLOW sign
60 km/h Advisory NB
End of Section

| LKI | Length <br> $(\mathrm{km})$ | Existing | Propose <br> d |
| :---: | :---: | :---: | :---: |
| 37.00 | 0.6 | 80 | 90 |
| 37.60 | 0.414 | 80 | 90 |
| 38.01 | 0.156 | 60 | 90 |
| 38.17 |  | 60 | 90 |
| Wtd. Avg. Posted |  | 77.3 | 90 |

### 5.4 Safety Performance

The existing accident rate in this section is not above the expected rate for a 2 lane highway with this access density. The expected rate is $0.27 \mathrm{a} / \mathrm{mvk}$ and the observed rate (based on 4 accidents in 5 years) is $0.17 \mathrm{a} / \mathrm{mvk}$. The rate used for analysis is $0.19 \mathrm{a} / \mathrm{mvk}$. The accident histogram in Exhibit 5-3 shows clustering at the viewpoint access and at the reduced speed curve.

Exhibit 5-3 Accident Histogram 1998 to 2002 - North Beach Road Area


The SOK CMP identified the viewpoint access as a high severity location, but caution should be used since this is likely based on a limited sample size. Existing and proposed safety performance is estimated in Appendix A and summarized here. The proposed case assumes a reduction in accident severity due to the wider cross section but no decrease in rate since the existing rate is already low. It may be that traffic has slowed down by this point due to the upstream speed reductions at reduced speed curves and there is a corresponding reduction in accidents in this section. This situation would reverse if upstream sections are improved leaving this as an isolated 2-lane section.

## Exhibit 5-4 Accident Rates Used for Analysis

|  | Fat | Inj | PDO | Fat \& Inj | All |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of Accidents (5 years)      <br> Observed Base Case Accidents 0 2 2 2 4 <br> Predicted Base Case Accidents <br> (RAU2) 0.19 3.1 3.0 3.3 6.3 <br> Base Case Estimate used for <br> Analysis 0.12 2.1 2.1 2.3 4.3 <br> Predicted Accidents for Proposed <br> Case (RAU4) 0.07 2.3 2.0 2.4 4.3 <br> Accident Reduction Potential (5 yrs) 0.05 -0.1 0.1 -0.1 0.0 <br> Observed Base Case Accidents <br> (1998 to 2002) 0.000 0.09 0.09 0.09 0.17 <br> Predicted Base Case Accidents <br> (RAU2) 0.008 0.13 0.13 0.14 0.27 <br> Base Case Estimate used for <br> Analysis 0.005 0.09 0.09 0.10 0.19 <br> Predicted Accidents for Proposed <br> Case (RAU4) 0.003 0.10 0.08 0.10 0.19 <br> Accident Reduction Potential (5 yrs) $39 \%$ $-7 \%$ $5 \%$ $-4 \%$ $0 \%$ |  |  |  |  |  |

### 5.5 Summary of Deficiencies

| Criteria | Description |
| :--- | :--- |
| Safety | The accident rate and severity is below the expected value for a 2 <br> lane rural highway with 1 intersection. |
| Geotechnical | With the speed advisories, the average posted speed in this section <br> is $77.3 \mathrm{~km} / \mathrm{hr}$ compared to a desired $80 \mathrm{~km} / \mathrm{hr}$. Truck speeds are <br> low due to grades in both directions. |
| Infrastructure | Alignment is physically constrained by the steep slopes on the west <br> side of Lake Okanagan |
| No substandard structures. <br> Local pavement condition is good with a PCR rating close to 8. |  |

### 5.6 Option Evaluation

The preferred option is a 4 lane undivided $100 \mathrm{~km} / \mathrm{hr}$ design consistent with the overall plan to upgrade Highway 97 between Peachland and Summerland. This will provide a level of service and safety consistent with projected traffic volumes for the long term.

The existing section is 1.17 km long. The estimated project costs for this section are $\$ 4.0$ million assuming 1.15 km of new construction at $\$ 3.5$ million $/ \mathrm{km}$.

Results of the Benefit Cost Analysis are presented in Exhibit 5-5 followed by an interpretation of the results.

## Exhibit 5-5 Benefit Cost Analysis Highway 97 - North Beach to Okanagan Lake Park

Discounted Costs (2003 \$millions)

| Construction | $\$ 3.786$ |
| :--- | :--- |
| Salvage Value | $\$ 0.793$ |
| Mtce \& Rehab C | $\$ 0.123$ |
| Total Discounted Cost | $\$ 3.116$ |

Discounted Benefits

| Delay Reduction | $\$ 2.510$ |
| :--- | :--- |
| VOC Reduction | $\$ 0.326$ |
| Accident Reduction | $\$ 0.910$ |
| Total Discounted Benefits | $\$ 3.746$ |


| Net Present Value (NPV) | $\$ 0.63$ |
| :--- | :---: |
| NPV/Project Cost | 0.16 |
| B/C Ratio | 1.2 |
| MicroBencost File | NBCH2OKL |

The project returns a benefit cost ratio of 1.2. Ratios greater than 1.0 are deemed to be economically beneficial to society. Benefits stem mainly from time savings due to the improved alignment, eliminating advisory speed zones, increasing the capacity and from shortening the traveled distance slightly.

Accident cost savings are minor since the existing accident rate is already lower than expected. As suggested previously, it may be that traffic has slowed down by this point due to the upstream speed reductions at reduced speed curves and there is a corresponding reduction in accidents in this section. Benefits stem from a reduction in accident severity associated with the higher design standard. There is an estimated $39 \%$ reduction in fatal accidents, 7\% increase in injury accidents and 5\% reduction PDO accidents. The wider cross section and higher design speed will help to reduce the severity of accidents and improve consistency with upstream and downstream sections.

### 5.7 Sensitivity Analysis

This is intended to show the impact of alternative project assumptions on the results. In this case, NPV is sensitive to discount rate and construction cost.

Exhibit 5-6 Sensitivity Analysis


### 5.8 Project Timing

The optimum timing is the implementation year that would maximize the net present value of the investment. It is measured as the year when the annual rate of return (annual benefits) exceeds the discount rate. A straight line interpolation of annual benefits suggests that this section is near its optimum timing. Ultimately this is a small section and timing should coincide with adjacent projects in order to avoid leaving an isolated 2lane section which potentially would become accident prone.

Exhibit 5-7 Optimum Timing

| Discount <br> Rate | Optimum <br> Timing |
| :---: | :---: |
|  |  |
| $4 \%$ | 1985 |
| $6 \%$ | 2002 |
| $8 \%$ | 2019 |

### 5.9 Conclusions and Recommendations

The improvements have the potential to reduce accident severity but not accident rate. It may be that traffic in this section has slowed down due to the upstream speed reductions at reduced speed curves in other sections and there is a corresponding reduction in accidents in this section. This situation would reverse if upstream sections are improved leaving this as an isolated 2-lane section

The added capacity will relieve congestion for the planning period and beyond and contribute to the corridor's role as a high speed north/south route consistent with long term planning objectives.

Based on the positive results of the benefit cost analysis, the growing congestion and bottleneck effect of the Peachland to Summerland section, the North Beach to Okanagan Lake Park section should proceed within the overall improvement program for Summerland to Peachland.

## 6 Okanagan Lake Park to Greata Ranch - 4 Laning Project 20283

### 6.1 Introduction

The purpose of this business case is to advance the project to Property Acquisition and Construction over three fiscal years $03 / 04,04 / 05$ and $05 / 06$. This project includes upgrading 4.51 km of rural Highway 97 at Okanagan Lake Park (Segment 1115 LKI 38.17 to 42.68 ) between Peachland and Summerland from a 2 lane $90 \mathrm{~km} / \mathrm{hr}$ (with a 70 $\mathrm{km} / \mathrm{hr}$ advisory) to a 4 lane $100 \mathrm{~km} / \mathrm{hr}$ standard. The general location is shown in Exhibit 6-1 .

Design work and cost estimates for this project have been prepared by McElhanney Engineering ${ }^{4}$ for a 4 lane, flush median, $100 \mathrm{~km} / \mathrm{hr}$ design with a total cost of $\$ 16.3$ million (excluding sunk costs of $\$ 332,000$ ). Subsequent modifications to allow for changes such as drainage plans, retaining walls and eliminating a proposed overpass structure bring the current estimate to $\$ 15.65$ million.

Exhibit 6-1 General Location


[^3]
### 6.2 Mobility

This section is part of the overall Summerland to Peachland segment identified in the SOK CMP as deficient with LOS 'F" projected for 2025. The average posted speed is $86.5 \mathrm{~km} / \mathrm{hr}$.

## Exhibit 6-2 Posted Speeds

60 km/h Advisory NB
End of $60 \mathrm{~km} / \mathrm{h}$ Advisory
South Access to Okanagan Lake Park
Access to Provincial Park Office
North Access to Okanagan Lake Park
$90 \mathrm{~km} / \mathrm{h}$ Posted

| LKI | Length <br> $(\mathrm{km})$ | Existing <br> Posted | Proposed <br> Posted |
| :---: | :---: | :---: | :---: |
| 38.17 | 0.374 | 90 | 90 |
| 38.54 | 0.522 | 60 | 90 |
| 39.07 | 0.144 | 90 | 90 |
| 39.21 | 0.657 | 90 | 90 |
| 39.87 | 0.243 | 90 | 90 |
| 40.11 | 1.782 | 90 | 90 |
| 41.89 | 0.788 | 90 | 90 |
| 42.68 |  |  |  |
| Wtd Av Posted |  |  | 86.5 |
|  |  | 90 |  |

### 6.3 Safety Performance

The SOK CMP presents collision data from an earlier safety review conducted by MoT using 1991 to 1995 collision statistics. The analysis did not identify any accident prone locations in this section but identified the north access to Okanagan Lake Park as a high severity location based on a limited sample size.

## Exhibit 6-3 Accident Histogram 1998 to 2002 - Okanagan Lake Park



The accident analysis for the current business case report uses data for 5 years from 1998 to 2002 for the Okanagan Lake Park section specifically, instead of the longer Bentley Road to Deep Creek Section. These accident statistics are also subsequent to introduction of animal fencing in the mid-1990's which has reduced animal collisions significantly, The estimated existing accident rate in this section is now $0.24 \mathrm{a} / \mathrm{mvk}$ which is below the predicted rate of $0.28 \mathrm{a} / \mathrm{mvk}$ for rural 2-lane highways but the severity appears greater than expected, confirming the previous findings. Fatal and injury accidents compose $67 \%$ of the 21 accidents between 1998 and 2002. Normal expectations for fatal and injury accidents would be $52 \%$ for this road class and intersection density. 15 of the 21 accidents between 1998 and 2002 in this segment are head-on and run-off-road accidents suggesting alignment and road width are issues.

There are 3 accesses in this segment including 2 to the Park on the east side and 1 to the office on the west side but these do not appear to be resulting in the right angle or rearend accidents normally associated with access traffic. The accesses will be improved but remain open in the proposed case. The proposed case is modeled as the equivalent of 2 intersections in order to reflect the intersection improvements.

Existing and proposed safety performance is estimated in 5 steps using a procedure developed for ICBC and augmented by more recent accident algorithms developed for

Provincial MicroBencost ${ }^{5}$ defaults from the Provincial HAS database. The steps are detailed in Appendix A and are summarized here in Exhibit 6-4

## Exhibit 6-4 Accident Rates Used for Analysis

|  | Fat | Inj | PDO | Fat \& Inj | All |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Number of Accidents (5 years) |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 2 | 12 | 7 | 14 | 21 |
| Predicted Base Case Accidents (RAU2) | 0.75 | 12.5 | 12.1 | 13.3 | 25.4 |
| Base Case Estimate used for Analysis | 0.60 | 10.5 | 10.1 | 11.1 | 21.2 |
| Predicted Accidents for Proposed Case <br> (RAU4) | 0.39 | 8.9 | 8.1 | 9.3 | 17.4 |
| Accident Reduction Potential (5 yrs) | 0.21 | 1.6 | 2.0 | 1.8 | 3.8 |
| Accident Rate (a/mvk) |  |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 0.022 | 0.13 | 0.08 | 0.16 | 0.24 |
| Predicted Base Case Accidents (RAU2) | 0.008 | 0.14 | 0.14 | 0.15 | 0.28 |
| Base Case Estimate used for Analysis | 0.007 | 0.12 | 0.11 | 0.12 | 0.24 |
| Predicted Accidents for Proposed Case (RAU4) | 0.004 | 0.10 | 0.09 | 0.10 | 0.20 |
| Accident Reduction Potential (5 yrs) | 36\% | 15\% | 20\% | 16\% | 18\% |

[^4]
### 6.4 Infrastructure

The alignment is physically constrained by the steep slopes on the west side of Lake Okanagan. Historically this section has also experienced some landslide activity but the identified locations are no longer active and the proposed project scope includes mitigation measures to improve stabilization of known slide areas.

There are no substandard bridge structures.
Pavement condition between Summerland and Peachland varies from fair to good with PCI between 6.3 and 7.5 with the majority below 7.0 (fair). No resurfacing for this section has been identified in the Ministry's Multi-Year Resurfacing Program (MYRP).

### 6.5 Summary of Deficiencies

| Criteria | Description |
| :--- | :--- |
| Safety | Higher than expected severity with run-off-road and head-on <br> accidents over represented possibly related to alignment and park <br> access. Overall accident rate is not above expected rate for 2-lane <br> highway. |
| Mobility | Presently operating at LOS 'E' . Will decline to 'F' during the <br> forecast period. There is one 70 km/hr advisory curve in this <br> section. Kaleden to Peachland average is 80 km/hr, due mostly to <br> the low speeds in the Summerland to Peachland section. The target <br> is 80 km/hr or higher. |
| Infrastructure | There are no substandard structures in this section. <br> Pavement condition varies from fair to good with the majority rated <br> fair. <br> This section has experienced landslide activity but the sites are not <br> currently active and will be mitigated in the scope of the project. |

### 6.6 Recommended Improvements

The recommended option is a 4 lane undivided $100 \mathrm{~km} / \mathrm{hr}$ design extending 4.51 km , spanning the limits of Okanagan Lake Park and Greata Ranch.. Features of the design include:

- Realigning to $100 \mathrm{~km} / \mathrm{hr}$ including the $70 \mathrm{~km} / \mathrm{hr}$ curve at the south end of the project. The cross section will be $4 \times 3.6 \mathrm{~m}$ lanes, 2.6 m flush median, 2.0 m shoulders
- Improving design of the south and north accesses to Okanagan Lake Park to provide channelization and protected acceleration and deceleration lanes for all movements, separating tuning traffic from though traffic.
- Reconstruction of about 1.2 km of park roads and frontage and allowance for future frontage roads.
- Improved access to Greata Ranch with a left-in deceleration lane a right-in slot and reconstruction of about 140 m of access road.
- Over 200m of retaining and slope retention walls
- Mitigation of potential slope instabilities

Transportation in the Okanagan Valley has been investigated through a series of previous studies which have led up to the current project.

The Okanagan Valley Transportation Plan completed in 1997 provided a comprehensive review of travel characteristics and demand forecasts throughout the Okanagan Valley and an evaluation of broader corridor strategies evaluated at a system level. These concluded with the strategy to maintain and improve mobility of the existing highway corridor. Alternative routes were examined but were not recommended.

The 2003 South Okanagan Corridor Management Plan specifically identifies capacity as an issue on Highway 97 between Summerland and Peachland and recommends 4-laning this section.

Exhibit 6-5 presents the summary of capital cost estimates. The total cost is estimated at $\$ 15.65$ million. This is slightly lower than original estimates due primarily to changes in the drainage plans, retaining walls and elimination of a proposed overpass structure. For analysis purposes, only future costs are included. Sunk costs are excluded. Future costs are $\$ 15.3$ million.

Exhibit 6-5 Cost Estimates as of November, 2003

| Summary of <br> Costs | Sunk <br> (Past) <br> Costs | $\mathbf{2 0 0 3 / 0 4}$ | $\mathbf{2 0 0 4 / 0 5}$ | $\mathbf{2 0 0 5 / 0 6}$ | Total <br> Excl. <br> Sunk <br> Costs | Total Incl. <br> Sunk <br> Costs |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Property <br> Engineering | 27 | 1,400 | 10 | 0 | 1,410 | 1,437 |
| Construction | 218 | 403 | 159 | 30 | 592 | 810 |
| Total | 332 | 2,729 | 9,350 | 3,239 | 15,318 | 15,650 |

### 6.7 Benefit Cost Results and Interpretation

At the corridor level, there is a mobility problem between Summerland and Peachland on Highway 97. The Okanagan Lake Park 4-laning project is a logical first step in addressing the overall solution. It has already been designed, enjoys local support and is the easiest segment to construct.

Results of the Benefit Cost Analysis are presented in Exhibit 6-6 followed by an interpretation of the results.

## Exhibit 6-6 Benefit Cost Analysis Highway 97 - Okanagan Lake Park 4-Laning

Discounted Costs (2003 \$millions)

| Construction | $\$ 13.59$ |
| :--- | :---: |
| Salvage Value | $\$ 3.09$ |
| Mtce \& Rehab C | $\$ 0.23$ |
| Total Discounted Cost | $\$ 10.73$ |

Discounted Benefits

| Delay Reduction | $\$ 5.9$ |
| :--- | :---: |
| VOC Reduction | $\$ 0.2$ |
| Accident Reduction | $\$ 4.3$ |
| Total Discounted Benefits | $\$ 10.4$ |


| Net Present Value (NPV) | $-\$ 0.36$ |
| :--- | :---: |
| NPV/Project Cost | -0.02 |
| B/C Ratio | $\mathbf{1 . 0}$ |
| MicroBencost File | OKLAKEPK |

The project returns a benefit cost ratio of 1.0. Ratios greater than 1.0 are deemed to be economically beneficial to society. Benefits stem mainly from time savings and reduced accident costs. Travel time is reduced as a result of increased capacity, increased design speed from 90 (with one $70 \mathrm{~km} / \mathrm{hr}$ advisory) to 100 and from a $2 \%$ shorter traveled distance. Accident cost savings stem mainly from a reduction in accident severity. There is an estimated $18 \%$ overall reduction in accidents but an estimated $36 \%$ reduction in fatal accidents. The accident statistics for this analysis segment suggest that head-on and run-off-road accidents are over-represented. The wider cross section and higher design speed will help to reduce these accidents.

The scope of the project also includes mitigation of potential landslide sites. Benefits related to landslide mitigation have not been evaluated but would reduce the potential for highway closures and property damage.

### 6.8 Sensitivity Analysis

This is intended to show the impact of alternative project assumptions on the results. In this case, is marginally positive or negative depending on the assumptions.

Exhibit 6-7 Sensitivity Analysis

|  | Sensitivity Option |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Baseline NPV | 4\% Discount Rate | $\begin{array}{\|c\|} \hline 8 \% \\ \text { Discount } \\ \text { Rate } \end{array}$ | $+25 \%$ <br> Construction Cost | $-25 \%$ <br> Construction Cost | Traffic Growth $+0.5 \%$ | Traffic Growth - $.5 \%$ |
|  | Net Present Value (millions \$) |  |  |  |  |  |  |
| NPV | -0.4 | 2.2 | -2.2 | -4.2 | 3.5 | 0.2 | -0.9 |

### 6.9 Other Alternatives

Alternative routes were considered at the system level in the 1997 Okanagan Valley Transportation Plan but were not recommended. Other options such as selective passing lanes are possible but at current traffic levels, they will only provide limited relief during peak demand periods. The investment in passing lanes (3-lane sections) may be lost if it cannot be incorporated into a future 4-lane scenarios required during the forecast period. The proposed improvement is an isolated 4 lane section and acts as a long passing lane section providing relief within the treated section and to downstream 2 lane sections. The investment in this option can also be incorporated directly into the ultimate plan for continuous 4-laning.

### 6.10 Project Timing

The project may be considered to be at it's optimum timing since it is approaching capacity, there is political and stakeholder support, this location is the easiest to construct between Peachland and Summerland and the project is designed and ready to proceed.

### 6.11 Implementation

The preferred delivery method of delivery is a conventional tender and unit price contract.

## Exhibit 6-8 Delivery Schedule

| Activity | Start | Complete |
| :--- | :--- | :--- |
| Property Purchase | November 2003 <br> ROW drawings are available now for property <br> purchase to proceed. | March 2004 |
| Tendering | February 2004 | February 2004 |
| Construction | April 2004 | May 2005 |



### 6.12 Coordination with Other Projects

All of Highway 97 between Summerland and Peachland requires upgrading from 2 lanes to 4 lanes, consistent with traffic volumes and with the 4-lane highway to the north and south. The Okanagan Lake Park Project should proceed as the first stage towards this goal. The priorities for the balance of sections between Peachland and Summerland is now under review.

Coordination with the pavement resurfacing program is not anticipated within the timeframe of this project.

### 6.13 Stakeholder Communications

Stakeholder consultations conducted as part of the SOK CMP included:
Ministry of Transportation - Steering Committee including the ADM 's of operations, planning and policy, Regional Director Southern Interior Region, Director of Planning Projects, Regional Manager Planning and Partnerships and District Highway Managers. South Okanagan District Staff were also directly involved in the process.

Local and Regional Governments - Informal meetings and workshops involving local and Regional Government administrators, Planning and Engineering Staff, Economic Development and Elected Officials

First Nations - First Nations Communities potentially affected by the CMP area were invited to meeting and workshops and were provided with copies of the reports. MoT Staff met with the Penticton Indian Band.

Other Agencies - Other key stakeholders participated through the local and Regional Government workshops and meetings. These included Ministries of Water, Land and Air Protection, Agriculture, Food and Fisheries and Municipal Affairs, RCMP Detachments, transportation companies, and business, tourism and agriculture representatives

### 6.14 Technical Risks

Technical risks include:

- Project Delivery - The highway is the only connection between Penticton and Kelowna. In order not to impact this link over 2 seasons the project is scheduled for delivery in 1 year.
- Slide Areas - Three slide areas have been identified. These are being mitigated but still present some risk from loads imposed by added fill in these areas.
- Rock Blasting - There are some properties located below bast areas which will have to be monitored for vibration or blast impacts.


### 6.15 Conclusions and Recommendations

Based on the positive results of the benefit cost analysis, the growing congestion and bottleneck effect of the Peachland to Summerland section, the support for the project and the ease of construction and readiness for this section, the Okanagan Lake Park Project should proceed as the first step in the overall improvement program.

The improvements have the potential reduce accidents by $18 \%$ and fatal accidents by $36 \%$. The added capacity will relieve congestion for the planning period and beyond and contribute to the corridor's role as a high speed north/south route consistent with long term planning objectives.

## 7 Greata Ranch to Deep Creek

### 7.1 Purpose

The purpose of this business case is to advance the project to preliminary design and Costing. No detailed design has been completed for this section.

### 7.2 Project Description and Location

This project includes upgrading 3.81 km of rural Highway 97 In the south of Peachland and north of Okanagan Lake Park (Segment 1115 LKI 42.68 to 46.49 ) from a 2 lane 90 $\mathrm{km} / \mathrm{hr}$ with 70 and $50 \mathrm{~km} / \mathrm{hr}$ speed advisories to a 4 lane $100 \mathrm{~km} / \mathrm{hr}$ standard. The general location is shown in Exhibit 7-1.

## Exhibit 7-1 Greata Ranch Section



### 7.3 Mobility

This section is part of the overall Summerland to Peachland segment identified in the SOK CMP as deficient with LOS 'F" projected for 2025. Specific to this section are low truck speeds related to grades and frequent speed advisories. The average posted speed is $84.8 \mathrm{~km} / \mathrm{hr}$. The average posted speed for the proposed case assumes some speed reduction on the approach to urban Peachland.

## Exhibit 7-2 Posted Speeds

Start Section
Brent Road Intersection
Entering Peachland Sign
70 km/h Posted Speed
Antler's Beach Access
50 km/h Speed Advisory NB
End section

| LKI | Length <br> $(\mathrm{km})$ | Existing | Proposed |
| :---: | :---: | :---: | :---: |
| 42.68 | 1.286 | 90 | 90 |
| 43.97 | 0.819 | 90 | 90 |
| 44.79 | 0.888 | 90 | 90 |
| 45.67 | 0.445 | 70 | 90 |
| 46.12 | 0.199 | 70 | 90 |
| 46.32 | 0.173 | 50 | 70 |
| 46.49 |  |  |  |
| Wtd. Avg. Posted |  | 84.8 | 89.09 |

### 7.4 Safety Performance

The existing accident rate is relatively higher than other sections due to the increasing access density and skewed intersections. The observed rate is $.42 \mathrm{a} / \mathrm{mvk}$, the expected rate with this access density is $.44 \mathrm{a} / \mathrm{mvk}$ and the rate used for analysis is $0.43 \mathrm{a} / \mathrm{mvk}$. The accident histogram shows accident clustering at the south and north entrance to Brent Road and at the entrances to Antler Beach Park.

Exhibit 7-3 Accident Histogram 1998 to 2002 - Greata Ranch Section


The SOK CMP identified these locations as accident prone based on 1991 to 1995 data. The accident analysis for the current business case report uses data for 5 years from 1998 to 2002. The estimated existing accident rate in this section is $0.43 \mathrm{a} / \mathrm{mvk}$ which is normal for a 2 lane road with this volume. There have been no fatal accidents in this section in the 5 year data period. Existing and proposed safety performance is estimated in Appendix A and summarized in Exhibit 7-4.

## Exhibit 7-4 Accident Rates Used for Analysis

|  | Fat | Inj | PDO | at \& In | All |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Number of Accidents (5 years) |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 0 | 22 | 10 | 22 | 32 |
| Predicted Base Case Accidents (RAU2) | 0.85 | 16.5 | 15.9 | 17.4 | 33.3 |
| Base Case Estimate used for Analysis | 0.91 | 15.8 | 15.3 | 16.8 | 32.0 |
| Predicted Accidents for Proposed Case (RAU4) | 0.53 | 15.2 | 13.2 | 15.7 | 29.0 |
| Accident Reduction Potential (5 yrs) | 0.38 | 0.6 | 2.0 | 1.0 | 3.1 |
| Accident Rate (a/mvk) |  |  |  |  |  |
| Observed Base Case Accidents (1998 to 2002) | 0.000 | 0.29 | 0.13 | 0.29 | 0.42 |
| Predicted Base Case Accidents (RAU2) | 0.011 | 0.22 | 0.21 | 0.23 | 0.44 |
| Base Case Estimate used for Analysis | 0.012 | 0.21 | 0.20 | 0.22 | 0.43 |
| Predicted Accidents for Proposed Case (RAU4) | 0.007 | 0.20 | 0.18 | 0.21 | 0.38 |
| Accident Reduction Potential (5 yrs) | 41\% | 4\% | 13\% | 6\% | 10\% |

### 7.5 Summary of Deficiencies

| Criteria | Description |
| :--- | :--- |
| Safety | The accident rate is relatively high compared to the rest of the <br> Summerland to Peachland corridor but is close to the expected <br> value for a 2 lane rural highway with this access density. The OK <br> CMP identified several accident prone locations, particularly at the <br> skewed Antler Beach intersections. <br> Mobility <br> Geotechnical <br> With the speed advisories, the average posted speed in this section <br> is 82 km/hr compared to a desired 80 km/hr. |
| Alignment is physically constrained by the steep slopes on the west <br> side of Lake Okanagan |  |
| No substandard structures. |  |
| Local pavement condition is good with a PCR rating close to 8. |  |

### 7.6 Option Evaluation

The preferred option is a 4 lane undivided $100 \mathrm{~km} / \mathrm{hr}$ design consistent with the overall plan to upgrade Highway 97 between Peachland and Summerland. This will provide a level of service and safety consistent with projected traffic volumes for the long term.

The existing section is 3.81 km long. The estimated project costs for this section are $\$ 13.2$ million assuming 3.77 km of new construction at $\$ 3.5$ million $/ \mathrm{km}$.

Results of the Benefit Cost Analysis are presented in Exhibit 7-5 followed by an interpretation of the results.

Exhibit 7-5 Benefit Cost Analysis Highway 97-Great Ranch Section

Discounted Costs (2003 \$millions)

| Construction | $\$ 12.454$ |
| :--- | :---: |
| Salvage Value | $\$ 2.608$ |
| Mtce \& Rehab C | $\$ 0.408$ |
| Total Discounted Cost | $\$ 10.254$ |

Discounted Benefits

| Delay Reduction | $\$ 3.947$ |
| :--- | :---: |
| VOC Reduction | $\$ 0.684$ |
| Accident Reduction | $\$ 7.404$ |
| Total Discounted Benefits | $\$ 12.035$ |


| Net Present Value (NPV) | $\$ 1.78$ |
| :--- | :---: |
| NPV/Project Cost | 0.13 |
| B/C Ratio | 1.2 |
| MicroBencost File | GREATA |

The project returns a benefit cost ratio of 1.2. Ratios greater than 1.0 are deemed to be economically beneficial to society. Benefits stem mainly first accident reduction and second from travel time savings. Travel time savings derive from improving the alignment to eliminate advisory speed zones, from increasing the capacity and from shortening the traveled distance slightly.

Accident cost savings stem from a reduction in accident rate and severity associated with the higher design standard. There is an estimated $10 \%$ overall reduction in accidents but an estimated $41 \%$ reduction in fatal accidents. The wider cross section and higher design speed will help to reduce the number and severity of accidents.

### 7.7 Sensitivity Analysis

This is intended to show the impact of alternative project assumptions on the results. In this case, NPV is negative for an $8 \%$ discount rate and $+25 \%$ construction cost.

## Exhibit 7-6 Sensitivity Analysis



### 7.8 Project Timing

The optimum timing is the implementation year that would maximize the net present value of the investment. It is measured as the year when the annual rate of return (annual benefits) exceeds the discount rate. A straight line interpolation of annual benefits suggests that this section is near its optimum timing.

Exhibit 7-7 Optimum Timing

| Discount <br> Rate | Optimum <br> Timing |
| :---: | :---: |
|  |  |
| $4 \%$ | 1989 |
| $6 \%$ | 2004 |
| $8 \%$ | 2020 |

### 7.9 Conclusions and Recommendations

The improvements have the potential to reduce accidents by $10 \%$ and fatal accidents by $41 \%$. The added capacity will relieve congestion for the planning period and beyond and contribute to the corridor's role as a high speed north/south route consistent with long term planning objectives.

Based on the positive results of the benefit cost analysis, the growing congestion and bottleneck effect of the Peachland to Summerland section, the Greata Ranch Section should proceed within the overall improvement program for Summerland to Peachland.

## 8 Summary

### 8.1 Purpose

The purpose of this business case summary is to assess all 5 segments as a whole to advance the project to subsequent design and or construction stages. At this time, detailed design has been completed for the Okanagan Lake Park section only.

### 8.2 Project Description and Location

This project includes upgrading 14.98 km of Highway 97 between Peachland and Summerland (Segment 1115 LKI 31.51 to 46.49 ) from a 2 lane $90 \mathrm{~km} / \mathrm{hr}$ with frequent speed advisories to a 4 lane $100 \mathrm{~km} / \mathrm{hr}$ standard, posted $90 \mathrm{~km} / \mathrm{hr}$.

### 8.3 Mobility

This section is part of the overall Summerland to Peachland segment identified in the SOK CMP as deficient with LOS 'F" projected for 2025. There are several sections with low truck speeds related to grades and frequent speed advisories. The average posted speed is $80.5 \mathrm{~km} / \mathrm{hr}$. The proposed improvements will increase this by $11 \%$ to $89.7 \mathrm{~km} / \mathrm{hr}$ and provide adequate capacity beyond the 25 year planning horizon.

## Exhibit 8-1 Posted Speeds

|  | 1 <br> Bentley <br> Rd to N. <br> Beach | $\mathbf{2}$ N. Beach | N.Beach to OK Park | $4$ <br> OK Park |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LKI Segment | 1115 | 1115 | 1115 | 1115 | 1115 | 1115 |
| Start | 31.51 | 34.80 | 37.00 | 38.17 | 42.68 | 31.51 |
| End | 34.80 | 37.00 | 38.17 | 42.68 | 46.49 | 46.49 |
| Length (km) | 3.29 | 2.20 | 1.17 | 4.51 | 3.81 | 14.98 |
|  | Average Posted Speed (km/hr) |  |  |  |  |  |
| Existing | 73.6 | 72.6 | 77.3 | 86.5 | 84.8 | 80.5 |
| Proposed | 90.0 | 89.3 | 90.0 | 90.0 | 89.1 | 89.7 |
| \% Increase | 22\% | 23\% | 16\% | 4\% | 5\% | 11\% |

### 8.4 Safety Performance

The existing accident rate ( $0.28 \mathrm{a} / \mathrm{mvk}$ ) is generally not above the expected rate for 2 lane rural highways ( $0.45 \mathrm{a} / \mathrm{mvk}$ ). There is an estimated $4 \%$ overall accident reduction and a 32\% fatal accident reduction. The accident histogram shows accident clustering at the south and north entrance to Brent Road and at the entrances to Antler Beach Park. Existing and proposed safety performance is summarized in Exhibit 8-3.

Exhibit 8-2 Accident Histogram 1998 to 2002 - Bentley Road to Greata Ranch


Exhibit 8-3 Accident Rates Used for Analysis

|  | Fat | Inj | PDO | Fat \& Inj | All |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of Accidents (5 years)      <br> Observed Base Case Accidents 4 53 27 57 84 <br> Predicted Base Case Accidents <br> (RAU2) 2.83 51.3 49.4 54.1 103.5 <br> Base Case Estimate used for <br> Analysis 2.39 41.6 40.2 44.0 84.2 <br> Predicted Accidents for Proposed <br> Case (RAU4) 1.62 41.9 37.3 43.5 80.8 <br> Accident Reduction Potential (5 yrs) 0.77 -0.3 2.9 0.5 3.4 <br> Observed Base Case Accidents <br> (1998 to 2002) 0.014 0.18 0.09 0.19 0.28 <br> Predicted Base Case Accidents <br> (RAU2) 0.010 0.17 0.17 0.18 0.35 <br> Base Case Estimate used for <br> Analysis 0.008 0.14 0.14 0.15 0.28 <br> Predicted Accidents for Proposed <br> Case (RAU4) 0.005 0.14 0.13 0.15 0.27 <br> Accident Reduction Potential (5 yrs) $32 \%$ $-1 \%$ $7 \%$ $1 \%$ $4 \%$ |  |  |  |  |  |

### 8.5 Summary of Deficiencies

| Criteria | Description |
| :--- | :--- |
| Safety | Accident rates are generally not above expected rates for a 2 lane <br> highway. There is a higher frequency of run-off road and head-on <br> accidents which may be the result of the frequent substandard <br> curves. Generally there is accident clustering at these curves where <br> they first appear and at major intersections. <br> Mobility <br> Geotechnical <br> With the frequent speed advisories, average posted speed in the <br> corridor is section is 80.5 km/hr compared to a desired 90 to 100 <br> for high speed rural segments. |
| Infrastructure | Alignment is physically constrained by the steep slopes on the west <br> side of Lake Okanagan. Some historical landslide sites have been <br> identified in the Okanagan Lake Park Section. |
| There are no substandard structures. <br> Local pavement condition is generally good with a PCR rating <br> close to 8. |  |

### 8.6 Option Evaluation

The preferred option is a 4 lane undivided $100 \mathrm{~km} / \mathrm{hr}$ design consistent with the overall plan to upgrade Highway 97 between Peachland and Summerland. This will provide a level of service and safety consistent with projected traffic volumes for the long term.

Overall, the 5 individual sections extend 14.98 km between Summerland and Peachland. The estimated project costs are $\$ 51.4$ million. Results of the individual Benefit Cost Analyses are presented in Exhibit 8-4 followed by an interpretation of the results.

Exhibit 8-4 Benefit Cost Analysis Highway 97 - Summary -Summerland to Peachland


The overall project returns a benefit cost ratio of 1.22. Ratios greater than 1.0 are deemed to be economically beneficial to society. Benefits stem first from time savings and second from accident savings. Travel time savings derive from improving the alignment to eliminate advisory speed zones, from increasing the capacity and from shortening the traveled distance slightly.

Accident cost savings stem from a small reduction in accident rate and a larger reduction in severity associated with the higher design standard. There is an estimated $4 \%$ overall reduction in accidents but an estimated $32 \%$ reduction in fatal accidents. The wider cross section and higher design speed will help to reduce the number and severity of accidents.

### 8.7 Sensitivity Analysis

This is intended to show the impact of alternative project assumptions on the results. In this case, NPV for the overall Summerland to Peachland Corridor becomes negative with a $25 \%$ increases in construction cost.

## Exhibit 8-5 Sensitivity Analysis

|  |  |  |  | ivity Opti |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Baseline NPV | 4\% Discount Rate | $\begin{array}{\|c\|} \hline 8 \% \\ \text { Discount } \\ \text { Rate } \end{array}$ | $+25 \%$ <br> Construction Cost | $-25 \%$ Construction Cost | Traffic Growth +0.5\% | Traffic <br> Growth - <br> $.5 \%$ |
|  |  |  | Net Prese | t Value (mi | ons \$) |  |  |
| NPV | 8.7 | 20.2 | 0.2 | -4.2 | 21.5 | 11.2 | 6.3 |

### 8.8 Conclusions and Recommendations

The overall assessment for Four-laning Highway 97 between Summerland and Peachland returns a positive $\mathrm{B} / \mathrm{C}$ ratio $=1.22$ which supports proceeding with the project as a whole in the benefit cost perspective. It can also be supported operationally since this section of highway is now operating at LOS 'E' and has been identified in previous studies as the lowest level of service on Highway 97 south of Kelowna.

Overall, the project has the potential to reduce fatal accidents by $32 \%$ and total accidents by $4 \%$. This amounts to one fatal accident every 6.5 years. Average posted speed will increase $11 \%$ from 80 to $90 \mathrm{~km} / \mathrm{hr}$. The increases in average travel speed during peak periods will be more pronounced with increases of 20 to $30 \mathrm{~km} / \mathrm{hr}$.

The comparative performance of each section is summarized in Exhibit 8-6.

Exhibit 8-6 Performance Summary by Section

|  | Bentley <br> Rd to N. <br> Beach | N. Beach | $\begin{aligned} & \text { N.Beach } \\ & \text { to OK } \\ & \text { Park } \end{aligned}$ | Okanagan <br> Lake Park | Greata Ranch to Deep Creek |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Length (km) | 3.29 | 2.20 | 1.17 | 4.51 | 3.81 |
| Substandard Curves | 2 | 2 | 1 | 1 | 2 |
| Average Posted Speed (existing) | 73.6 | 72.6 | 77.3 | 86.5 | 84.8 |
| Existing Accident Rate (a/mvk) | 0.28 | 0.25 | 0.19 | 0.24 | 0.43 |
| Number of Accidents (1998- | 18 | 11 | 4 | 87 | 32 |
| Accident Reduction Potential (5 yrs) | 0.59 | 1.56 | 0.00 | 3.75 | 4.48 |
| B/C Ratio | 1.4 | 1.6 | 1.2 | 1.0 | 1.2 |
| Project Cost (\$millions) | 11.3 | 7.5 | 4.0 | 15.3 | 13.2 |

With overall project costs estimated to be $\$ 51.5$ million, the project will have to be completed in stages. The Okanagan Lake Park does not have the highest B/C ratio but is already designed, enjoys good local support and is the most constructable in the corridor making it the logical first priority for practical reasons.

For construction purposes, the second priority should be package which includes Sections 1, 2 and 3 from Bentley Road to Okanagan Lake Park. Collectively, these sections return a benefit cost ratio of 1.4. They are adjacent sections which facilitates construction and will make continuous the 4 lane highway to the south of Bentley Road all the way to the remaining Greata Ranch to Deep Creek Section.

The third priority is the Greata Ranch to Deep Creek Section, which has a generally better existing alignment compared to Bentley Road to Okanagan Lake Park section.

The initial improvements from Okanagan Lake to Greata Ranch will provide some relief in terms of passing opportunity but continued improvement of the remaining sections should be considered in order to address safety, driver expectations and construction efficiencies. It is recommended that the project proceed to preliminary design to confirm cost estimates and review project priorities.

## Appendix A <br> Accident Rate Calculations

Existing and proposed safety performance is estimated in 5 steps using a procedure developed for ICBC and augmented by more recent accident algorithms developed for Provincial MicroBencost ${ }^{6}$ defaults from the Provincial HAS database. The steps used for each segment are summarized here.

1. The accident rate and severity are calculated for the existing highway analysis section based on observed data.
2. A second calculation is done to predict what the expected base case safety performance would be using Provincial accident prediction algorithms for the existing service class and number of intersections in the analysis section.
3. A weighted average of the predicted and observed safety performance is calculated (empirical Bayes Estimate) in order to correct for sample size and regression to the mean. This average is used to represent the base case. Severity is based on Provincial average for RAU2 instead of the observed data. This corrects for the portion of fatal accidents which has the greatest impact on benefit cost results but is often based on the least amount of data
4. The proposed case accident rate is estimated using Provincial models for RAU4 to estimate the section (mid-block) accidents and the IHSDM model to predict intersection accidents.
5. The accident reduction potential is the number and severity of accidents expected to be prevented in 5 years.
[^5]
[^0]:    1 "South Okanagan Corridor Management Plan" prepared by Urban Systems Ltd For MoT Southern Interior Region, January 2003.

[^1]:    2 "Okanagan Highway No. 97 - Okanagan Lake Park to Greata Ranch", prepared by McElhanney Engineering Ltd For MoT Southern Interior Region, MoT Project 20283-0001, October 17, 2003.

[^2]:    3 "South Okanagan Corridor Management Plan" prepared by Urban Systems Ltd For MoT Southern Interior Region, January 2003.

[^3]:    4 "Okanagan Highway No. 97 - Okanagan Lake Park to Greata Ranch", prepared by McElhanney Engineering Ltd For MoT Southern Interior Region, MoT Project 20283-0001, October 17, 2003.

[^4]:    ${ }^{5}$ Lyall P., "2003 Update for MicroBencost Default Values" prepared by Apex Engineering Limited for BC MoT, Planning and Major Projects, May 2003.

[^5]:    ${ }^{6}$ Lyall P., "2003 Update for MicroBencost Default Values" prepared by Apex Engineering Limited for BC MoT, Planning and Major Projects, May 2003.

