### **MINISTRY OF TRANSPORTATION**

#### **NORTHERN REGION**

## **Geotechnical and Materials Engineering**

#### **GRAVEL INVESTIGATION**

### BERYL PRAIRIE EXTENSION PEACE DISTRICT

**Prepared for:** 

Gravel Resource Management Northern Region

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# **EXECUTIVE SUMMARY**

A subsurface gravel investigation of the Beryl Prairie Pit and the extension area was carried out in November 2007 to assess the quantity, quality, and suitability of the granular materials. This site is located approximately 8.5km west of Hudson's Hope. An application to convert the portion of the extension area in to a Section 16 Map Reserve is pending with Integrated Land Management Bureau.

Based on the results of the investigation, one granular area – Area A – was identified. The proven granular volume of Area A is 2,623,075 cubic meters. Additional volumes within the investigated area may exist at depth. The total combined topsoil and overburden volume for Area A is over 380,500 cubic meters.

The gravel in Area A is generally poorly graded and sandy with an average fine content of 3.2% and sand content at 22.2%. There is a greater percent of fine gravel (38.7%) than coarse gravel (35.7%). The average oversize content is 13.1%. Crushing the oversized rock during gravel production will likely improve the quality of the aggregates and will ensure full use of the pit. Some test pits ended in gravel at depth and some in sand, while the whole area contained primarily clean gravel.

The durability test results indicate that the gravel quality in Area A is generally good. The granular material appears suitable for pit run Bridge End Fill and SGSB and crushed WGB, IGB (25mm), Asphalt Mixes, Asphalt Base Course Mix, and Graded Aggregate Seals A-D.

Area A will require logging as well as clearing, grubbing, and stripping of topsoil and overburden. Ground water was not encountered in Area A.

All miming activities must be carried out according to the Health, Safety, & Reclamation code for Mines in British Columbia, Standard Specifications for Highway Construction, and the pit development plan. Progressive reclamation should be carried out of the depleted areas to return the land to productivity.

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## 1.0 Introduction

A subsurface gravel investigation was completed at the proposed Beryl Prairie Gravel Pit Extension in November of 2007 at the request of Gerry Hofmann, Gravel Resources Manager, Geotechnical and Materials Engineering, Northern Region. The purpose of this investigation was to assess the quantity, quality, and suitability of the granular materials within the boundaries of the existing reserve and notation of interest.

# 1.1 Location

The Beryl Prairie Gravel Pit is located approximately 8.5km west of Hudson's Hope, BC. Access to the site is gained by traveling on Canyon Drive west approximately 8.5km from the intersection of Canyon Drive and Highway 29 in Hudson's Hope and turning left to the pit entrance across from Beryl Prairie Road. It is located on the National Topographic Series map sheet 94 B/01 – Butler Ridge. The location of the site is shown in Figure 1.

# **1.2 Background Information**

This site falls within both a Section 16 Reserve and a Notation of Interest (NOI) held by the Ministry of Transportation. An application for the establishment of a Section 16 Reserve over a portion of the NOI has been sent to the Integrated Land Management Bureau for their approval. Previous subsurface investigations have been carried out on the existing Section 16 Reserve, but not in the NOI.

A study was completed on the existing Section 16 in 1982 with field investigation and lab testing. A total of 27 test pits were excavated identifying one granular area with a proven quantity of 2,623,075 cubic meters. This aggregate was suitable for pit run bridge end fill and SGSB, and crushed WGB, IGB (20-50mm), Hot Asphalt Mix, Asphalt Base Coarse Mix, and Graded Aggregate Seals A-D.

# 1.3 2007 Geotechnical Investigation

The 2007 geotechnical investigation comprised of test pitting and laboratory testing of retrieved samples. Eight days in November were spent in the field with a John Deere 200LC excavator from Quigley Contracting Ltd. Investigation was completed by Tim Woolnough of MoT.

A total of 65 test pits were excavated with depths ranging from 3.5 to 5.9m. While test pitting, the soils were visually identified and logged. Where appropriate, granular material was sampled for laboratory testing to determine durability and gradational characteristics. The locations of the test pits are shown in Figure 2 and a summary of the test pit data can be found in Appendix A.

Laboratory testing of the samples was carried out by Henry Nagasaka of AMEC Earth and Environmental, Prince George. Laboratory testing included wet sieve analysis for assessing grain size distribution (according to the Modified USCS), presence or absence of plastic fines (sand equivalent), absorption, abrasion loss (micro-deval testing), and bulk relative density (specific gravity). Based on the results of the field investigation and laboratory testing, one granular area – Area A – has been defined as shown in Figure 3. Test Pits excavated outside of Area showed little or no potential for granular materials.

# 2.0 Granular Characteristics

The following sections discuss the granular characteristics of material that was sampled and tested in the laboratory.

## 2.1 Pit Run Gradation

Pit run gradations were assessed from the results of the wet sieve analysis carried out on the materials passing the 75mm sieve. Table 1 shows the average pit run gradation results and the average pit run gradation corrected for the estimated oversize content. The pit run gradation summaries and charts are supplied in Appendix B and selected photographs of pit run material are provided in Appendix C.

Table 1Average Pit Run Gradation and Average Pit Run Gradation with Oversize<br/>(percent by weight of the minus 75mm fraction and of the entire deposit)

			Grav	el		Estimated	
Material	Fines (%) (<0.075 mm)	Sand (%) (0.075 - 4.75mm)	Fine (4.75 - 25mm)	Coarse (25-75mm)	Estimated Small Boulders (%) (75-300mm)	Large Boulders (%) (>300mm)	USC
Pit Run	3.2	22.2	38.7	35.8			GP
Corrected	2.8	19.5	33.6	31.1	13.1	0.0	GP

The gravel in Area A is classified as GP with an average gradation of 3.2% fines, 22.2% sand, 38.7% fine gravel, and 35.8% coarse gravel. The average oversize content of this area is 13.1% small boulders (Appendix B), and had an estimated maximum size of 280mm. Large boulders were not encountered in any of the pits.

# 2.2 Durability

Durability test results of the tested samples are summarized in Table 2.

Both the Micro Deval and the Sand Equivalent test results are well within the MoT specifications. The results for absorption tests on the coarse fraction are <2 and meet the requirements for asphalt mix aggregate. The absorption results for the fine fraction (1.4 & 1.3) meet the specifications for seal coating aggregates, but only one of the two results for the coarse fraction meets this requirement. The other result on the coarse fraction is only marginally high at 1.1; the specification requires <1.0. The Bulk Relative Density test results of both the coarse and fine fraction are slightly below the preferred value of 2.65, with the exception of one which is at this value. The durability characteristics may improve somewhat if more durable oversized rock is crushed with the gravel.

		Laborat	tory Durat	oility Resul	ts
Test Pit		Are	ea A		MoT Specifications
Test Fit	TP07-17	TP07-20	TP07-25	TP07-26	Mor Specifications
Micro Deval (%) (Coarse)	6			5	<30 for granular sub-base (coarse) <25 for granular base (coarse) <17 for asphalt aggregates (coarse)
Sand Equivalent	54			71	<ul><li>&gt;20 for sub-base and surfacing aggregates</li><li>&gt;40 for asphalt aggregates</li></ul>
Absorption (Coarse/Fine)		1.1/1.4	1.0/1.3		<2% for asphalt aggregates (coarse) <1.0/<1.5 for seal coating aggregates
Bulk Relative Density (Coarse/Fine)		2.63/2.62	2.65/2.63		~2.65 for all aggregate products

Table 2Laboratory Durability Results

#### **3.0** Granular Quantity

#### **3.1 Proven Quantity**

Table 3 provides the proven volumes of topsoil, overburden, and gravel in Area A. These volumes are based on the results of the test-pitting program.

Prove	n Top Soil, C	overburden a	and Gravel (	Juantities
Area A	Average Thickness (m)	Area (m²)	Proven Volume (m <sup>3</sup> )	Overburden* to Gravel Ratio
Top Soil	0.05	746,250	37,325	
Overburden	0.46	746,250	343,275	
Gravel	3.515	746,250	2,623,075	1:7

Table 3Proven Top Soil, Overburden and Gravel Quantities

\* Includes Topsoil

### **3.2 Probable Quantity**

Some test pits bottomed out in gravel suggesting that there is potential for more gravel at depth in Area A. A drilling program will be required to prove if additional volumes exist.

### 4.0 Gravel Suitability

Based on the lab results for gradation and durability, the gravel is anticipated to meet the MoT specifications for a number of different aggregate products (summarized in Table 4). Some products may require additional processing such as blending and/or rejecting of materials to meet specification. For specific requirements for each product see Sections 202, 501, and 531 in 2006 Standard Specifications for Highway Construction. Gradation plots and the gradation specification for the different aggregate products are presented in Appendix D.

The granular materials from Areas A and B can be used as pit run Bridge End Fill and pit run Select Granular Sub Base.

Gr	avel Suitability
Pit Run	Crushed
Bridge End Fill	Well Graded Base 25-75mm
Select Granular Sub-Base	Intermediate Graded Base 25-50mm
	Hot Asphalt Mixes
	Asphalt Base Course Mix
	Graded Aggregate Seals A-D

Table 4Gravel Suitability

Materials from Area A can be used for the production of crushed products including bases, seal coats, and asphalt mixes.

### 5.0 Pit Development

All mining activities must be carried out in accordance with the Health, Safety and Reclamation Code for Mines in British Columbia, 2006. The Pit Development Plan must be followed as outlined in Figure 3. Also, when the pit has been depleted, the disturbed areas must be returned to productive land as soon as possible.

# 5.1 Development Considerations

## 5.1.1 Vegetation

Area A contains vegetation as both mature coniferous and deciduous trees that will require logging before granular removal takes place. Both areas will require clearing, grubbing and disposal which should be carried out according to Section 200 of the 2006 Standard Specifications for Highway Construction. All slash is to be burned according to the provisions of the Forest Act and Regulations, and the remaining ashes are to be mixed with the top soil for nutrient enrichment.

# 5.1.2 Topsoil and Overburden Stripping

Top Soil and Overburden must be stripped and stockpiled separately as shown on the development plan. Only six test pits contained overburden while all test pits had 0.05m thick topsoil. The overburden in each pit was sand with no gravel and some fines, and depths ranged from 0.65 - 2.45m.

# 5.1.3 Water Table

Water table was not encountered in any of the test pits. However, the gravel at the bottom of TP07-06 appeared wet indicating that the water table may be present at this location. If a water table is encountered during mining, the gravel removal should be at least 0.5m above the water table. If gravel extraction is to take place below the water table, the water will have to be drained. The water draining plan will have to be designed by a professional geoscientist/engineer in consultation with the environmental agencies.

## 5.1.4 Access

Access to the development area (Area A) can be gained by extending the existing pit face into the new area. The access to the pit is off Canyon Drive and via the pit access road located at the northwest corner of the existing reserve (Figure 3).

# 6.0 Gravel Extraction

Gravel extraction into Area A will be from the existing pit face located just north of Area A (Figure 3). The extraction is to be directed towards the south and then east. Crusher set up, staging and stockpiling can be located on the existing pit floor. No new pit face needs to be developed.

## 7.0 Pit Reclamation

Progressive pit reclamation of the disturbed and depleted areas must be carried out to return the areas to productive land, without interfering with future gravel extraction and pit development.

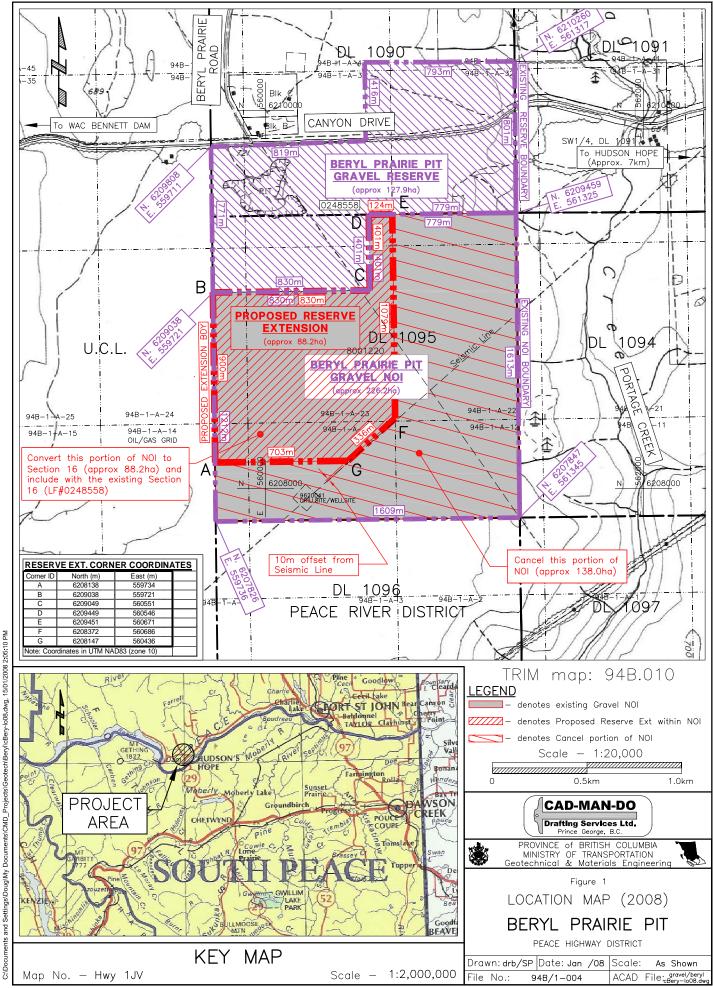
The minimum reclamation procedure should consist of:

- i) Re-sloping of the pit face to no steeper than 2H:1V
- ii) Spreading subsoil over the reclamation area. The subsoil should consist of silt or clay available from the overburden stockpiles.
- iii) Spreading topsoil over the subsoil.
- iv) Seeding the topsoil with appropriate vegetation.

A pit reclamation plan should be prepared and approved before proceeding with the reclamation.

Figure 1

**Location Map** 



15/01/2008 2:05:10 Projects/Geotech/BervI/cBerv-lo08.dwg ents/CMD Settings/Doug/My s pue

Figure 2

Pit Plan

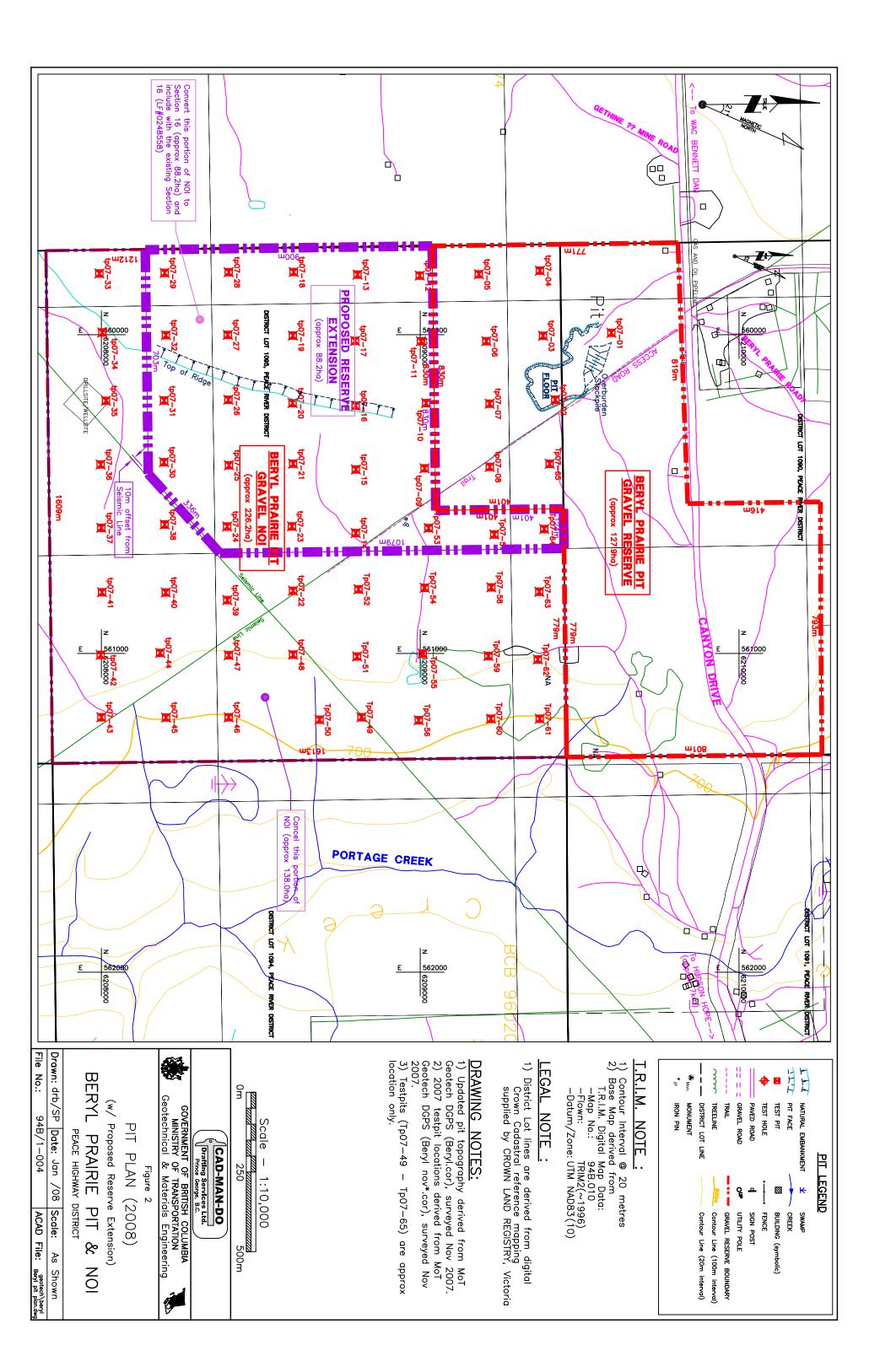
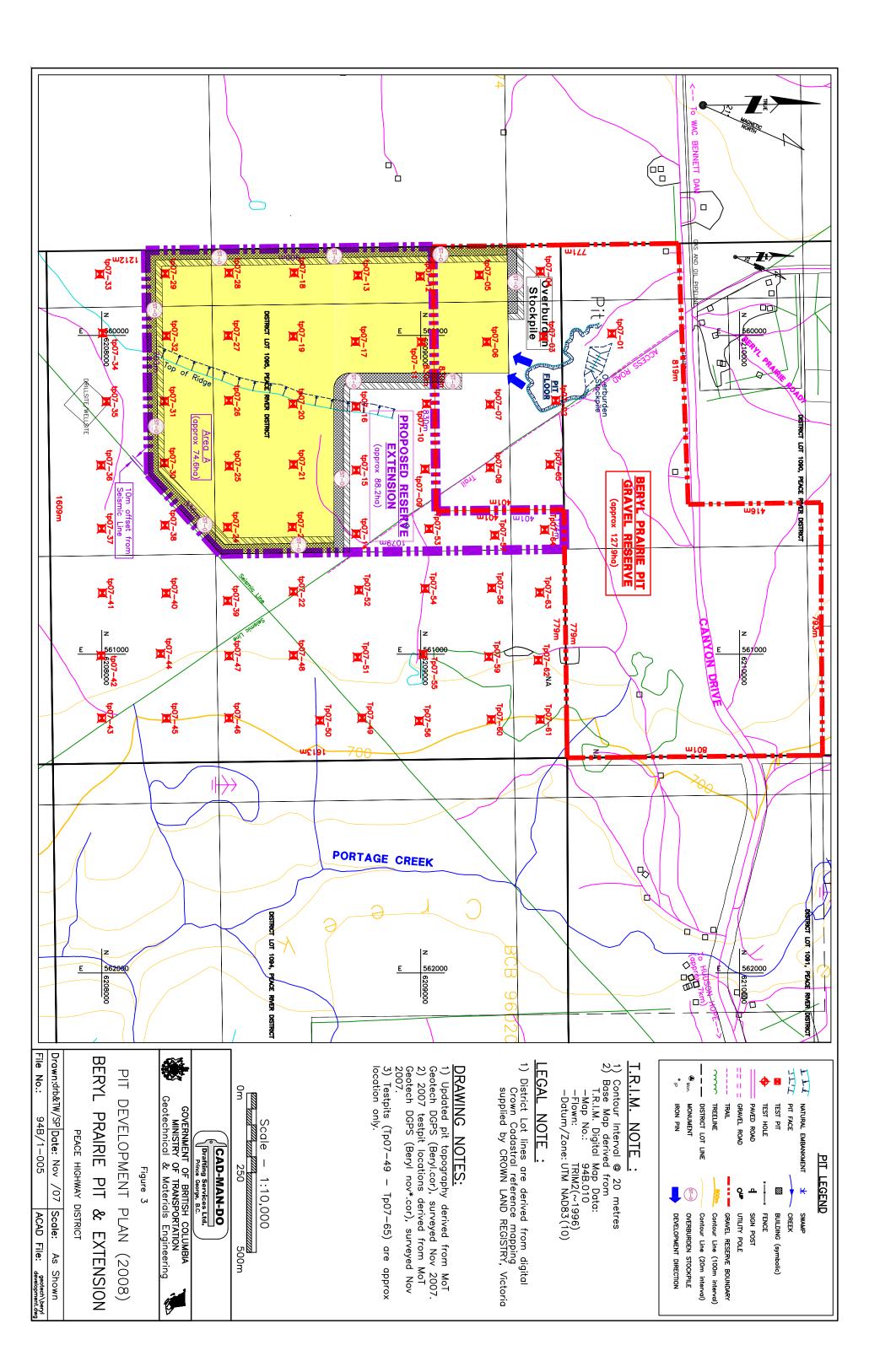


Figure 3

**Proposed Development Plan** 



Appendix A

**Test Pit Summary** 

												TE	ES	ΓΡΙΤ	SU	MM		Y									
PRO	JECT	r				BER	YL P	RAIR	IE P	IT AN	ID NO	CI				TES		ГНО	D				TRA	CKED E	XCAVA	FOR	
DISTI	RICT	•						PE	ACE							DAT	E					NO	VEMBE	ER 20-23	AND 27	7-30, 2007	
				Ê	F	ELD	visi	JAL I	DEN	TIFIC	ATIC	ON							LAE	BORA	TORY	TEST	RESUL	TS			
ER	ШШ			S (r					(	OVE	RSIZI	=			PIT R	RUN		<b>C</b>	RUS	ы				DURAE	BILITY		
TEST PIT NUMBER	SAMPLE NUMBER			LAYER THICKNESS (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	>300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND EQUIVALENT	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	FRACTURE A/B	B.R.D coarse/fines	ABSORPTION %coarse/%fines
TES	SA	From	То	ΓАΥΕ	CLAS				75-15	150-3	>30	MAX	WATI	CLASS	ш		U			G	EQL	MICF	B	A %coa	FRAC	Соа	ABS %coa
07-01	1	0.0	1.7	1.7	GW-GM	6	38	56	5	8		250															
		1.7	4.4	2.7	SP-SM	6	94																	<b> </b>			
07-02		0.0 0.1	0.1 4.9	0.1 4.8	GW-GM SP-SM	6 7	40 93	54	1	1		200															
07-03		0.0	0.1	0.1	TS																						
07-03		0.0	4.9	4.8	SP	2	98																				
		4.9	5.1	0.2	GW-GM	5	33	62	2			130															
07-04		0.0	0.1	0.1	TS	0	07																				
		0.1	5.0	4.9	SP	3	97																	-			
07-05		0.0	0.1	0.1	TS																						
		0.1	1.7	1.6	SP	2	98																				
	1	1.7	4.6	2.9	GW	3	40	57	8	9		230		GP-GM	6	28	66										
07-06		0.0	0.1	0.1	TS																						
01 00	1	0.1	1.8	1.7	GP-GM	5	30	65	10	16		280															
	2	1.8	4.8	3.0	GW-GC	7	35	58	4	6		160		GP	4	18	78										
07.07		0.0	0.4	0.1	TO																			<b> </b>			
07-07		0.0	0.1 4.2	0.1 4.1	TS SP	3	97							}										<u> </u>			
		0.1	7.4	7.1			57																				
07-08		0.0	0.1	0.1	TS																						
		0.1	5.1	5.0	SP	2	98																				
07-09		0.0	0.1	0.1	TS																						
07-09	1	0.0	1.5	1.4	GW	4	41	55	6	3		180												<u> </u>			
	Ė	1.5	1.8	0.3	CH	100																					
	1	1.8	4.8	3.0	GW	4	41	55	7	3		180															
07.40		0.0	0.4	0.4	ТО						<b> </b>								<u> </u>	<u> </u>				<u> </u>			
07-10		0.0	0.1 4.7	0.1 4.6	TS SC1	15	85																	<u> </u>			
L	I	0.1	7.7	7.0	001	10	00				I	I		I		1		I			1		I	1	I	1	

												TE	ES1	ΓΡΙΤ	SU	MM	AR	Y									
PRO.	JECT	Г				BER	YL P	RAIR	IE P	IT AN	ID NO	DI				TES		гно	D				TRA	CKED EX	XCAVAT	OR	
DISTI	RICT	-						PE	ACE							DAT	E					NO	VEMBE	ER 20-23	AND 27	<b>′-</b> 30, 2007	
				Ê	FI	FI D	VISI	JAL I		TIFIC		)N							I AF	SORA	TORY	TEST	RESUI	TS			
L K	Ľ.	1	Ê	L) (L)						OVEF					PIT R									DURAE			
MBF	ABE			ES:	Z						SIZE		(E)	z													
TEST PIT NUMBER	SAMPLE NUMBER			LAYER THICKNESS (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	>300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND EQUIVALENT	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	FRACTURE A/B	B.R.D coarse/fines	ABSORPTION %coarse/%fines
TE:	SA	From	То	ΓАΥΕ	CLAS				75-15	150-3	>30	MAX	WATI	CLAS			G			0	EQL	MICF	B	N %co8	FRAC	соа	ABS %coa
07-11		0.0	0.1	0.1	TS																						
	1	0.1	3.8	3.7	GW-GC	8	40	52	6	9		250		GP	2	20	78										
07-12		0.0	0.1	0.1	TS																						
07-12		0.0	0.1	0.1	SP	3	97																				
		0.7	1.4	0.7	GW	4	42	54	2			120															
		1.4	5.0	3.6	SP	3	87	10	_																		
07-13		0.0	0.1	0.1	TS																						
		0.1	1.7	1.6	GW-GM	10	40	50	8	12		260															
		1.7	4.5	2.8	SP-SM	5	73	22	5			100															
07-14		0.0	0.1	0.1	TS																						
0		0.1	1.4	1.3	GW-GM	6	45	49	3	5		180															
		1.4	3.8	2.4	SP	2	85	13					1														
07-15		0.0	0.1	0.1	TS																						
	1	0.1	4.8	4.7	GW	3	45	52	3	3		180															
07-16		0.0	0.1	0.1	TS																						
0, 10	1	0.0	2.9	2.8	SP	4	96													1							
	1	2.9	4.3	1.4	GW	4	45	51	4	4		170						1	1	1				1	1		
		4.3	5.3	1.0	SP	4	83	13																			
07-17		0.0	0.1	0.1	TS		05		4.0	10		000		0.5		45	0.1	<u> </u>	<u> </u>	<u> </u>	54						
	1	0.1	4.4	4.3	GW	5	35	60	10	12		280		GP	4	15	81				54	6					
07-18		0.0	0.1	0.1	TS																						
0, 10	1	0.0	2.3	2.2	GW	4	40	56	3	2		160		GP	2	35	63			1							
	İ İ	2.3	2.7	0.4	SP	2	98		-				1					1	1	1				1	İ		
	2	2.7	4.7	2.0	GW	3	40	57	9	16		280															
07-19		0.0	0.1	0.1	TS		67		_			000		65						<u> </u>							
	1	0.1	3.5	3.4	GW	3	37	60	7	8		200		GP	3	23	74										

												TE	ES1	ΓΡΙΤ	SU	MM	AR	Y									
PRO	JECT	Г				BER	RYL P	RAIR	IE P	IT AN	ID NO	JI				TES	T ME	гно	D				TRA	CKED EX	XCAVA	TOR	
DIST	RICT	-						PE	ACE							DAT	E					NO	VEMBE	R 20-23	AND 27	7-30, 2007	
				Ê	FI	ELD	VISU	JAL I	DEN	TIFIC	ATIC	ON							LAE	BORA	TORY	TEST	RESUL	TS			
L H	ВR	<u></u>	Ē	S (L					(	OVEF	SIZE				PIT R	UN		0	CRUS	н				DURAE	BILITY		
TEST PIT NUMBER	SAMPLE NUMBER			LAYER THICKNESS (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	>300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND EQUIVALENT	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	FRACTURE A/B	B.R.D coarse/fines	ABSORPTION %coarse/%fines
TES	SAI	From	То	ГАУЕ	CLAS			Ū	75-15	150-3	>30(	MAX	WATE	CLASS			U			Ū	EQU	MICF	DE	V %coa	FRAC.	coa	ABS %coa
07-20		0.0	0.1	0.1	TS																						
	1	0.1	4.4	4.3	GW	4	40	56	6	9		230		GP	3	24	73									2.63/2.62	1.1/1.4
07-21		0.0	0.1	0.1	TS					<u> </u>		<u> </u>	<u> </u>														
0		0.1	2.5	2.4	SP	4	96																				
	1	2.5	5.0	2.5	SW	3	50	47	3	2		190															
		5.0	5.2	0.2	GW-GM	5	38	57	3	2		170															
07-22		0.0	0.1	0.1	TS																						
01 22		0.0	4.4	4.3	SP	1	99																				
07-23		0.0	0.1	0.1	TS SP	3	97																				
	1	1.5	2.9	1.4	GW	3	40	57	5			150		GP	1	30	69										
		2.9	4.8	1.9	SP	2	78	20	Ŭ			100		0		00	00										
07-24		0.0	0.1	0.1	TS																						
07-24	1	0.0	0.1 2.8	2.7	GW	4	40	56	4	5		170															
	1	2.8	4.3	1.5	SP	2	86	12	4	5		170															
07-25		0.0	0.1	0.1	TS									_													
	1	0.1	4.2	4.1	GW	4	40	56	7	6		200	<u> </u>	GP	2	22	76			<u> </u>						2.65/2.63	1.0/1.3
07-26		0.0	0.1	0.1	TS																						
	1	0.1	5.2	5.1	GW	4	35	61	8	12		220		GP	2	19	79				71	5					
07.07		0.0	0.4	0.1			<u> </u>											<u> </u>	<u> </u>								
07-27		0.0	0.1 2.3	0.1	TS SP	2	98																<u> </u>				
	1	2.3	4.0	1.7	GP	4	90 25	71	7	2		170		GP	4	13	83							<u> </u>			
07-28		0.0	0.1	0.1	TS				6			455						<u> </u>	<u> </u>								
	1	0.1	5.5	5.4	GP	4	28	68	8			150															
<b> </b>							<del> </del>											<u> </u>	<u> </u>								
L					1		1	8			l	I	I	1	1	I	1			ı		L	I	8	1	1	

												TE	ES	Γ ΡΙΤ	SU	MM		Y									
PRO.	JECT	Г				BER	RYL P	RAIR	IE P	IT AN	ID NO	DI				TES		гно	D				TRA	CKED E	XCAVAT	FOR	
DIST	RICT	-						PE	ACE							DAT	E					NO	VEMBE	R 20-23	AND 27	7-30, 2007	
				Ê	FI	ELD	VISU	JAL I	DEN	TIFIC	ATIC	ON							LAE	BORA	TORY	TEST	RESUL	TS			
ER	Ш	1		S (r					(	OVEF	RSIZE	Ξ			PIT R	UN		0	RUS	SH				DURAE	BILITY		
TEST PIT NUMBER	SAMPLE NUMBER			LAYER THICKNESS (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	>300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND EQUIVALENT	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	FRACTURE A/B	B.R.D coarse/fines	ABSORPTION %coarse/%fines
TES	SAI	From	То	LAYEI	CLAS				75-15	150-3	>30(	MAX	WAT	CLASS			Ū			Ū	EQU	MICF	В	 %coa	FRAC	L	ABSe %coa
07-29		0.0	0.1	0.1	TS																						
	1	0.1	5.1	5.0	GW	4	31	65	5	7		200															
07-30		0.0	0.1	0.1	TS																						
01 00	1	0.1	2.3	2.2	GW	4	45	51	4	3		170															
	2	2.3	4.5	2.2	GW	2	40	58	4			150															
07-31		0.0	0.1	0.1	TS																						
01 01		0.0	1.5	1.4	GW	4	46	50	7			150															
	1	1.5	4.3	2.8	GW	2	43	55	5	3		170															
07-32		0.0	0.1	0.1	TS																						
07 02		0.0	0.8	0.7	SP-SM	6	94																				
	1	0.8	3.5	2.7	GW-GM	5	40	55	6	5		200		GP-GM	6	21	73										
07-33		0.0	0.1	0.1	TS																						
		0.1	4.7	4.6	SP-SC	12	88																				
		4.7	5.0	0.3	СН	100																					
07-34		0.0	0.1	0.1	TS																						
		0.1	1.7	1.6	SP	4	96																				
		1.7	2.1	0.4	GW	3	40	57	6			150															
		2.1 3.8	3.8 5.3	1.7 1.5	SP SP	3	97 75	22	18	7		220															
		0.0	0.0	1.0	- Or	5	13	~~~	10			220															<u>                                     </u>
07-35		0.0	0.1	0.1	TS																						
		0.1	0.8	0.7	SP	4	96									ļ		<u> </u>	<u> </u>	<u> </u>			ļ				
		0.8	1.3 5.2	0.5	SP SP	4	73 77	23 20	2			100	<u> </u>					<u> </u>									$\vdash$
		1.0	0.2	0.0	5	-		20				100															
07-36		0.0	0.1	0.1	TS																						
		0.1	5.3	5.2	SP	1	99											<u> </u>	<b> </b>	<b> </b>							
	1	I	1	I	1			I	1	I	L	I	I	1	1	I	1	I	I	1	I		I	I	1	1	

												TE	S	ΓΡΙΤ	SU	MM	AR	Y									
PRO	JECT	-				BER	RYL P	RAIF	RIE P	IT AN	ID NO	CI				TES	T ME	гно	D				TRA	CKED E	XCAVA	FOR	
DIST	RICT	•						PE	ACE							DAT	E					NO	VEMBE	R 20-23	AND 27	7-30, 2007	
				_ ۲	F	IELD	VISU	JAL	IDEN	TIFIC	ATIC	ON							LAE	BORA	TORY	TEST	RESUL	TS			
SER	ШШ	(a)		ss (r					(	OVEF	RSIZE				PIT R	RUN		0	RUS	ы				DURAE	BILITY		
TEST PIT NUMBER	SAMPLE NUMBER	DEDTH (m)		LAYER THICKNESS (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	>300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND EQUIVALENT	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	rure a'b	B.R.D coarse/fines	ABSORPTION %coarse/%fines
TES	SAN	From	То	LAYEF	CLAS				75-15	150-3(	>300	MAX :	WATE	CLASS			0			5	EQU	MICR	DE	N %coa	FRACTURE	E	ABS( %coa
07-37		0.0	0.1	0.1	TS																						
		0.1	1.3	1.2	SP	2	98					400						<u> </u>									
	1	1.3	4.7	3.4	GW	3	42	55	3	3		180															
07-38		0.0	0.1	0.1	TS		+		$\vdash$		-							+						<u> </u>			┼──┤
		0.1	1.0	0.9	GW	3	40	57	3			130															
		1.0	5.5	4.5	SP	2	98																				
07-39		0.0	0.1	0.1	TS																						
07-39		0.0	5.6	0.1 5.5	SP	4	96																				
			0.0	0.0	•																						
07-40		0.0	0.1	0.1	TS																						
		0.1	4.2	4.1	SP	3	97																				
07-41		0.0	0.1	0.1	TS																						
01 11		0.1	5.2	5.1	SP	3	96	1																			
07-42		0.0	0.1	0.1	TS	4	00																				
		0.1	4.1	4.0	SP	4	96																				
07-43		0.0	0.1	0.1	TS		1											1									
		0.1	4.4	4.3	SP	2	98																				
07.41		0.0	0.1	0.1																				<u> </u>			
07-44		0.0	0.1 4.7	0.1 4.6	TS SP	4	96																				
	<u> </u>	0.1	4.1	4.0	35	4	30		+		-							+									+
07-45		0.0	0.1	0.1	TS																						
		0.1	4.8	4.7	SP-SM	5	95																				
07-46		0.0	0.1	0.1	TS		<u> </u>		<u> </u>									<u> </u>						<b> </b>			
07-46	<u> </u>	0.0	0.1 5.9	0.1 5.8	SP-SM	5	95																	<u> </u>			
		0.1	0.0	0.0					1									1									
07-47		0.0	0.1	0.1	TS										l												
		0.1	5.3	5.2	SP	2	98																				

												TE	ES1	ΓΡΙΤ	SU	MN	AR	Y									
PRO	JECT	-				BER	YL P	RAIR	RIE P	IT AN	ID NO	CI				TES	T ME	гно	D				TRA	CKED E	XCAVA	FOR	
DIST	RICT	•						PE	ACE							DAT	E					NO	VEMBE	R 20-23	AND 27	7-30, 2007	
				Ê	FI	ELD	VISU	JAL I	DEN	TIFIC	ATIC	ON							LAE	BORA	TORY	TEST	RESUL	TS			
ER	ШШ	<u></u>	Ê	s (r					(	OVEF	SIZE	=			PIT R	RUN		0	RUS	SH				DURAE	BILITY		
TEST PIT NUMBER	SAMPLE NUMBER			LAYER THICKNESS (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	75-150mm (%)	150-300mm (%)	>300mm (%)	MAX SIZE (mm)	WATERTABLE (m)	SOIL CLASSIFICATION	FINES	SAND	GRAVEL	FINES	SAND	GRAVEL	SAND EQUIVALENT	MICRO DEVAL	DEGRADE	MgSO4 %coarse/ %fine	FRACTURE A/B	B.R.D coarse/fines	ABSORPTION %coarse/%fines
TES	SAI	From	То	ГАУЕІ	CLAS				75-15	150-3	>30(	MAX	WATE	CLASS			Ū			Ū	EQU	MICF	В	N V	FRAC.	F coa	ABS %coa
07-48		0.0	0.1	0.1	TS																						
	1	0.1	5.2	5.1	SP	4	96									<b> </b>		<u> </u>	<u> </u>	<b> </b>							
07-49		0.0	0.1	0.1	TS														-								
		0.1	5.2	5.1	SP	4	96																				
07-50		0.0	0.1	0.1	TS																						
		0.1	5.0	4.9	SP	2	98																				
07-51		0.0	0.1	0.1	TS																						
		0.1	5.1	5.0	SP	3	97																				
07-52		0.0	0.1	0.1	TS																						
07-52		0.0	0.1	0.1	GW	3	45	52																			
		0.7	3.0	2.3	SP	2	98	02																			
		3.0	5.1	2.1	SP	2	83	15																			
07-53		0.0	0.1	0.1	TS																						
		0.1	1.0	0.9	GW	4	40	56	3			130															
		1.0	2.1	1.1	SP	4	96																				
	1	2.1 3.6	3.6 5.0	1.5 1.4	ML GW-GM	97 6	3 40	54	4			130				<u> </u>			-								
		3.0	5.0	1.4	300-0101	0	40	54	4			130						<del> </del>									
07-54		0.0	0.1	0.1	TS																						
		0.1	4.0	3.9	SP	2	98																				
07.55		0.0	0.1	0.1	TO											<u> </u>		<u> </u>									
07-55		0.0	0.1 4.8	0.1 4.7	TS SP	3	97																	<u> </u>			
-		0.1	0	7.1	5	5	57									<u> </u>		1									
07-56		0.0	0.1	0.1	TS																						
		0.1	5.2	5.1	SP	4	96																				
																		<u> </u>						<b> </b>			
L				1	1		I	I	I	I		I	I	I	1	<u> </u>	1	1	I		1		I	I	I	1	1

1000000000000000000000000000000000000										Y	AR	MM	SU	ΓΡΙΤ	ST	TE												
UNING       FIELD VISUAL IDENTIFICATION       LABORATORY TEST RESULTS         UNING       FIELD VISUAL IDENTIFICATION       LABORATORY TEST RESULTS         UNING       FIELD VISUAL IDENTIFICATION       LABORATORY TEST RESULTS         VIEL       FIELD VISUAL IDENTIFICATION       CRUSH       DURABILITY         VIEL       FIELD VISUAL IDENTIFICATION       VIEL       PTT RUN       CRUSH       DURABILITY         VIEL       FIELD VISUAL IDENTIFICATION       VIEL       VIEL       PTT RUN       CRUSH       DURABILITY         VIEL       FIELD VISUAL IDENTIFICATION       VIEL       IEL       VIEL       <	ર	TOR	XCAVA	CKED EX	TRA				D	тно	Т МЕ	TES				JI	ID NO	IT AN	IE P	RAIR	YL P	BER				Г	JEC	PRO.
Ham       E       S       S       OVERSIZE       PTRUN       CRUSH       DURABILITY         1<	0, 2007	27-30, 2	AND 2	R 20-23	/EMBE	NO					E	DAT							ACE	PE						Г	RICT	DIST
O7-57       0.0       0.1       0.1       TS       0 </td <td></td> <td></td> <td></td> <td>тѕ</td> <td>RESUL</td> <td>TEST F</td> <td>TORY</td> <td>ORA</td> <td>LAB</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>DN</td> <td>ATIC</td> <td>TIFIC</td> <td>DEN</td> <td>JAL I</td> <td>VISU</td> <td>ELD</td> <td>FI</td> <td>(L</td> <td></td> <td></td> <td></td> <td></td>				тѕ	RESUL	TEST F	TORY	ORA	LAB							DN	ATIC	TIFIC	DEN	JAL I	VISU	ELD	FI	(L				
O7-57       0.0       0.1       0.1       TS       0 </th <th></th> <th></th> <th>BILITY</th> <th>DURAE</th> <th></th> <th></th> <th></th> <th>н</th> <th>RUS</th> <th>C</th> <th></th> <th>UN</th> <th>PIT R</th> <th></th> <th></th> <th>Ξ</th> <th>RSIZE</th> <th>OVEF</th> <th>(</th> <th></th> <th></th> <th></th> <th></th> <th>S (r</th> <th>Ê</th> <th>1</th> <th>ШЧ</th> <th>L L</th>			BILITY	DURAE				н	RUS	C		UN	PIT R			Ξ	RSIZE	OVEF	(					S (r	Ê	1	ШЧ	L L
O7-57       0.0       0.1       0.1       TS       0 </th <th>B.R.D coarse/fines ABSORPTION %coarse/%fines</th> <th>3.R.D</th> <th>A/B</th> <th></th> <th>GRADE</th> <th>O DEVAL</th> <th>SAND IVALENT</th> <th></th> <th></th> <th></th> <th>RAVEL</th> <th></th> <th></th> <th>SOIL</th> <th>ertable (m</th> <th></th> <th>(%)</th> <th></th> <th></th> <th><b>BRAVEL</b></th> <th>SAND</th> <th>FINES</th> <th>SOIL</th> <th>R THICKNES</th> <th>DEPTH</th> <th></th> <th></th> <th>T PIT NUME</th>	B.R.D coarse/fines ABSORPTION %coarse/%fines	3.R.D	A/B		GRADE	O DEVAL	SAND IVALENT				RAVEL			SOIL	ertable (m		(%)			<b>BRAVEL</b>	SAND	FINES	SOIL	R THICKNES	DEPTH			T PIT NUME
0.1     0.8     0.7     SP-SM     8     92     0 <t< td=""><td>E coa ABSC %coa</td><td></td><td>FRAC</td><td>N %coa</td><td>DE</td><td>MICR</td><td>EQU</td><td>ß</td><td>0,</td><td></td><td>Ū</td><td></td><td></td><td>CLASS</td><td>WATE</td><td>MAX</td><td>&gt;30(</td><td>150-3(</td><td>75-15</td><td>0</td><td></td><td></td><td>CLAS</td><td>ГАУЕ</td><td>То</td><td>From</td><td>SAI</td><td>TES</td></t<>	E coa ABSC %coa		FRAC	N %coa	DE	MICR	EQU	ß	0,		Ū			CLASS	WATE	MAX	>30(	150-3(	75-15	0			CLAS	ГАУЕ	То	From	SAI	TES
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$																								0.1	0.1	0.0		07-57
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		$\perp$																								-		
1     2.2     4.3     2.1     GW     3     42     55     5     150     1 <t< td=""><td></td><td>่</td><td>┣───</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td> </td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td><u> </u></td><td></td></t<>		่	┣───																								<u> </u>	
07-58       0.0       0.1       0.1       TS       0 </td <td></td> <td>┥──</td> <td>┣───</td> <td></td> <td></td> <td></td> <td><math>\vdash</math></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>150</td> <td> </td> <td> </td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td> </td>		┥──	┣───				$\vdash$									150			-									
0.1     2.3     2.2     SP     4     96     1     10 <t< td=""><td></td><td>+</td><td>───</td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td>150</td><td></td><td> </td><td>5</td><td>55</td><td>42</td><td>3</td><td>GW</td><td>2.1</td><td>4.3</td><td>2.2</td><td>1</td><td> </td></t<>		+	───							-						150			5	55	42	3	GW	2.1	4.3	2.2	1	
0.1     2.3     2.2     SP     4     96     1     10 <t< td=""><td></td><td></td><td><u> </u></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>TS</td><td>0.1</td><td>0.1</td><td>0.0</td><td></td><td>07-58</td></t<>			<u> </u>																				TS	0.1	0.1	0.0		07-58
07-59       0.0       0.1       0.1       TS       0 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>96</td> <td>4</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0.00</td>																					96	4						0.00
0.1     4.1     4.0     SP     2     98     1		_														150			6	60	37	3	GW	2.5	4.8	2.3	1	
0.1     4.1     4.0     SP     2     98     1																												
07-60     0.0     0.1     0.1     TS     1 <t< td=""><td></td><td>┥──</td><td><math>\vdash</math></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td>07-59</td></t<>		┥──	$\vdash$																			-						07-59
0.1     4.9     4.8     SP     2     98  <		┥──	┣───																		98	2	SP	4.0	4.1	0.1		
0.1     4.9     4.8     SP     2     98  <		<u> </u>	<u> </u>																				те	0.1	0.1	0.0		07-60
07-61     0.0     0.1     0.1     TS     1 <t< td=""><td></td><td>+</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>98</td><td>2</td><td></td><td></td><td></td><td></td><td></td><td>07-00</td></t<>		+																			98	2						07-00
0.1     5.2     5.1     SP     3     97  <																					00		0.			0.1		
07-62     0.0     0.1     0.1     TS     0 <t< td=""><td></td><td>_</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>TS</td><td>0.1</td><td>0.1</td><td>0.0</td><td></td><td>07-61</td></t<>		_																					TS	0.1	0.1	0.0		07-61
0.1     5.0     4.9     SP     2     98     Image: SP of the system																					97	3	SP	5.1	5.2	0.1		
0.1     5.0     4.9     SP     2     98  <		—	┣───																									07.00
07-63     0.0     0.1     0.1     TS     Image: state stat		+	┣───																		00	°.				-		07-62
0.1     5.1     5.0     SP     4     96     Image: state st		+	├───							-											90	2	37	4.9	5.0	0.1	+	<b> </b>
0.1     5.1     5.0     SP     4     96     Image: state st	<u> </u>	+																					TS	0.1	0.1	0.0	-	07-63
07-64     0.0     0.1     0.1     TS     Image: state stat		+																			96	4					1	0.00
0.1 5.3 5.2 SP 3 97																												
																										0.0		07-64
		┥	└──																		97	3	SP	5.2	5.3	0.1	<b> </b>	
		┥	──																				TO	0.4	0.1	0.0		07.05
07-65       0.0       0.1       0.1       TS       Image: Constraint of the co		+	┣───							-											00	۰ ر						07-65
0.1       0.9       0.8       SP       2       98		+	├───							-						180		5	7	60						-	1	
1       0.9       3.0       2.7       GW       3       37       60       7       5       160         3.6       4.1       0.5       SP       3       82       15       160       1	<del></del>	+	<u> </u>													100		5										<b> </b>
4.1 4.9 0.8 GW 3 45 52 2 130		+														130			2							-	1	
	<u> </u>	1								1			1				1	1	İ		_	-				1	1	

Appendix B

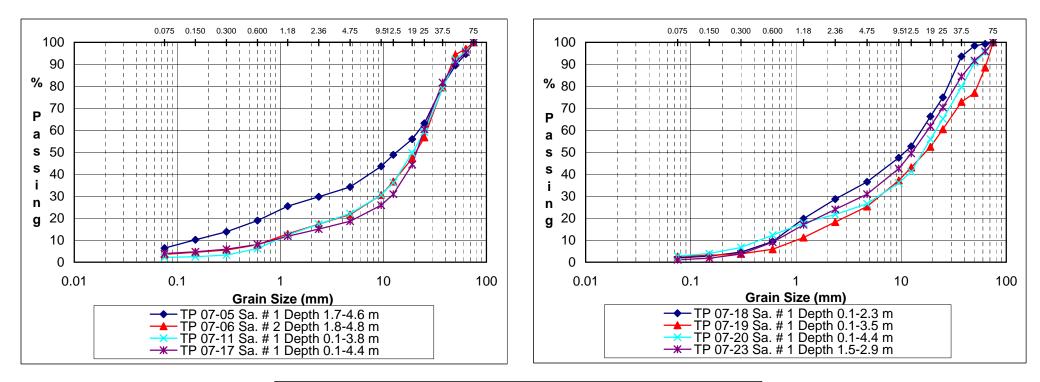
**Pit Run Gradation Summary and Plots** 

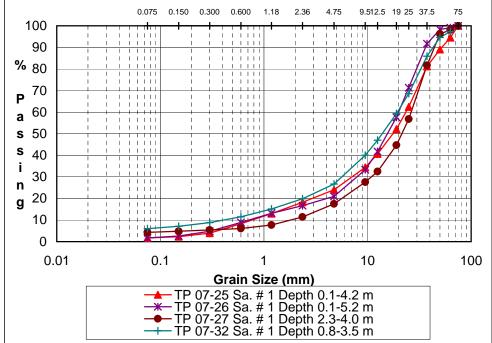
Area		De	pth			Grav	USC	
	Test Pit TP07- #	From To		Fines (%) (<0.075 mm)	Sand (%) (0.075 - 4.75mm)	Fine (4.75 - 25mm)		
	5	1.7	4.6	6.4	27.8	29.0	36.8	GP-GM
	6	1.8	4.8	3.6	18.1	35.1	43.2	GP
	11	0.1	3.8	2.2	19.9	37.1	40.8	GP
	17	0.1	4.4	3.9	14.7	41.9	39.5	GP
	18	0.1	2.3	2.0	34.5	38.5	25.0	GW
А	19	0.1	3.5	2.6	22.7	35.2	39.5	GP
~	20	0.1	4.4	2.8	23.8	38.6	34.8	GP
	23	1.5	2.9	1.1	29.8	39.5	29.6	GP
	25	0.1	4.2	1.7	22.3	38.4	37.6	GP
	26	0.1	5.2	1.7	19.3	50.3	28.7	GP
	27	2.3	4.0	4.3	13.2	39.3	43.2	GP
	32	0.8	3.5	6.0	20.7	41.9	31.4	GP-GM
Mean				3.2	22.2	38.7	35.8	GP

Laboratory Pit Run Gradation Results

Laboratory Pit Run Gradation Results Corrected to Include Oversized Estimates

Area	Test Pit TP07- #			G	ravel	Estimated	Estimated	
		Fines (%) (<0.075 mm)	Sand (%) (0.075 - 4.75mm)	Fines (%) (<0.075 mm)	Coarse (%) (0.075 - 4.75mm)	Small Boulders (%) (75-300mm)	Large Boulders (%) (>300mm)	Max Size (mm)
	5	5.3	23.1	24.1	30.5	17.0		230
	6	3.2	16.3	31.6	38.9	10.0		160
	11	1.9	16.9	31.5	34.7	15.0		250
	17	3.0	11.5	32.7	30.8	22.0		280
	18	1.9	32.8	36.6	23.8	5.0		160
А	19	2.2	19.3	29.9	33.6	15.0		200
~	20	2.4	20.2	32.8	29.6	15.0		230
	23	1.0	28.3	37.5	28.1	5.0		150
	25	1.5	19.4	33.4	32.7	13.0		200
	26	1.4	15.4	40.2	23.0	20.0		220
	27	3.9	12.0	35.8	39.3	9.0		170
	32	5.3	18.4	37.3	27.9	11.0		200
Mean		2.8	19.5	33.6	31.1	13.1	0.0	





Appendix C

Selected Photographs of Pit Run Material



Photo # 1. Pit Run Materials – TP07-06



Photo # 2. Pit Run Materials – TP07-11



Photo # 3. Pit Run Materials – TP07-17

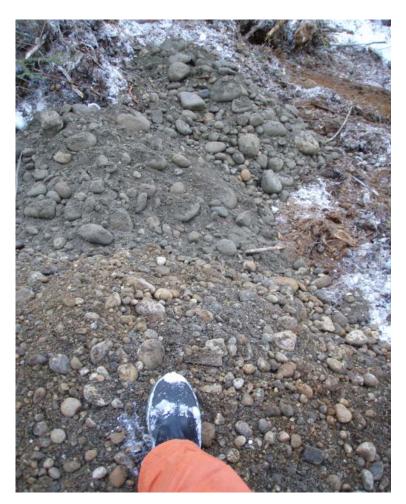


Photo # 4. Pit Run Materials – TP07-18



Photo # 5. Pit Run Materials – TP07-19



Photo # 6. Pit Run Materials – TP07-20



Photo # 7. Pit Run Materials – TP07-21



Photo # 8. Pit Run Materials – TP07-23



Photo # 9. Pit Run Materials – TP07-24



Photo # 10. Pit Run Materials – TP07-25



Photo # 11. Pit Run Materials – TP07-26



Photo # 12. Pit Run Materials – TP07-27



Photo # 13. Pit Run Materials – TP07-28



Photo # 14. Pit Run Materials – TP07-29





Photo # 16. Pit Run Materials – TP07-31

Appendix D

Pit Run and Material Specification Gradations

Sample Information							Sieve Sizes (mm)														
Test Pit	Sample	Depth	USC	GRAVEL	SAND	FINES	<b>.</b>														
#	#	(m)		%	%	%	75	63	50	37.5	25	19	12.5	9.5	4.75	2.36	1.18	0.6	0.3	0.15	0.08
07-05	1	1.7-4.6	GW	66	28	6	100	89.5	80.2	63.2	56.0	48.8	43.6	34.2	29.8	25.5	18.9	13.8	10.2	6.4	6.4
07-06	2	1.8-4.8	GP	78	18	4	100	97.1	94.3	79.5	56.8	47.2	36.7	30.6	21.7	17.3	12.9	7.9	5.5	4.5	3.6
07-11	1	0.1-3.8	GP	78	20	2	100	96.1	92.2	79.0	59.2	49.7	36.2	30.6	22.1	17.3	12.4	6.2	3.3	2.5	2.2
07-17	1	0.1-4.4	GP	81	15	4	100	95.5	90.9	81.7	60.5	44.4	30.9	25.9	18.6	15.1	11.8	8.1	5.9	4.7	3.9
07-18	1	0.1-2.3	GP	64	35	2	100	99.3	98.5	93.6	75.0	66.3	52.7	47.5	36.5	28.7	19.8	9.4	4.6	2.8	2.0
07-19	1	0.1-3.5	GW	75	23	3	100	88.5	77.0	72.9	60.5	52.5	43.1	37.2	25.3	18.3	11.2	5.9	3.8	3.1	2.6
07-20	1	0.1-4.4	GP	73	24	3	100	95.4	90.8	79.7	65.2	56.0	41.2	36.1	26.6	21.7	17.9	12.3	6.7	3.9	2.8
07-23	1	1.5-2.9	GW	69	30	1	100	95.9	91.7	84.5	70.4	61.8	49.5	42.6	30.9	24.0	17.0	9.1	3.7	1.8	1.1
07-25	1	0.1-4.2	GW	76	22	2	100	94.5	89.0	81.2	62.4	52.1	40.6	34.4	24.0	18.2	13.0	8.3	3.9	2.3	1.7
07-26	1	0.1-5.2	GP	79	19	2	100	99.1	98.3	91.7	71.3	57.6	41.7	33.3	21.0	16.7	13.1	8.9	4.8	2.6	1.7
07-27	1	2.3-4.0	GW	83	13	4	100	98.1	96.2	81.7	56.8	44.7	32.5	27.6	17.5	11.4	7.7	6.1	5.4	4.8	4.3
07-32	1	0.8-3.5	GP-GM	73	21	6	100	97.3	94.6	85.9	68.6	59.3	47.0	40.0	26.7	19.9	15.1	11.5	8.8	7.1	6.0
	MAX			83	35	6	100	99.3	98.5	93.6	75.0	66.3	52.7	47.5	36.5	29.8	25.5	18.9	13.8	10.2	6.4
	MIN			64	13	1	100	88.5	77.0	72.9	56.8	44.4	30.9	25.9	17.5	11.4	7.7	5.9	3.3	1.8	1.1
	MEAN		GW	75	22	3	100	96.0	91.9	82.6	64.2	54.0	41.7	35.8	25.4	19.9	14.8	9.4	5.9	4.2	3.2

#### BASE COURSE AGGREGATE-WELL GRADED

100

80

**%** <sup>90</sup>

**P** 70

**a** 60

**s** 50

**s** 40

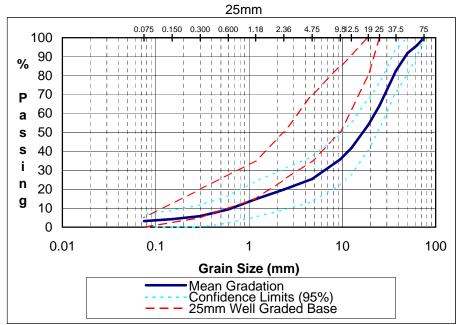
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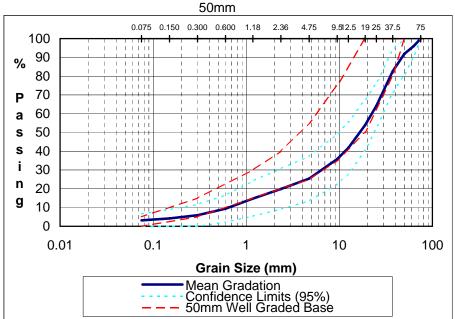
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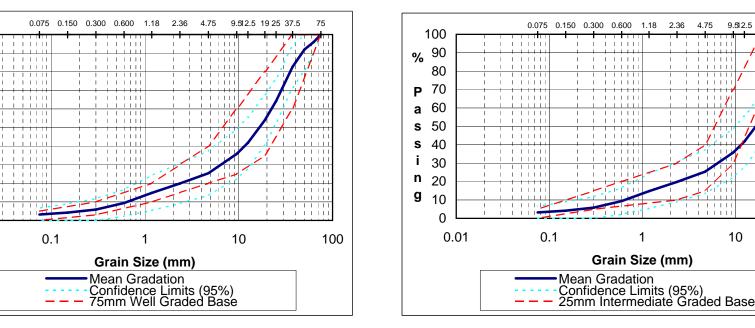
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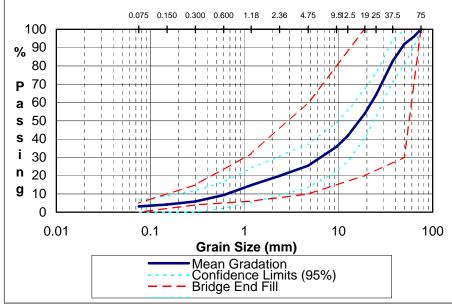
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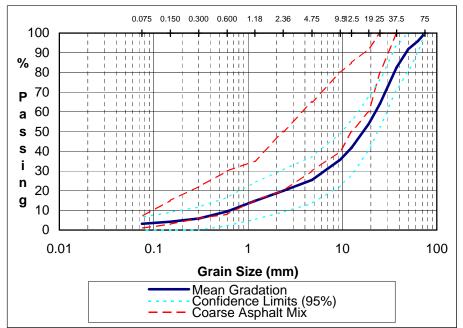


75mm

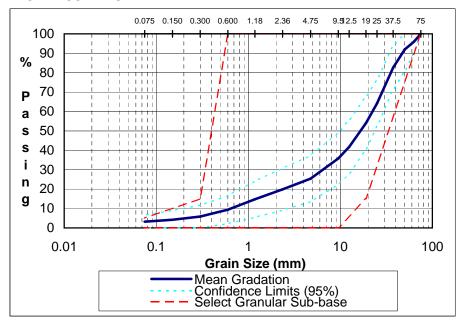
#### REQUIREMENTS FOR EMBANKMENT MATERIAL BRIDGE END FILL

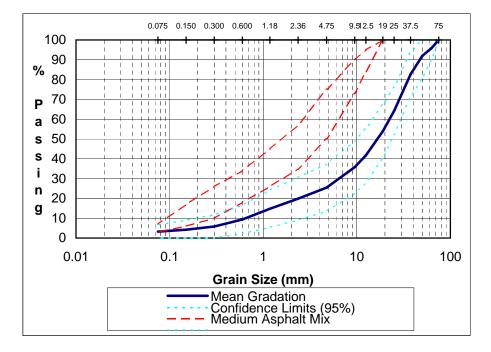


GRADATION LIMITS FOR AGGREGATES HOT MIXED ASPHALT PAVEMENT

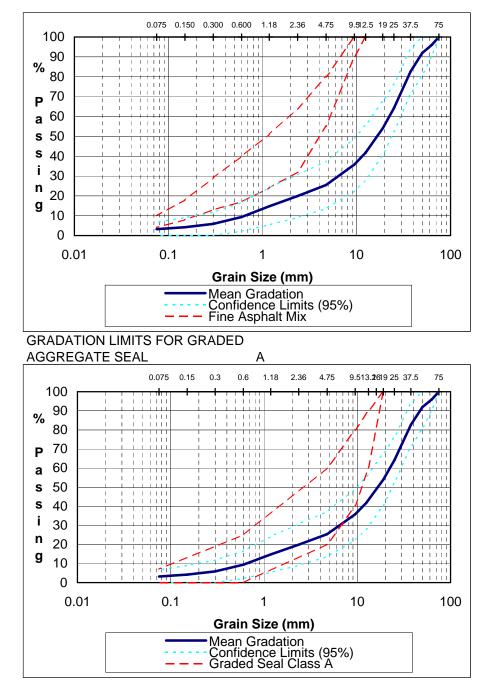


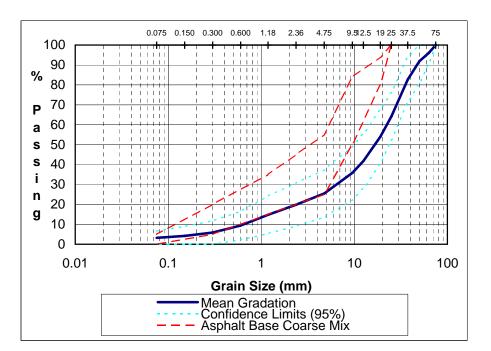
#### REQUIREMENTS FOR SELECT GRANULAR SUB-BASE

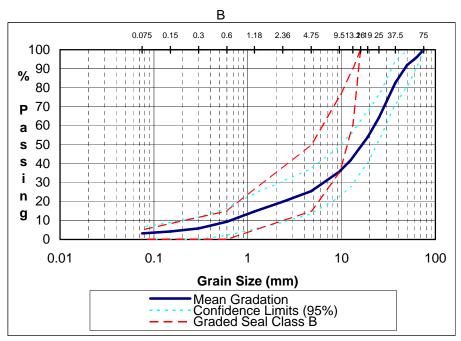




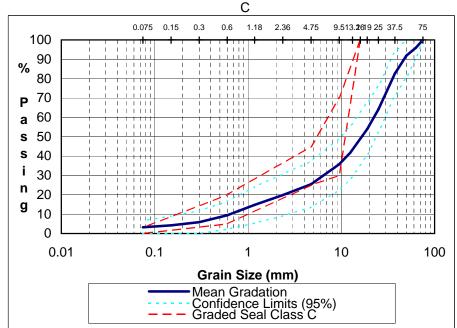
- Continued







#### GRADATION LIMITS FOR GRADED **AGGREGATE SEAL - Continued**



GRADATION REQUIREMENTS FOR AGGREGATE (Asphalt Mix)

