

March 29, 2023

ISSUED FOR USE FILE: 704-ENG.VGE004287-01

Ministry of Transportation and Infrastructure 1500 Woolridge Street – Suite 300 Coquitlam, BC V3K 0B8

Via Email: scott.b.cosman@gov.bc.ca

Attention:	Scott Cosman, P.Eng. Lead Geotechnical Engineer, Material and Standards

Subject: Chilliwack Lake Road Flood Recovery – Sandhill Slide Factual Items

Tetra Tech Canada Inc. (Tetra Tech) is providing the following factual geotechnical information with reference to the Chilliwack Lake Road Flood Recovery Sandhill Slide project:

- Borehole Records;
- Drilling and Sampling Methodology;
- Summary of Laboratory Tests and Standards; and
- Vibrating Wire Piezometer Installations

Table 1 below provides a summary of the locations and drilling depth and dates for each of the boreholes. The borehole locations are also shown on the Tender Drawings titled "Chilliwack Lake Road Flood Recovery Sandhill Slide, Grading and Paving", provided by Stantec Consulting Ltd.

Table 1: Borehole Location, Elevation, Depth and Date Summary

Borehole ID	Easting ^{1,2} (m)	Northing ^{1,2} (m)	Collar Elevation ² (masl)	Depth (m)	Date Drilled
BH22-01	594997.6	5437158.9	282.6	48.8	July 4, 2022 – July 7, 2022
BH22-02	594971.5	5437222.1	255.8	20.4	July 8, 2022
BH22-03	594898.7	5437110.0	269.8	15.9	December 13, 2022
BH22-04	594884.5	5437115.3	261.6	12.8	December 14, 2022
BH22-05	594856.3	5437083.6	258.5	9.1	December 14, 2022

(1) Coordinates are in UTM NAD83 Zone 10

(2) Coordinates and collar elevations were provided by Van Bower Construction Services Ltd.

Boreholes were advanced using sonic drill rigs, equipped with a 108 mm inner diameter core barrel. Boreholes BH22-01 and BH22-02 were advanced using a Boart Longyear LS600 drill rig, and boreholes BH22-03, BH22-04 and BH22-05 were advanced using a Boart Longyear LS250 drill rig.

Standard Penetration Tests (SPT) were carried out at various intervals in all the boreholes, as indicated on the attached logs. SPT split spoon samples were collected using a AWJ (to 15.2 m depth) and NWJ (below 15.2 m) sampling rods connected to a 38.1 mm (inner) diameter split spoon.

All boreholes were backfilled with cement bentonite grout in accordance with the Province of British Columbia's Groundwater Protection Regulation.

Table 2 summarizes the types of laboratory tests undertaken as part of the drilling program, and the associated ASTM standard. The testing results are provided on the borehole logs.

Table 2: Summary of Laboratory Tests

Laboratory Test	ASTM Standard
Water (Moisture) Content of Soil and Rock by Mass	D2216
Amount of Material Finer than 75-µm (No. 200) Sieve in Soils	D1140
Particle-Size Distribution (Gradation) of Soils	D6913
Liquid Limit, Plastic Limit, and Plasticity Index of Soils	D4318

Vibrating Wire Piezometers (VWP) supplied by RST Instruments Ltd. were installed in boreholes BH22-01 and BH22-02. The approximate depth of installation of each VWPs, along with the associated serial number and borehole in which the VWP was installed, is provided in Table 3 below.

Table 3: Summary of VWP Installations

Serial Number	Approximate Installation Depth	Installation Borehole ID
VW148119	7.6 m	BH22-01
VW148172	24.1 m	BH22-01
VW148105	19.5 m	BH22-02

CHILLIWACK LAKE ROAD FLOOD RECOVERY – SANDHILL SLIDE FILE: 704-ENG.VGE004287-01 | MARCH 29, 2023 | ISSUED FOR USE

We trust this document meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted, Tetra Tech Canada Inc. SHEPPARD # 48807 Mai 29,2023 -01 BAITISH 11 22 1 GEO04287-01 VGINEE 704-ENG.VGEO04287-01 FILE: Prepared by: 6 Reviewed by: Amanda Sheppard, M.Eng., P.Eng. Ryan Clare, P.Eng., PMP Geotechnical Engineer Senior Geotechnical Engineer Direct Line: 236.987.9394 Direct Line: 606.812.7865 Amanda.Sheppard@tetratech.com Ryan.Clare@tetratech.com

/sy

Attachments: Appendix A – Tetra Tech's Limitations on the use of this Document Appendix B – Borehole Logs

> PERMIT TO PRACTICE TETRA TECH CANADA INC.

> PERMIT NUMBER: 1001972

LET_Chilliwack_Lake_Road_factual memo (29Mar_2023).docx

ATTACHMENT A

TETRA TECH'S LIMITATIONS ON THE USE OF THIS DOCUMENT



GEOTECHNICAL

1.1 USE OF DOCUMENT AND OWNERSHIP

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Both electronic file and/or hard copy versions of TETRA TECH's Instruments of Professional Service shall not, under any circumstances, be altered by any party except TETRA TECH. TETRA TECH's Instruments of Professional Service will be used only and exactly as submitted by TETRA TECH.

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Services performed by TETRA TECH for the Professional Document have been conducted in accordance with the Contract, in a manner consistent with the level of skill ordinarily exercised by members of the profession currently practicing under similar conditions in the jurisdiction in which the services are provided. Professional judgment has been applied in developing the conclusions and/or recommendations provided in this Professional Document. No warranty or guarantee, express or implied, is made concerning the test results, comments, recommendations, or any other portion of the Professional Document.

If any error or omission is detected by the Client or an Authorized Party, the error or omission must be immediately brought to the attention of TETRA TECH.

1.4 DISCLOSURE OF INFORMATION BY CLIENT

The Client acknowledges that it has fully cooperated with TETRA TECH with respect to the provision of all available information on the past, present, and proposed conditions on the site, including historical information respecting the use of the site. The Client further acknowledges that in order for TETRA TECH to properly provide the services contracted for in the Contract, TETRA TECH has relied upon the Client with respect to both the full disclosure and accuracy of any such information.

1.5 INFORMATION PROVIDED TO TETRA TECH BY OTHERS

During the performance of the work and the preparation of this Professional Document, TETRA TECH may have relied on information provided by third parties other than the Client.

While TETRA TECH endeavours to verify the accuracy of such information, TETRA TECH accepts no responsibility for the accuracy or the reliability of such information even where inaccurate or unreliable information impacts any recommendations, design or other deliverables and causes the Client or an Authorized Party loss or damage.

1.6 GENERAL LIMITATIONS OF DOCUMENT

This Professional Document is based solely on the conditions presented and the data available to TETRA TECH at the time the data were collected in the field or gathered from available databases.

The Client, and any Authorized Party, acknowledges that the Professional Document is based on limited data and that the conclusions, opinions, and recommendations contained in the Professional Document are the result of the application of professional judgment to such limited data.

The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this document, at or on the development proposed as of the date of the Professional Document requires a supplementary exploration, investigation, and assessment.

TETRA TECH is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.



1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, TETRA TECH has not been retained to explore, address or consider and has not explored, addressed or considered any environmental or regulatory issues associated with development on the subject site.

1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems, methods and standards employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historical environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional exploration and review may be necessary.

1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

Construction activity can impact structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques, and construction sequence are known.

1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, and the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

1.15 DRAINAGE SYSTEMS

Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function. Where temporary or permanent drainage systems are installed within or around a structure, these systems must protect the structure from loss of ground due to mechanisms such as internal erosion and must be designed so as to assure continued satisfactory performance of the drains. Specific design details regarding the geotechnical aspects of such systems (e.g. bedding material, surrounding soil, soil cover, geotextile type) should be reviewed by the geotechnical engineer to confirm the performance of the system is consistent with the conditions used in the geotechnical design.

1.16 DESIGN PARAMETERS

Bearing capacities for Limit States or Allowable Stress Design, strength/stiffness properties and similar geotechnical design parameters quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition used in this report. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions considered in this report in fact exist at the site.

1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.

1.18 APPLICABLE CODES, STANDARDS, GUIDELINES & BEST PRACTICE

This document has been prepared based on the applicable codes, standards, guidelines or best practice as identified in the report. Some mandated codes, standards and guidelines (such as ASTM, AASHTO Bridge Design/Construction Codes, Canadian Highway Bridge Design Code, National/Provincial Building Codes) are routinely updated and corrections made. TETRA TECH cannot predict nor be held liable for any such future changes, amendments, errors or omissions in these documents that may have a bearing on the assessment, design or analyses included in this report.

ATTACHMENT B

BOREHOLE LOGS



			COLUMBIA	Pr	ojeo	ct: Chi	illiwa	ck La	ake R	load	- Sandhi	II Slide	;		Project	No: 704-E	NG.VGEO)4287-()1	
			Ministry of Transportation	Lc	cati	ion: Cl	hilliw	ack I	_ake	Roa	d				Ground	Elev: 282	.591 m			
			& Infrastructure	Cł	nilliv	vack, E									UTM: 5	94997.581	E; 543715	8.929 N	N; Z 10) nac
(m)	Method	Diameter (mm)	Soil	Graphical Representation	Sample Type	Sample Number		Distri	le Siz butior Silt Clay	n : &		eld Blo ows/3			Post-I	>	Peak	- VW148119	/W149172	Elevation
	Me	Core	Description	Graphical Re	Samp	Sample	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	□ S 20	PT 40	60	80	10 Plastic Limit 20		e Liquid t Limit	- M	VW1	Elev
1			ASPHALT (50 mm) SP - SAND (FILL), some gravel, poorly graded, dry, brown; medium to coarse sand; fine gravel.			S1														28
2			 GW - GRAVEL (FILL), some sand, well graded, dry, grey; fine to coarse sand; fine to coarse gravel, angular to subangular, maximum size 25 mm diameter. SB - COBBLES, trace gravel, trace sand; subangular to subrounded cobbles, maximum size 100 mm diameter. 	000 000 000		S2 SPT01														28
5			SP - SAND, trace gravel, word debris, poorly graded, moist, very dense, brown to grey; fine to medium sand. \$PT01 - 16/24/19/24 SM - SAND, silty, some to trace gravel, trace cobbles, well graded, moist, very dense, brown			SPT01 S3 SPT02	12	71	1	7					•					27
;			to grey; fine to coarse sand; fine gravel to coarse gravel, subangular to subrounded, maximum size 75 mm diameter. SPT02 - 12/34/23/24 SPT03 - 25/30/43/50 - (SPT Refusal below 45 cm			S4 S5												· · · · · · · · · · · · · · · · · · ·		2
2022-Jul-04			penetration) SM - SAND, silty, some gravel, trace cobbles, loose, wet, brown and grey; fine to medium sand; fine to coarse gravel, subrounded to rounded, maximum size 38 mm diameter.			SPT03 S6 S7	5								•				0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5
	Sonic		CL - CLAY, trace gravel, trace sand, occasional silt seams 25 mm thick, occasional wood pieces, w>PL, soft, medium to high plastic, brown to grey. SPT04 - 0/1/1/1		\square	S8 SPT04 S9	0	1	99	9			-			•				2
			- grey below 7.6 m SPT05 - 0/1/2/7		X	SPT05 S10	5		86	.1				· · · · · ·	F	• 1		0.0000		2
0			ML - SILT and SAND, moist, compact, non-plastic, brown to grey; fine sand. SPT06 - 1/8/21/25 SW-SM/GW-GM - SAND, gravelly, silty, to SAND		XX XX	S11 SPT06/ SPT06E S12		39	6	1	C]			•				~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	2
1			and GRAVEL, silty, well graded, wet, very dense, grey; fine to coarse sand; fine gravel subangular to subrounded, maximum size 50 mm diameter. SPT07 - SPT Refusal (50 blows in 10 cm)		X	S13 SPT07	26	47	2	7					•					2
3			SP/GP - SAND and GRAVEL, some fines, some cobbles, poorly graded, moist, very dense, brown to grey; fine to medium sand; fine to			S14 S15 SPT08	49	38	1:	3			-	· · · · · · · · · · · · · · · · · · ·	•			,		2
4			coarse gravel, subangular to rounded, maximum size 63 mm diameter. - boulder at 13.1 m (700 mm diameter) SPT08 - SPT Refusal (50 blows in 2.5 cm) - cobble at 13.9 m (200 mm diameter)			S16 SPT09/	39 4	49	1:	2			-		•					20
15			· ·	<u>a:</u> 0 C	ontra	actor:	Four	ndex							Comple	tion Depth	: 48.8 m	1.0		
		٦	TETRA TECH							ngy	ear LS 60	00				ite: July 4,				
	1-					ed By:	-								-	tion Date:		_		

			BRITISH COLUMBIA	B	ore	eh	0	e	Ν	o: BH22-0	1		
			COLUMBIA	Proj	ect: Ch	illiwa	ck La	ake R	load	- Sandhill Slide	Project No: 704-ENG.VGEO0	4287-01	
			Ministry of Transportation	Loc	ation: C	Chilliw	ack	Lake	Roa	d	Ground Elev: 282.591 m		
			& Infrastructure	Chil	liwack,	B.C.					UTM: 594997.581 E; 5437158	3.929 N; Z 10) NAD83
Uepth Depth 15	Method	Core Diameter (mm)		Graph	Sample Number	Gravel (%)	<u>Distri</u>	le Siz butior Silt Clay (%) tils	n : &	Field Blowcount (blows/300 mm) □ SPT 20 40 60 80	Field Vane (kPa) Post-Peak Peak 10 20 30 40 Plastic Moisture Liquid Limit Content Limit 20 40 60 80	VW148119 VW149172	Elevation (m)
E			SP/GP - SAND and GRAVEL, some fines, some cobbles, poorly graded, moist, very dense,	<u>3</u> 92	<\$PT09)B						0.000	
16 17 17			brown to grey; fine to medium sand; fine to coarse gravel, subangular to rounded, maximum size 63 mm diameter. SPT09 - 6/28/50 - (SPT Refusal below 30 cm penetration) \$PT10 - 21/25/15/11 SP-SM/ML - SAND, some silt, to SAND and SILT,		\$PT10 \$PT10 \$PT10 \$PT10 \$18)A)B							267-
18			trace cobbles, poorly graded, wet, compact, brown to grey; fine to medium sand; non-plastic fines.		S19			40	.3		•		265
19			SPT11 - 2/3/10/12		S20 SPT1 S21	1							263
20			SPT12 - 3/6/8/9		S22 SPT12	24							262-
22	lic	2	- sand becomes fine below 21.2 m	V	SPT12 S23								261
23	Sonic				S24						•		260
24		100	SPT13 - 10/7/9/14 CL - CLAY, silty, w>PL, stiff to very stiff, low to medium plastic, grey.		SPT1 S25								258
20					S26			98	.6				257
27			SPT14 - 5/5/8/8		SPT1 S27	4					•		256
28			- fine sand at 28.6 m		S28								255-
					tractor						Completion Depth: 48.8 m		
			TETRA TECH	Equ	ipment	Туре	e: Bo	art Lo	ongy	ear LS 600	Start Date: July 4, 2022		
	J			Log	ged By	: AL					Completion Date: July 7, 2022	2	
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			BRITISH COLUMBIA	E	30	ore	eh	0	е	Ν	o: BH22-0	1					
				Pr	oje	ct: Chi	lliwa	ck La	ake R	Road	- Sandhill Slide	Project N	lo: 704-ENC	G.VGEO0)4287-()1	
			Ministry of Transportation & Infrastructure	Lc	cat	ion: Cł	hilliw	ack l	Lake	Roa	d		Elev: 282.59				
				Cł	nilliv	vack, E						UTM: 59	4997.581 E	; 543715	8.929 1	N; Z 10) NAD8
00 Depth (m)	Method	Core Diameter (mm)	Soil Description	Graphical Representation	Sample Type	Sample Number		<u>Distril</u>	le Siz butior Silt Clay (%) tils	n t&	Field Blowcount (blows/300 mm) □ SPT 20 40 60 80	Post-P ↔ 10	eld Vane (kł eak 20 30 Moisture Content 40 60	Pa) Peak 40 Liquid Limit 80	VW148119	VW149172	Elevation (m)
			SPT15 - 6/12/17/19		\bigtriangledown	SPT15								:			
- 31			CL - CLAY, occasional lenses of fine sand, w>PL, very stiff to hard, low to medium plastic, grey.			S29											252-
- 32						S30			99	.5		F	• 1				251-
- 33			SPT16 - 6/7/8/20		Х	SPT16									· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	249-
- 34						S31									0,00,00,00,00,00,00,00,00,00,00,00,00,0		248-
- 35 - 36			SPT17 - 37/23/24/32			S32									0 0 0 0		247-
- 37	v		- fine sand, silty at 36.6 m		Х	SPT17	,								0 0 0 0		246-
- 38	Sonic					S33 S34			99	3			•		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		
- 39			SPT18 - 4/6/7/8		X	SPT18							-		0 0 0 0 0		244
- 40																0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,	
- 41 - 42			- fine sand at 41.1 m - fine sand at 41.8 m			S35									0 0 0 0		241
- 43			SPT19 - 4/7/8/16 - fine sand at 42.7 m		X	SPT19 S36											240
- 44																	239
						S37											238
45					ontr	actor:	Four	ı ıdex			. : : :	Completi	ion Depth: 4	8.8 m	r 0. ~	· · · ~	1
		٦	TETRATECH							onav	ear LS 600		e: July 4, 20				
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			BRITISH	E	30	ore	eh	0	е	Ν	lo: BH22 -	01	
			COLUMBIA	Pr	ojeo	ct: Chi	lliwa	ck La	ake F	Road	- Sandhill Slide	Project No: 704-ENG.VGEO04287-01	
			Ministry of Transportation & Infrastructure	Lo	cati	ion: Cł	nilliw	ack I	Lake	Roa	d	Ground Elev: 282.591 m	
			& milastructure	Cł	nilliv	vack, E						UTM: 594997.581 E; 5437158.929 N; Z 10	NAD8
45 (m)	Method	Core Diameter (mm)	Soil Description	Graphical Representation	Sample Type	Sample Number		Sand (%)	butio Si Clay	on It & y (%)	Field Blowcount (blows/300 mm) SPT 20 40 60 8	Limit Content Limit	Elevation (m)
- - - - - - 46			CL - CLAY, occasional lenses of fine sand, w>PL, very stiff to hard, low to medium plastic, grey. SPT20 - 1/3/9/12		X	SPT20 S38							237-
47	Sonic					S39							236
48			SPT21 - 3/4/8/22		$\overline{\vee}$	SPT21							235-
49			 fine sand at 48.5 m END OF BOREHOLE AT 48.8 m DEPTH Soil descriptions are based on visual classifications, field observations and testing, drill performance, and laboratory testing. Some 		\square								234
50 			 variation through the interpreted soil layers is expected. Soil consistency and density descriptions based on SPT blow counts where available, and are 										232-
51			 otherwise based on drill performance and visual observation. SPT blow counts shown on logs are uncorrected field N-values. SPTs were carried out using an automatic trip hammer and AWJ (to 15.2 m) 										231-
52			 and NWJ (below 15.2 m) rods. Upon completion of drilling two vibrating wire piezometers (VW148119 and VW148172) were installed at this location to a depth of 7.6 										230-
53			m and 24.1 m. - Upon completion the borehole was tremie grouted to surface with cement/bentonite grout. The hole was finished at the surface with a flush mount cover.										229-
54			 UTM coordinates and elevations were collected by Van Bower Construction Services Ltd. 										228-
50													227-
57													226-
58													225-
- - - - 59													224
- - - 60													223-
				-		actor:						Completion Depth: 48.8 m	
7			TETRA TECH				• •	: B0	art Lo	ongy	ear LS 600	Start Date: July 4, 2022 Completion Date: July 7, 2022	
Ľ						ed By: wed B		S				Page 4 of 4	

			BRITISH	Bo	re	eho	ble	9	N	0:	BH	2	2-()2							
			COLUMBIA	Project	: Chi	illiwack	Lak	e Ro	oad -	Sand	hill Slide			Pro	ect No:	704	-ENG.	VGEO	004287-0)1	
			Ministry of Transportation	Locatio	n: C	hilliwad	ck La	ake F	Road					Gro	und Ele	ev: 25	55.785	m			
			& Infrastructure	Chilliwa	ack,	B.C.								UTI	N: 5949	71.5	26 E;	54372	222.075	N; Z 10	NAD83
o Depth (m)	Method	Core Diameter (mm)		Graphical Representation	Sample Type	Sample Number		Partic Distrii (%) Sand (%)	butio Si Clav			ows/3	owco 300 m 60		Pla Lir	Fie Dost-Pe ↓ 10 Istic mit 20	eld Var _{lak} 20 Moisi Cont 40	30 ture	Pa) ⊃eak ◆ 40 Liquid Limit √ 80	VW148105	Elevation (m)
Ē			SW-SM - SAND (SLIDE DEBRIS), gravelly, silty, occasional wood pieces, well graded, moist, comp	act 🕅	X															••••••••••••••••••••••••••••••••••••••	
1			brown; fine to coarse sand; angular to subrounded gravel, maximum size 50 mm diameter. SPT01 - 3/6/7/5			SPT01	20	66	1	4					•						255
F			WOOD DEBRIS (SLIDE DEBRIS)		X																
3			SPT02 - SPT Refusal (50 blows in 15 cm) SAND and WOOD DEBRIS (SLIDE DEBRIS); moist, grey; fine to medium sand. WOOD DEBRIS (SLIDE DEBRIS)			SPT02 S2									 P	•					253
5			SW - SAND (SAND DEBRIS), trace to some gravel, trace fines, well graded, moist, loose, brown; fine t coarse sand; fine gravel. \$PT03 - 5/6/3/4	∘		SPT03	1				. 🗆		- - - - - - - - - - - - - - - - - - -								251-
10022-Jul-08 ¹ → 9			WOOD DEBRIS and SAND (SAND DEBRIS); fine to medium sand. SW - SAND (SLIDE DEBRIS), trace gravel, trace fine well graded, moist, compact, brown to grey; fine to medium sand; fine to coarse gravel subangular to subrounded, maximum size 50 mm diameter.	s,		SPT04															250 99 80-141 1111111111111111111111111111111111
⊽ 1111118 9	Sonic		\$PT04 - 4/7/4/5 SW - SAND, trace gravel, well graded, moist to wet, compact, reddish brown to tan; fine to medium sar fine to coarse gravel up to 25 mm diameter. SPT05 - 6/10/6/8] [S4 SPT05 S5	4	92		4			- - - - - - - - - - - - - - - - - - -		•	· · · · · · · · · · · · · · · · · · ·					248
10		100	GP - GRAVEL, sandy, trace cobbles, moist, very dem brown to grey; coarse gravel, subrounded to round up to 75 mm diameter; fine to coarse sand. SPT06 - SPT Refusal (50 blows in 5 cm) - No Recovery on SPT-06 at 9.1 m - cobble at 9.8 m			SPT06	j									· · · · · · · · · · · · · · · · · · ·					246
11			SPT07 - 34/23/22/18 NO RECOVERY			SPT07	,					•••								0, 0, 0, 0 0, 0, 0	245
12																· · · · · · · · · · · · · · · · · · ·				0 0 0	244
13 			SW - SAND, trace to some gravel, well graded, wet, loose to compact, grey-brown; fine to coarse sand fine to coarse gravel, subrounded to rounded up to mm diameter. SPT08 - 1/2/10/14		X	SPT08									••••••						243
	_			Contra	ctor:	Found	ex							Cor	npletion	n Dep	oth: 20	.4 m			
		_1	TETRA TECH	Equipn	nent	Type: I	Boar	t Lor	ngyea	ar LS	600			Sta	t Date:	July	8, 202	22			
	J			Logged	d By:	AL								Cor	npletion	Dat	e: July	/ 8, 20)22		
			,	Review	/ed E	By: AS							-	Pag	e 1 of 2	2					

			BRITISH	Bc	re	eho	ole	e	N	D:	BH	22	2-0	2					
			COLUMBIA	Project	: Chi	lliwack	Lak	æ Ro	oad -	Sano	hill Slide			Projec	ct No: 704	-ENG.VC	GEO04287-	01	
			Ministry of Transportation & Infrastructure	Locatio	on: C	hilliwa	ck La	ake F	Road					Grour	nd Elev: 2	55.785 m			
			& milastructure	Chilliw	ack,	B.C.								UTM:	594971.5	626 E; 54	37222.075	N; Z 10	NAD83
Depth (m)	Method	Core Diameter (mm)	Soil Description	Graphical Representation	Sample Type	Sample Number		<u>Distri</u>	Silt (%) Sil	n t&		eld Blov ows/30 PT			Fi Post-Pi 今 10 Plastic Limit	eld Vane eak 20 3 Moisture Conten	Peak 0 40 e Liquid	VW148105	Elevation (m)
15			CIM CAND trace to come group well graded wat	0						_	20	40	60	80	20	40 6	0 80		
 16			SW - SAND, trace to some gravel, well graded, wet, loose to compact, grey-brown; fine to coarse sand fine to coarse gravel, subrounded to rounded up to mm diameter.	75		S7													240
17	.0		SPT09 - 1/2/7/8			S8 SPT09)								•			o , o , o , o 0 , 0 , 0	239
18	Sonic																	, , , , , , , , , , , , , , , , , , ,	238-
19			- some gravel to gravelly beow 19.2 m			S9 S10												, o, c, o, (237-
20			CL - CLAY, silty, trace sand, w>PL, hard, low to medi plastic, brown to grey. SPT10 - 1/21/12/18 becomes grey below 20.3 m	um		SPT10, SPT10,										•			236
21			END OF BOREHOLE AT 20.4 m DEPTH - Soil descriptions are based on visual classifications, field observations and testing, drill performance, and laboratory testing. Some variation through the	nd															235
22			 interpreted soil layers is expected. Soil consistency and density descriptions based on SPT blow counts where available, and are otherwi based on drill performance and visual observation. 																234
23			 SPT blow counts shown on logs are uncorrected fiel N-values. SPTs were carried out using an automat trip hammer and AWJ (to 15.2 m) and NWJ (below 15.2 m) rods. 	ic															233-
24			 Upon completion of drilling one vibrating wire piezometer (VW148105) was installed at this locat to a depth of 19.5 m. Upon completion the borehole was treme grouted to 																232
25			surface with cement/bentonite grout. The hole was finished at the surface with a flush mount cover - UTM coordinates and elevations were collected by N Bower Construction Services Ltd.																231-
26																			230-
27																			229
- 28																			228
29																			227-
- 30														1					226-
				Contra											letion De		m		
			TETRA TECH	Equipn			Boar	t Lor	ngyea	ar LS	600				Date: July		0000		
				Logge											letion Da	te: July 8,	2022		
				Review	/ed E	sy: AS								Page	Z 01 2				

			BRITISH	Bor	eł	າດ	ble	N	١c):	В	Hź	22-	03						
			COLUMBIA	Project: C	hilliw	ack	Lake	Roa	d - S	andł	nill Sl	ide - F	hase II	Pro	ject No:	704-E	ENG.VO	GEO0428	37-01	
			Ministry of Transportation	Location:											, ound Ele					
			& Infrastructure	Chilliwacł	к, В.С									UT	M: 5948	98.66	1 E; 54;	37110.0 ⁻	12 N; Z 10) NAD83
		m)					<u>ب</u>		artic Distri	butio	n									
Depth (m)	Method	Diameter (mm)	Soil		Graphical Representation	Sample Type	Sample Number	(%	(9		lt & / (%)		Field E (blows			Р	ost-Peak ↔		Peak	Elevation (m)
De	Met		Description		al Re	ample	nple	Gravel (%)	Sand (%)		(%)		SPT					0 30	40	Elev:
		Core			phic	ő	Sar	Gra	Sal	Silt (%)	Clay (%)							loisture Content	Liquid Limit	
0					Gra					ŝ	ö	2	0 40	60	80		- I	0 60	- I 80	
			SW/GW - SAND and GRAVEL (FILL), fine to coarse,	gravel up	\boxtimes														:	
Ę			to 38 mm, well graded, trace silt, brown, non-cohe moist.	sive,	\bigotimes		G-01													
Ē			SM - SAND, silty, fine to coarse, well graded, some fi	ine to																
			coarse subrounded gravel up to 64mm, brown, no	n-cohesive,			G-02	7	79		 4		· · · · · · · · · · · · · · · · · · ·							269-
- '			moist, loose.				G-02	1	/9					-	÷		-		÷	
-							04 TO							-			-			-
È			SPT01 - 5/4/4/7			\geq	SPT-01							-			-			
- 2													: 							268-
_														-					÷	-
-							G-03										-			-
Ę			SW - SAND, gravelly, fine to coarse, fine to coarse su to angular gravel, well graded, trace fines, trace file	ubrounded	م ب ب		G-04										÷		÷	267-
- 3			organics (wood), occassional cobbles up to 76 mn		0.0 00(· · · · · ·			· · · · · · · · · · · · · · · · · · ·				· · · · · · · · · · · · · · · · · · ·	-
-			non-cohesive, moist, compact. SPT02 - 11/12/8/7			4	SPT-02					L	-				-			-
Ē			- no recovery in SPT-02; wood stuck in base of SPT t	tip	$\mathcal{O}_{\mathcal{O}}$									-			-		÷	
E			- wet throughout third sonic run (3.0 m-4.3 m)		0.0															266-
- 4							G-05						· · · · · · · · ·	•••••	••••		•		· · · ·	-
Ē			SW - SAND, gravelly to some gravel, fine to coarse, v	well			0.00	200	74		9		· · ·							-
-			graded, fine to coarse subrounded to subangular f elongated gravel, brown to orange-brown, non-coh	lat and			G-06 SPT-03		71		9			-					÷	
Ē	je.		moist to wet, loose to compact.	100170,										-			-			265-
- 5 -	Sonic		SPT03 - 10/5/3/4 - no recovery in fourth sonic run (4.6 m to 6.1 m); mai	torial type	0.0								:						·····	-
F			inferred based on drilling and sample collected ab		ە م								· · ·							
-			below this depth.		0.0 0.0												-			
- 6																				264-
-			SPT04 - 6/7/7/6			\mathbf{i}	G-07							-					:	
Ē					0.0		SPT-04										-			
Ē					\circ												-			
- 7			- approximately 75 mm thick black sand layer at 6.9 r	n	000															263-
-				_	9 ()° 20-0		G-08	20	71		9									-
-			- grey coarse sand to fine gravel seams from 7.3 to 7	.5 m.	$\circ \bigcirc \bigcirc$								· · ·				-		÷	
Ē			SPT05 - 15/11/5/6		000 0	\boxtimes	SPT-05													262-
8					$\dot{\odot}$:		••••				•••••	-
-					0.0		G-09							-			÷			
F			- grey from approximately 8.5 to 8.8 m.				G-10													
Ē			SM - SAND, silty, fine to coarse, well graded, trace g	ravol	bQ.(LTP															261-
— 9 -			orange, non-cohesive, wet, compact.	idvei,			G-11						· · · · · · · · · · · · ·							
Ē			SPT06 - 3/4/14/25				SPT-06						1 : : :				÷		:	
Ē																			:	
E E 10																	÷		:	260-
		_		Contracto	or: Vai	nMa	ars	•	•	•	•			Co	mpletion	n Dept	h: 15.8	m		
			TETRA TECH	Equipmer	nt Typ	e: E	Boart L	ong	year	LS2	50			Sta	irt Date:	2022	Decem	ber 13		
	J			Logged B	y: ST									Co	mpletion	n Date	: 2022 [Decembe	er 13	
			,	Reviewed	By: A	٩S				_				Pa	ge 1 of 2	2				

			BRITISH COLUMBIA	Bor Project: 0											1	ect No:	704 ⊑			14.28.	7 01	
			Ministry of Transportation	Location:						anui	1111 31	liue - r	Tidse	; 11		und Ele				J4Z0	-01	
			& Infrastructure	Chilliwac				e nu	Jau											0.01	2 N; Z 1	Ο ΝΔΙ
		(mm)			Graphical Representation		her		Partic Distri	butic Si	on It &	-			wcou	nt		Fie	ld Van	e (kP	a)	
(m)	Method	Diameter	Soil		Repres	Sample Type	Sample Number	(%)	(%)	Clay	y (%)		`		00 mi	n)	-	ost-Pe ↔ 10	-	F 30	Peak ◆ 40	Elevation
		Core Dia	Description		iical F	Samp	ampl	Gravel (%)	Sand (%)	(%	Clay (%)] SP	Т			-	-	Moist		Liquid	Ē
0		ပိ			Graph			0		Silt (%)	Clay	2	20 4	40	60	80		nit ┣ 20	Conte	ent 60	Limit - I 80	
			SW/GW - SAND and GRAVEL, fine to coarse, well of trace silt, orange-brown, non-cohesive, moist.	- /			G-12 G-13				4.5							-	н			
			CL - SILTY CLAY, low to medium plasticity, grey, co w~PL.	/			G-14			4	+.5											
1			SP/ML - SILT and SAND, low-plasticity, fine sand, s trace fine to coarse subangular gravel up to 38 m with dark bands, non-cohesive, moist, loose to cc - cobbles at 10.6 m depth. SPT07 - 6/5/5/5	m, orange		X	SPT-07						· · · · · ·					· · · ·		-		2
																						2
2			CH - CLAY, medium to high plasticity, trace gravel a grey, cohesive, w~PL, firm to very stiff. SPT08 - 21/12/9/8	ind cobbles,			G-15 SPT-08	3														
3	Sonic												•	· · · ·	-							2
			SPT09 - 4/3/3/4				G-16 SPT-09						•					F	•	1		2
4							G-17											÷				
5		-	SW - SAND, fine to coarse, some gravel, well grade non-cohesive, wet to moist, compact.	d, grey,			SPT-10						· · · · ·					· · · · · · · · · · · · · · · · · · ·				2
			SPT10 - 9/10/4/3																			2
6			END OF BOREHOLE at 15.9 m DEPTH																			
			 Soil descriptions are based on visual classifications observations and testing, drill performance, and la testing. Some variation through the interpreted so expected. 	aboratory																		
7			 Soil consistency and density descriptions based on counts where available, and are otherwise based performance and visual observation. SPT blow counts shown on logs are uncorrected fit N-values. SPTs were carried out using an automa 	on drill eld																		2
8			 Where can be out using an automatic hammer and AWJ rods. Upon completion the borehole was tremie grouted with cement/bentonite grout. The hole was finisher surface with sand. UTM coordinates and elevations were collected by Construction Services Ltd. 	to surface ed at the																		2
9																						2
0																						2
_				Contracto	or: Va	anMa	ars								Con	npletion	n Depth	n: 15.	.8 m			
			TETRA TECH	Equipme	nt Ty	pe: E	Boart L	ong	year	LS2	50					t Date:						
		- I		Logged E	N/- CT	г									Con	nlation	Data.	202	2 Dece	mbe	· 13	

		BRITISH COLUMBIA	Bor Project: (Project No: 704-ENG.VGEO04287-01									
		Ministry of Transportation & Infrastructure	Chilliwa	ack Lał	ke Ro	bad			Gro	ound Ele	ev: 261.6)2 m								
		& minastructure	Chilliwac	k, B.C.								UT	M: 5948	84.539 E	; 5437115.29	98 N; Z 1	0 NAE			
	Method Diameter (mm)			Graphical Representation	Sample Number	[Partic <u>Distri</u>	butic Si			ield B			Post-	Ę					
<u>ا</u>	Method Diameter	Soil		Repr		(%)	(%)	<u> </u>		,		000 11		10	20 30	→ 40	Elevation			
:				ical	amp	Gravel (%)	Sand (%)	9	(%)		SPT			Plasti	c Moisture	Liquid	Ē			
	COP C			raph	S	0		Silt (%)	Clay					Limit	Content	Limit				
					_	_				20	40	60	80	20	40 60	80	_			
		TOPSOIL SW - SAND, gravelly, silty, fine to coarse, fine to co to subangular gravel up to 75 mm, trace organics brown, non-cohesive, moist, compact.	arse angular s (rootlets),														26			
				G-01												260-				
		SPT01 - 6/8/8/8		SPT-0	1															
																	200			
		- some gravel and silt, higher moisture, and lighter b below 2.0 m.	prown colour																	
					G-02	6	86		8						Ð		25			
		- grey gravel layer from approximately 2.8 m to 2.9 r	m.		G-03															
		\- inferred boulder encountered at 3.0 m based on dr	ill response.		SPT-02															
		Borehole abandoned and re-drilled directly adjac borehole location.	ent to initial		SPT-02	1														
		CH - CLAY, medium to high plasticity, tan, cohesive	e, w>PL,		G-04										•		2			
		very stiff. SPT02 - 4/5/12/9		B.C.							····									
		150 mm thick gravel layer at 3.3 m depth; gravel a	ppears		G-05										· · ·					
		granitic in origin. SW/GW - SAND and GRAVEL, fine to coarse, angul															257			
	⊔	subangular gravel up to 19 mm, well graded, trac silt, brown, non-cohesive, moist, compact to den	se to some	/#P2	G-06 GPT-0															
	Sonic	\$PT03 - 41/30/8/6										•••••		· · · · · · · · · · · · · · · · · · ·		•••••				
		LB/SB - BOULDER/COBBLE (inferred from 4.6 m to Granite fragment (larger than sonic bit) recovered																		
		four (4.6 m to 6.1 m). SP/ML - SILT/SAND, sandy/silty, fine, grey, non-coh	hosivo wot														2			
		loose to compact.															255-			
		- subsurface stratigraphy inferred based on drilling response, split spoon recovery and adjacent bore		tle	SPT-0	4		l e	 59			÷								
		to no recovery from sonic core from 4.6 m to 9.0 m	depth.			1]				-							
		Drilling conditions described as very soft by driller. \$PT04 - 2/2/3/5															2			
		CH - CLAY, medium to high plasticity, trace organ tan, trace orange sand pockets, cohesive, w~P																		
		soft to firm.											-		· · ·					
		- subsurface stratigraphy inferred based on drilling r split spoon recovery and adjacent boreholes; little	e to no		≤SPT-0	5						-					2			
		recovery from sonic core from 4.6 m to 9.0 m dep conditions described as very soft by driller.	oth. Drilling		T	1							-			-				
		\$P - SAND, fine to coarse, well graded, trace to sor	ne gravel,																	
		trace silt, grey, non-cohesive, moist, loose. - subsurface stratigraphy inferred based on drilling r																		
		split spoon recovery and adjacent boreholes; little recovery from sonic core from 4.6 m to 9.0 m dep									:						25			
		conditions described as very soft by driller.	Juniy		C 07	12	Q./				· · · · · · · ·	····		<u> </u>						
		SPT05 - 1/2/4/50 ML - SILT, sandy, medium to fine sand, poorly graded, tan, non-cohesive, moist to wet, compact to very dense.			SPT-0		04		Ī		:			₽ ľ	-					
																	25			
		SPT06 - SPT Refusal (50 blows in 75 mm), no reco	overy																	
5		1	Contract	ontractor: VanMars									: : 12.8 m	:	_					
				uipment Type: Boart Longyear LS250									Start Date: 2022 December 14							
TETRATECH Equipment						y	, <i>ou</i>	202				Completion Date: 2022 December 14								
		J	Reviewe										ge 1 of 2			.				

			BRITISH	Bor	eł	าด	ble	N	١c):	Β	H22-()4					
			COLUMBIA	Project: C	hilliw	/ack	Lake	Roa	d - S	andł	nill SI	ide - Phase II	Project No:	704-ENG.VGEO04287-01				
			Ministry of Transportation	Location:	Chilli	iwac	k Lake	e Ro	ad				Ground Elev	v: 261.602 m				
			& Infrastructure	Chilliwack	к, В.С	С.							UTM: 59488	34.539 E; 5437115.298 N; Z 10	NAD83			
Depth (m)	Method	Core Diameter (mm)	Soil Description		Graphical Representation	Sample Type	Sample Number		artici Distril (%) Sand (%)	outic Sil			owcount 300 mm) 60 80	Field Vane (kPa) Post-Peak Peak 10 20 30 40 Plastic Moisture Liquid Limit Content Limit 20 40 60 80	Elevation (m)			
							G-08			72	2.3			•	-			
- - - - - - - - - - - - - - - - - - -	Sonic		 SP - SAND, silty, medium to fine, poorly graded, tan, non-cohesive, moist to wet, compact. Occassional increased fines content and orange/gre mottling from 10.7 m depth. SPT07 - 6/7/8/9 			X	G-09 SPT-07			32	2.4			•	251			
- - - - - 12	S		ML - SILT, sandy, medium to fine sand, poorly graded	d tan			G-10							•	250			
- - - - - -			non-cohesive, moist to wet, compact. - occassional bands of orange/grey SPT08 - 3/4/7/11 - increasing sand content and mottled orange/grey be m			X	SPT-08							•	249-			
- 13 			 END OF BOREHOLE at 12.8 m DEPTH Soil descriptions are based on visual classifications, observations and testing, drill performance, and la testing. Some variation through the interpreted soi expected. Soil consistency and density descriptions based on counts where available, and are otherwise based of the source of the sour	boratory I layers is SPT blow											248-			
- - - - - - - - - - - - - - - - - - -			 performance and visual observation. SPT blow counts shown on logs are uncorrected fiel N-values. SPTs were carried out using an automat hammer and AWJ rods. Upon completion the borehole was tremie grouted to with cement/bentonite grout. The hole was finished surface with sand. UTM coordinates and elevations were collected by N 	tic trip o surface d at the											247			
- - - - - - 16			Construction Services Ltd.												246			
- - - - - - - - - - - - -															245-			
- 17 															244			
- 10 															243-			
 															242			
				Contracto	r: Va	inMa	ars						Completion	Depth: 12.8 m				
			TETRA TECH	Equipmer			Boart L	ong	year	LS2	50			2022 December 14				
"		-		Logged B	-									Date: 2022 December 14				
				Reviewed	I By:	AS							Page 2 of 2					

			COLUMBIA	Project: (Chilliw	/ack	Lake	Roa	d - S	and	hill S	lide - P	hase II	Pro	oject No	: 704-EN	NG.VG	EO0428	37-01	
Ministry of Transportation & Infrastructure								e Ro	ad			Gr	Ground Elev: 258.468 m							
			& Infrastructure	Chilliwac	к, В.С).								UT	M: 5948	56.27 E	; 5437	083.564	N; Z 10	NAD
		Core Diameter (mm)			Graphical Representation	be	hber		artic <u>Distri</u>	butio Si	on ilt &	Field Blo (blows/30			owcount		Field Vane (kPa)			
(m)	Method	meter	Soil		epre	le Ty	Sample Number		(%	Cia	Clay (%			/300 n	nm)	Post-Peak Peak 10 20 30 40			Elevation (m)	
)	Re	e Dia	Description		cal R	Sample Type	ample		Sand (%)	Silt (%)	(%)						10 20 30 Plastic Moisture		Liquid	Ele
		Co			raphi		ő	Ū			Silt (%) Clay (%)					Lim		ontent	Limit	
)			TOPSOIL									2	0 40	60	80	2	0 40	60	80	
			SW - SAND, fine to coarse, well graded, trace to som trace silt, trace cobble up to 100 mm, occassional (twigs/rootlets), light brown, non-cohesive, moist.	ne gravel, organics			G-01													25
			SW - SAND, gravelly, silty, fine to coarse, well grade cobbles, black-brown, earthy odour, non-cohesive compact, lightweight.	, moist, /		G-02 PT-01 PT-01	a	65		 14 					•				25	
2			GW - SAND and GRAVEL to gravelly, fine to coarse, subrounded to angular gravel up to 60 mm, gap g occassional organics (twigs), some silt, brown-gre non-cohesive, moist. SPT01 - 2/5/13/8	graded,			G-03 30	59		11					•				25	
•			CH - CLAY, medium to high plasticity, trace organics (rootlets), tan, trace orange sand pockets, cohesive, w~PL to w>PL,				G-04 SPT-02													
4			tan, trace orange sand pockets, conesive, w~PL to soft to firm. - grey, w>PL from approximately 4.6 to 5.2 m - reddish brown tan from approximately 5.2 m SPT02 - 1/1/2/3	0 W>PL,		G-05			9	8.9						P	-•1		25	
	Sonic		SPT03 - 2/2/3/5				SPT-03													25
5							G-06 G-07										•			25
			SP - SAND, silty to some silt, fine to coarse, poorly graded, trace fine gravel, pockets of clay up to 40 mm diameter, brown, non-cohesive, moist. - grey from 5.8 to 6.1 m.				G-08 G-09 SPT-04		84		14					•				
			 - Inferred granite boulder/cobble from 6.1 m to 6.2 m. SPT04 - SPT Refusal (50 blows in 75 mm), no recover ML - SILT, non-plastic, trace sand, light brown, non-c moist to dry. 	ery			G-10			9	 5.7 					•				25
,			CI - CLAY, medium plasticity, tan, cohesive, w~PL to firm.) w>PL,			G-11								· · · · · · · · · · · · · · · · · · ·		1	•		25
5			- grey from 7.6 m. - 50 mm thick gravel seam at 7.7m - no sonic recovery from 7.6 to 9.1 m. SPT05 - 3/4/4/3			15 1	PT-05 PT-05								-					
•																				25
			END OF BOREHOLE at 9.1 m DEPTH																	
0			 Soil descriptions are based on visual classifications observations and testing, drill performance, and la testing. Some variation through the interpreted soil 	boratory il layers is																24
				ontractor: VanMars										Completion Depth: 9.1 m						
			TETRA TECH	uipment Type: Boart Longyear LS250										Start Date: 2022 December 14						
Logged By: ST Reviewed By: AS													Completion Date: 2022 December 14							

			BRITISH COLUMBIA	Bor	eł	າດ	ole	N	١c):	В	H22-0	5					
				Project: C	Chillin	/ack	Lake	Roa	d - S	andł	nill Sl	ide - Phase II	Project No: 7	704-ENG.VGEO04287-01				
			Ministry of Transportation	Location:	Chill	iwac	k Lak	e Ro	ad			Ground Elev: 258.468 m						
			& Infrastructure	Chilliwack	к, В.С).							UTM: 59485	6.27 E; 5437083.564 N; Z 10 N	NAD83			
Depth (m)	Method	Core Diameter (mm)	Soil Description		Graphical Representation	Sample Type	Sample Number				on It & / (%)	Field Blo (blows/3 □ SPT		Field Vane (kPa) Post-Peak Peak ↓ 10 20 30 40 Plastic Moisture Liquid Limit Content Limit	Elevation (m)			
10					ð					l S	C	20 40	60 80	20 40 60 80				
- - - - - - - - - - - - - - - - - - -			 expected. Soil consistency and density descriptions based on counts where available, and are otherwise based or performance and visual observation. SPT blow counts shown on logs are uncorrected fiel N-values. SPTs were carried out using an automa hammer and AWJ rods. Upon completion the borehole was tremie grouted to with cement/bentonite grout. The hole was finisher surface with server and an available and an available and an available and a surface with cement/bentonite grout. 											248				
- - - - - - - - - -			surface with sand. - UTM coordinates and elevations were collected by V Construction Services Ltd.	Van Bower														
- - - - - - - - - - - - - - - - - - -															246			
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				Equipmer				000	VOOR	100	50	Completion Depth: 9.1 m Start Date: 2022 December 14						
17			TETRA TECH	Logged B	-		ωαιι		yeai	L32	50							
				Reviewed	-								Page 2 of 2	Completion Date: 2022 December 14 Page 2 of 2				