

Kicking Horse River Bridges Replacement Project Geotechnical Factual Data Report – Phases 1 and 2



PRESENTED TO Urban Systems Ltd. British Columbia Ministry of Transportation and Infrastructure

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LIMITATIONS OF REPORT

This report and its contents are intended for the sole use of Urban Systems Ltd. (USL) and The British Columbia Ministry of Transportation and Infrastructure (MoTI) and their agents. Tetra Tech Canada Inc. (Tetra Tech) does not accept any responsibility for the accuracy of any of the data, the analysis, or the recommendations contained or referenced in the report when the report is used or relied upon by any Party other than USL and MoTI, or for any Project other than the proposed development at the subject site. Any such unauthorized use of this report is at the sole risk of the user. Use of this document is subject to the Limitations on the Use of this Document attached in Appendix A or Contractual Terms and Conditions executed by both parties.



1.0 INTRODUCTION

Tetra Tech Canada Inc. (Tetra Tech) is pleased to present the following data report to Urban Systems Ltd. (USL) and the British Columbia (BC) Ministry of Transport and Infrastructure (MoTI) for the Highway 95 Kicking Horse River (KHR) Bridges Replacement Project (the Project).

The following report summarizes Phase 1 and Phase 2 of the geotechnical subsurface exploration program carried out by Tetra Tech from March 8 to 22, 2021 and September 13 to 17, 2021 respectively, to support the functional and detailed design phases of the Project.

Phase 1 and Phase 2 of the exploration program were carried out in general accordance with the Site Investigation Work Plan, submitted to MoTI and USL in December 2020, but with changes in Phase 2 due to the required relocation of the testholes out of the CP property, and considering relative costs of different drilling methods and findings of the previous phase of exploration. These changes in the program were discussed with MoTI and USL and agreed upon in July and August 2021.

This report presents the factual data collected during the March and September 2021 exploration programs. Some limited data interpretation is included as part of typical processing of raw data. The analytical components of the design will be presented in milestone Design Reports under a separate cover as the project progresses.

2.0 PROJECT DESCRIPTION

The KHR Bridges are located in Golden, BC, approximately 73 km west of the Alberta border and 148 km east of Revelstoke, BC. The existing KHR Bridges 1 and 2, originally built in 1952, are nearing the end of their service life and have been identified for replacement by MoTI. The bridges cross the KHR on Highway 95 to link the north and south sides of the Town of Golden, including a small stretch of road on Gould's Island. Regionally, these bridges provide a key link between Highway 1 and Highway 3 and several communities along Highway 95 between Golden and Cranbrook.

USL is leading this project along with Tetra Tech and COWI North America Ltd. (COWI) to perform functional and detailed design tasks, including discussions with MoTI regarding alternative bridge alignment option(s). The new bridges will likely be pile-supported structures with abutments on the north and south sides of the KHR with a pier located near the east end of Gould's Island.

The geotechnical exploration was undertaken to obtain additional information regarding soil stratigraphy and geotechnical properties for pile foundation design, embankment stability analyses, and pavement structure design. A previous site exploration program was completed as part of the Preliminary Design work (Golder 2020).

3.0 SUBSURFACE EXPLORATION PROGRAM

3.1 General

A summary of the subsurface exploration completed is provided in Table 3-1 and the testhole locations are presented on Figure 1.

Phase 1 was completed between March 8 to 22, 2021 and consisted of:

- One sonic testhole on Gould's Island to characterize the soil conditions near the proposed pier location of the main bridge.
- One Instrumented Becker Penetration Test (iBPT) testhole to characterize the soil density and compaction properties near the proposed south abutment of the main bridge.
- Two sonic testholes on Highway 95 to obtain asphalt thickness and characterize the pavement structure and materials.
- One sonic testhole in Kumsheen Park to characterize the subsurface conditions below the proposed trenchless watermain crossing.

Phase 2 was completed from September 13 to 17, 2021 and consisted of:

- One sonic testhole to characterize the subsurface conditions near the proposed north abutment.
- Two sonic testholes on Highway 95 north of the river to obtain asphalt thickness and characterize the pavement structure and materials.
- One hydrovac testhole on Highway 95 south of the river to obtain asphalt thickness and characterize the pavement structure and materials.

Caroline Reader and Katrina Berube of Tetra Tech were on site through the duration of the Phase 1 and Phase 2 exploration programs, respectively. They observed drilling conditions, logged soil conditions, recorded in-situ testing data, and collected samples for laboratory testing. Soils were initially classified visually and by texture in accordance with Tetra Tech internal geotechnical soil classification work method. Tetra Tech geotechnical soil classification work method has been developed based on the general guidelines provided in the Canadian Foundation Engineering Manual 4th Edition and ASTM standards. The soil logging was in general conformance with American Society for Testing and Materials (ASTM) standard D2487 and D2488.

Tetra Tech conducted BC One Call information requests to identify potential buried services across the site and retained the services of Okanagan Utility Locators for the locating and marking of all underground services in the field. Buried utility locates for all testholes in Phase 1 and Phase 2 were completed on March 1, 2021 and September 13, 2021 respectively, to minimize the risk of damage to services during the subsurface exploration program. The utility locator used electromagnetic techniques sensitive to buried conductors, such as metal pipes and cables. The Town of Golden Public Works Utilities Team aided in the locating of buried water and sewage pipes, and a member of their team was present on site during the utility locates on March 1, 2021 and September 13, 2021.

For BH21-01 and HV21-07 during Phase 2, where utilities were detected in close proximity during the utility locate, utility clearance by means of hydrovac excavation was used to physically clear the testhole locations to a depth of 2.4 m and 3.5 m, respectively. Kootenay Pumping was subcontracted by Tetra Tech to hydrovac excavate BH21-01 and HV21-07 on September 14 and September 13, 2021, respectively.

Traffic management was required during both phases in areas where the drilling had to be performed on or close to existing roads. Tetra Tech subcontracted Sentry Traffic Control Ltd. to create traffic management plans during Phase 1 and Crossroads Highway Services Ltd. to provide traffic control operations during both Phase 1 and 2.

Cultural monitoring was coordinated by Tetra Tech through MoTI Indigenous Relations and Shuswap Band. Shuswap Band (Kenpesq't Territorial Operations Ltd.) provided cultural monitoring services to inspect the soils for archeological remnants for the duration of both the Phase 1 and Phase 2 exploration programs.

A summary of the testhole locations is presented in Table 3-1, and the geotechnical testhole logs are presented in Appendix B. UTM (NAD83) coordinates and elevations of the testhole locations were surveyed by Geometric Surveys Ltd. on March 18, 2021 (Phase 1), September 17, 2021 (Phase 2) and December 3, 2021 (Phase 2: HV21-07).

	Testhole ID	Drilling Method	UTM (NAD83, Zone: 11) Coordinates ⁽¹⁾		Drilling Dates		Target		
Phase			Easting	Northing	Elevation (masl)	Start	End	Depth (m)	Actual Depth (m)
2	BH21-01	Hydrovac/ Sonic	502544.763	5683077.296	789.029	15-Sep-2021	17-Sep-2021	50	52.3
1	iBPT21-02	iBPT	502573.183	5682957.541	788.948	15-Mar-2021	22-Mar-2021	50	47.5
1	BH21-03	Sonic	502559.621	5683006.745	788.789	09-Mar-2021	12-Mar-2021	60	64.0
2	BH21-04	Sonic	502408.265	5683262.233	786.651	14-Sep-2021	14-Sep-2021	3	3.5
1	BH21-05	Sonic	502346.306	5683377.475	786.372	13-Mar-2021	13-Mar-2021	3	3.5
2	BH21-06	Sonic	502455.388	5683196.769	787.071	14-Sep-2021	14-Sep-2021	3	3.5
2	HV21-07	Hydrovac	502578.518	5682820.613	788.404	13-Sep-2021	13-Sep-2021	3	3.0
1	BH21-08	Sonic	502521.106	5682956.036	789.522	13-Mar-2021	13-Mar-2021	3	3.5
1	BH21-09	Sonic	502626.001	5682957.348	789.576	12-Mar-2021	13-Mar-2021	15	15.2

Table 3-1. Summary of Testholes

(1) Coordinates and elevations were surveyed (+/- 0.03 m) and precise values (to three decimal places) were provided by Geometric Surveys Ltd. after completion of each exploration phase.

3.2 Sonic Drilling

Sonic drilling was carried out by Mobile Augers and Research Ltd. (Mobile Augers) using a Fraste Multidrill XL track-mounted rig. Drilling was completed using the method outlined by ASTM D6914 for Sonic Drilling. The soil is recovered in intervals of 1.5 m to 3.0 m, and then laid out in core boxes for the geotechnical engineer to log the recovered material. Core boxes were transported to Tetra Tech's geotechnical laboratory in Calgary, AB.

Sampling in coarse-grained (granular) soils was also carried out by means of the Standard Penetration Test (SPT) split-spoon sampler at 1.5 m intervals down to 15 m depth, then at 3.0 m intervals thereafter where the testhole continued deeper. A 63.5 kg (140 lb) hammer weight was used for SPT in accordance with ASTM D1586-18 with a drop height of 30 inches (76.2 cm). However, during Phase 1 after completion of BH21-03 and during drilling of BH21-09 at about 11 m depth, the automatic trip hammer malfunctioned and required repair. It was discovered that the drilling crew had placed a 300 lb (136 kg) hammer weight inside the automatic trip hammer. This weight is used for performing Large Penetration Tests (LPT). It was replaced with a 140 lb (63.5 kg) weight that is used for SPT in accordance with ASTM D1586-18. Blow counts were recorded by Tetra Tech's field engineer for every 150 mm of



penetration and the split-spoon samples were visually classified and logged in the field. Undisturbed piston sampling using thin-walled Shelby tubes was attempted in a silt and sand layer at about 54.9 m depth in BH21-03 during Phase 1; however, no sample was recovered.

After the samples were inspected and logged, representative disturbed samples were selected for classification testing at Tetra Tech's geotechnical laboratory. Photographs of the core box samples from Phase 1 to 2 and split-spoon samples from Phase 2 are included at the end of this report. The recorded blow counts are shown on the testhole logs attached in Appendix B.

At the request of Tetra Tech, energy transfer measurements were performed by Mobile Augers on March 15, 2021, results of which are shown in Appendix C. These energy measurements can be used to convert the blow counts obtained using LPT hammer to an equivalent SPT hammer blow count.

3.3 Instrumented Becker Penetration Testing

Instrumented Becker Penetration Testing (iBPT) was performed by Foundex Explorations Ltd. (Foundex) during Phase 1, using their truck-mounted Becker rig; and ConeTec Investigations Ltd. (ConeTec) provided and operated the instrumentation used to collect downhole energy measurements and the iBPT blow counts, which can be correlated to equivalent SPT N₆₀ values. The results are shown in Appendix D.

iBPT21-02 hit refusal (>250 blow counts in a 150 mm interval during closed, instrumented testing) four times during drilling as shown on the iBPT plots in Appendix D. When refusal occurred, the closed bit was pulled up and then switched to an open bit with the instrumentation removed, and the dense material was drilled through until the blow counts were low enough to return to instrumented testing. When the final refusal occurred at 47.5 m (2.5 m above the target depth of 50 m), the testhole was terminated.

3.4 Testhole Closure

Testhole backfilling varied depending on the testhole location and depth. Table 3-2 details the backfill material used in each testhole. All testholes were backfilled upon completion in accordance with the BC Groundwater Protection Regulations and MoTI requirements.

Testhole ID	Location	Backfill
BH21-01	Northeast of existing bridge	Cement-Bentonite grout to 1.5 m topped with bentonite chips to surface
iBPT21-02	Kumsheen Park	Pea gravel and bentonite tablets (time release), compacted base rock to surface
BH21-03	Gould's Island	Cement-Bentonite grout to surface
BH21-04	10 th Ave N (Highway 95), north of existing bridge	Bentonite chips and sand topped with compacted cold mix asphalt to surface
BH21-05	Highway 95	Cuttings topped with compacted cold mix asphalt to surface
BH21-06 10 th Ave N (Highway 95), north of existing bridge		Bentonite chips and sand topped with compacted cold mix asphalt to surface
HV21-07 10 th Ave S (Highway 95), south of existing bridge		Compacted sand topped with compacted cold mix asphalt to surface

Table 3-2. Testhole Backfilling

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Testhole ID	Location	Backfill
BH21-08	Gravel parking area adjacent to Highway 95	Cuttings, compacted base rock to surface
BH21-09	Kumsheen Park	Bentonite chips, compacted base rock to surface

In BH21-01 and BH21-03, the grout mix followed the industry standard Mikkelsen mix ratio of 2 water to 1 cement to 1/3 bentonite (by weight).

4.0 LABORATORY TESTING

4.1 Samples

The samples collected during the drilling were taken to Tetra Tech's geotechnical laboratory in Calgary, AB, for laboratory testing. The samples collected from the SPT testing and sonic cores are disturbed and were used for general classification tests.

The following soil tests were completed as part of the testing program:

- Soil Description and Classification (11, 12)
- Natural Moisture Content (16, 2)
- Sieve Analysis (11, 10)
- Fines Content Percent Passing #200 Sieve (5, 9)

The numbers in brackets refer to the total number of each test performed on the samples from the testholes in Phase 1 (preceding the comma) and Phase 2 (after the comma). The results of the geotechnical and material testing are presented on the testhole logs in Appendix B and the particle size distribution tests are presented in Appendix E.

Other test results, e.g., those related to corrosion, will be presented in a separate report after testing requirements are provided by other design disciplines and testing is completed.

4.2 Soil Description and Classification

In the field and upon recovery, the soil samples were classified visually and by texture in accordance with MoTI's Notes for Completion of Soil Field Logs (MoTI 2016) and Geotechnical and Materials Engineering Standards for Bridge Foundation Investigations (MoTI 1991).

The detailed soil descriptions include the following information:

- Main soil type
- Secondary soil components
- Qualitative assessment of grading (granular soils)
- Shape and angularity (granular soils)
- Plasticity (cohesive soils)



- Colour
- Structure, texture, or other relevant observations
- Moisture
- Compactness / Consistency

Soils have been classified as coarse-grained (non-cohesive) soils or fine-grained (cohesive) soils. Non-cohesive soils are described in terms of the relative proportions of the mineral constituents. Cohesive soils are classified based on plasticity (Atterberg limits) as per ASTM D2487-17.

On the drilling logs in Appendix B, the field descriptions have been revised to consider the results of the laboratory classification tests at the specific depth of the samples.

4.3 Natural Moisture Content

Moisture content determinations were performed on the samples recovered from the testholes. Natural moisture contents are determined from the difference in measured total and dry weights after oven drying of specimens taken from soil samples recovered from the testhole. Measurements are performed in accordance with the procedures described in ASTM D2216-19.

The results of the natural water content tests are plotted on the testhole logs presented in Appendix B.

4.4 Fines Content and Particle Size Distribution

Particle size distribution tests and fines content determinations were performed on the disturbed soil samples to characterize the granular layers identified within the testholes. Particle size distribution tests were performed in accordance with the ASTM D6913-17 and the results are presented on a semi-logarithmic plot with grain size (log) versus percentage passing by weight finer than the grain (sieve) size. Data from the referred tests were used to verify the visual descriptions made in the field.

For fines content determinations, the proportion of fines (silt and clay) in a specimen was determined by washing the material through the #200 (75 µm) sieve and computing the percentage passing.

The relative proportions of gravel, sand, and fines are indicated on the testhole logs in Appendix B. The results of the particle size distribution tests are tabulated and presented graphically in Appendix E.

5.0 SUMMARY OF GEOTECHNICAL CONDITIONS

5.1 Soil Conditions

The general soil conditions at the site are represented by six (6) soil units. The layers identified are described as follows:

- Unit I TOPSOIL: A layer of topsoil approximately 0.2 m in thickness was encountered near the ground surface in BH21-01, BH21-03, and BH21-09.
- Unit II FILL: A layer of fill was encountered in BH21-05 and BH21-08 to a depth of about 3.5 m and 1.9 m, respectively. The layer consisted of silty sand and gravel, to sandy gravel, to gravelly sand, to sand with some silt, to gravelly sand and silt. Inclusions of tar were observed in both BH21-05 and BH21-08 to depths of about 0.5 m and 0.8 m, respectively.
- Unit III GRAVEL / COBBLES: This unit was encountered underlying topsoil in BH21-01, BH21-03, and BH21-09 to depths ranging from about 1.1 m to 2.4 m. The layer was also encountered underlying the fill in BH21-08 and underlying the road section in BH21-04, BH21-06, and HV21-07, to termination depths. The unit varies in composition from gravel to sandy silty gravel, to sandy gravel. Cobbles up to 250 mm in diameter were observed along with fine to coarse, subangular to rounded gravel, and fine to coarse sand. The unit is compact to very dense, grey-brown in colour, and dry to moist.
- Unit IV GRAVEL: This unit was encountered underlying Unit III and extending to a depth of about 38.7 m and 38.9 m in BH21-03 and BH21-01, respectively. The unit extended to the termination depth (15.2 m) in BH21-09. The unit varies in composition from gravelly sand, to sandy silty gravel, to sandy gravel with some silt, to gravel and silt. The gravel is fine to coarse, subangular to rounded and the sand is fine to coarse. Cobbles were recovered in the unit up to 100 mm in diameter (constrained by the diameter of the sonic drill casing; may be larger in the field). The unit varies in compactness from compact to very dense. It is damp to wet and greybrown in colour.
- Unit V SILTY SAND: This unit was encountered in BH21-01 and BH21-03 underlying Unit IV and extending to depths of about 39.8 m and 45.5 m, respectively. The unit consisted of silty sand with trace to no gravel. Occasional silt laminations up to 30 mm in thickness and silt inclusions up to 30 mm in diameter were observed throughout the samples from this unit. The sand was fine, yellow-brown in colour, wet, and compact.
- Unit VI SAND / SILT: This unit was encountered in BH21-01 and BH21-03 underlying Unit V and extending to termination depth. The unit is generally comprised of sand and silt. A sandy silt layer about 1.1 m thick was encountered in BH21-03 at about 56.1 m depth. A silt and sand layer was encountered at about 51.5 m depth and extended to the termination depth (52.3 m) of BH21-01. Interbedded silt layers were observed in the lower part of the unit. The sand is fine to medium and the unit is moist to wet and grey-brown in colour. The unit varies in compactness from compact to very dense, becoming very stiff to hard where the fines content increases. In BH21-03, the unit is clearly defined from the overlying silty sand unit (Unit V) by a marked colour change from yellowish brown to grey, along with an increase in silt content.

5.2 Groundwater Conditions

Inferred groundwater levels were taken from the testholes during subsurface exploration. Groundwater levels were inferred to range from approximately 3.0 m to 7.5 m depth in BH21-01, BH21-03, and BH21-09. These observations only reflect the groundwater conditions over the duration of the exploration program. Groundwater levels may vary seasonally or with precipitation; and can vary between testholes or at the time of construction. Inferred groundwater levels observed during the exploration may have been affected by the water used for drilling operations.



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6.0 CLOSURE

We trust this document meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully Submitted, Tetra Tech Canada Inc.

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FIGURES

Figure 1 Testhole Location Plan





LEGEND

- LEGEND
 Tetra Tech sonic testhole to 3 m
 Tetra Tech hydrovac testhole to 3 m
 Tetra Tech sonic testhole to 15 m
 Tetra Tech sonic testhole to 50 m
 Tetra Tech sonic testhole to 60 m
 Tetratech iBPT to 50 m
 Colder testhole legetinge

- Golder testhole locations
- McIntosh-Lalani testhole locations

- NOTES 1. Imagery from Google Earth Pro. Testhole locations on this image are approximate. See report for coordinates.

ISSUED FOR USE

100m



Scale: 1:2,000 @ 11"x17"



KICKING HORSE RIVER BRIDGES REPLACEMENT

BRITISH COLUMBIA Ministry of Transportation and Infrastructure

TESTHOLE LOCATION PLAN

— Figure 1
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PHOTOGRAPHS

Phase 1

Photo 1 to Photo 28 Sonic Core Photographs

Phase 2

Photo 1 to Photo 44 Sonic Core and Spilt Spoon Sample Photographs



PHASE 1



Photo 1. BH21-03, Box 1, 0.00 to 3.05 m



Photo 2. BH21-03, Box 2, 3.05 to 6.10 m





Photo 3. BH21-03, Box 3, 6.10 to 10.67 m



Photo 4. BH21-03, Box 4, 10.67 to 15.24 m



Photo 5. BH21-03, Box 5, 15.24 to 19.81 m



Photo 6. BH21-03, Box 6, 19.81 to 22.86 m





Photo 7. BH21-03, Box 7, 22.86 to 25.91 m



Photo 8. BH21-03, Box 8, 25.91 to 28.96 m





Photo 9. BH21-03, Box 9, 28.96 to 32.00 m



Photo 10. BH21-03, Box 10, 32.00 to 35.05 m





Photo 11. BH21-03, Box 11, 35.05 to 38.10 m



Photo 12. BH21-03, Box 12, 38.10 to 41.50 m



Photo 13. BH21-03, Box 13, 41.50 to 44.20 m



Photo 14. BH21-03, Box 14, 44.20 to 47.24 m



Photo 15. BH21-03, Box 15, 47.24 to 50.29 m



Photo 16. BH21-03, Box 16, 50.29 to 53.34 m





Photo 17. BH21-03, Box 17, 53.34 to 56.39 m



Photo 18. BH21-03, Box 18, 56.39 to 59.44 m



Photo 19. BH21-03, Box 19, 59.44 to 62.48 m



Photo 20. BH21-03, Box 20, 62.48 to 64.01 m



Photo 21. BH21-05, Run 1, 0.00 to 1.52 m



Photo 22. BH21-05, Run 2, 1.52 to 3.05 m





Photo 23. BH21-08, Run 1, 0.00 to 1.52 m



Photo 24. BH21-08, Run 2, 1.52 to 3.05 m





Photo 25. BH21-09, Run 1, 0.00 to 3.60 m



Photo 26. BH21-09, Run 2, 3.60 to 8.25 m



Photo 27. BH21-09, Run 3, 8.25 to 14.00 m



Photo 28. BH21-09, Run 4, 14.00 to 15.24 m

PHASE 2



Photo 1. BH21-01, SPT-01, 3.0 m to 3.5 m



Photo 2. BH21-01, Box 1, 3.0 m to 4.5 m





Photo 3. BH21-01, SPT-02, 4.5 m to 5.0 m



Photo 4. BH21-01, Box 2, 4.5 m to 7.5 m



PROJECT: KARB LOCATION: GOlden BC. TESTHOLE BH21-01 DEPTH: 6.0-6.5M SPT# 704-ENG. VGED 03783-01

Photo 5. BH21-01, SPT-03, 6.0 m to 6.5 m



Photo 6. BH21-01, SPT-04, 7.5 m to 8.0 m



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Photo 7. BH21-01, Box 3, 7.5 m to 10.5 m



Photo 8. BH21-01, SPT-05, 9.0 m to 9.5 m





Photo 9. BH21-01, SPT-06, 10.5 m to 11.0 m



Photo 10. BH21-01, Box 4, 10.5 m to 13.5 m


PROJECT: KI KB LOCATION: GOIDEN, BC. TESTHOLE BH21-01 DEPTH: 12-12.5m SPT#: 25,28,27 704 ENG. VGED 037 3-01

Photo 11. BH21-01, SPT-07, 12.0 m to 12.5 m

PROJECT: KIRB LOCATION: GOIDEN BC. TESTHOLE BH21- OI DEPTH: 13.5-14.0m SPT#: 30,30,45 704. ENG. VC=0 037 -Zerr.

Photo 12. BH21-01, SPT-08, 13.5 m to 14.0 m





Photo 13. BH21-01, Box 5, 13.5 m to 16.5 m



Photo 14. BH21-01, SPT-09, 15.0 m to 15.5 m





Photo 15. BH21-01, Box 6, 16.5 m to 19.5 m



Photo 16. BH21-01, SPT-10, 18.0 m to 18.5 m





Photo 17. BH21-01, Box 7, 19.5 m to 21.0 m

PROJECT: KHRS LOCATION: Golden, BC TESTHOLE BH21-01 DEPTH: 21.5m SP fusal (50 bla IN 5" 704-ENG VIGEO 03793-01

Photo 18. BH21-01, SPT-11, 21.0 m to 21.5 m





Photo 19. BH21-01, Box 8, 21.0 m to 24.0 m



Photo 20. BH21-01, SPT-12, 24.0 m to 24.5 m





Photo 21. BH21-01, Box 9, 24.0 m to 27.0 m

PROJECT: KHKB LOCAMON: GOLDEN, & TESTHOLE BH21-01 DEPTH: SPTH: 27.0 m to 27.5 REFUSAL (SO blows For 5-) 704-ENG NOED 03-TOG-01	

Photo 22. BH21-01, SPT-13, 27.0 m to 27.5 m





Photo 23. BH21-01, Box 10, 27.0 m to 29.8 m



Photo 24. BH21-01, SPT-14, 30.5 m to 31.0 m





Photo 25. BH21-01, Box 11, 30.5 m to 33.5 m

26 72 73 74 75 76 77 78 79 71 - 72 73 74 75 76 77 78 79	Contract of the second se	
	PRAJECT: KHRB LOCATION: Golden, BC TESTHOLE BH21-01 DEPTH: 33.5-34.0m	
	26,28,16	
	NOSAMPLE	

Photo 26. BH21-01, SPT-15, 33.5 m to 34.0 m





Photo 27. BH21-01, Box 12, 33.5 m to 36.6 m

	PROJECT. KHRB
and the second	LOCATION Golden, BC
and the second	TESTHOLE BH21-01
A A	DEPTH: 36.6-37.1m
	SPT#:
	Refusal (50 tor 5")
	704-FNG- 3500 D3CTCC34

Photo 28. BH21-01, SPT-16, 36.6 m to 37.1 m





Photo 29. BH21-01, Box 13, 36.6 m to 39.6 m



Photo 30. BH21-01, SPT-17, 39.6 m to 40.1 m





Photo 31. BH21-01, Box 14, 39.6 m to 42.7 m



Photo 32. BH21-01, SPT-18, 42.7 m to 43.2 m





Photo 33. BH21-01, Box 15, 42.7 m to 45.7 m



Photo 34. BH21-01, SPT-19, 45.7 m to 46.2 m





Photo 35. BH21-01, Box 16, 45.7 m to 48.8 m



Photo 36. BH21-01, SPT-20, 48.8 m to 49.3 m





Photo 37. BH21-01, Box 17, 48.8 m to 51.8 m

	PROJECT: KHRB LOCATION: GOLDUN, BC TESTHOLE BH21-01 DEPTH: SPTH: 51.8-52.3m	
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		- Color
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Photo 38. BH21-01, SPT-21, 51.8 m to 52.3 m





Photo 39. BH21-04, Box 1, 0.1 m to 3.0 m



Photo 40. BH21-04, SPT-01, 1.5 m to 2.0 m



PROJECT: KHEB LOCATION: Goiden, BC TESTHOLE BH21-04 0 DEPTH: 30m-3.5m SPT#: 10,26,27 704-ENG VGPA 03703-01

Photo 41. BH21-04, SPT-02, 3.0 m to 3.5 m





Photo 42. BH21-06, Box 1, 0.1 m to 3.0 m

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	PROJECT: KHRB	
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A CARLES AND CONTRACTOR		

Photo 43. BH21-06, SPT-01, 1.5 m to 2.1 m



PROJECT: KHRB LOCATION: Golden, BC TESTHOLE BH21-06 DEPTH: 3.0-3.5m SPT# 27, 26, 26 704-ENG. VGE0 0379. 21 0 2 ç

Photo 44. BH21-06, SPT02, 3.0 m to 3.5 m



APPENDIX A

LIMITATIONS ON THE USE OF THIS DOCUMENT



GEOTECHNICAL

1.1 USE OF DOCUMENT AND OWNERSHIP

This document pertains to a specific site, a specific development, and a specific scope of work. The document may include plans, drawings, profiles and other supporting documents that collectively constitute the document (the "Professional Document").

The Professional Document is intended for the sole use of TETRA TECH's Client (the "Client") as specifically identified in the TETRA TECH Services Agreement or other Contractual Agreement entered into with the Client (either of which is termed the "Contract" herein). TETRA TECH does not accept any responsibility for the accuracy of any of the data, analyses, recommendations or other contents of the Professional Document when it is used or relied upon by any party other than the Client, unless authorized in writing by TETRA TECH.

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The Professional Document and any other form or type of data or documents generated by TETRA TECH during the performance of the work are TETRA TECH's professional work product and shall remain the copyright property of TETRA TECH.

The Professional Document is subject to copyright and shall not be reproduced either wholly or in part without the prior, written permission of TETRA TECH. Additional copies of the Document, if required, may be obtained upon request.

1.2 ALTERNATIVE DOCUMENT FORMAT

Where TETRA TECH submits electronic file and/or hard copy versions of the Professional Document or any drawings or other project-related documents and deliverables (collectively termed TETRA TECH's "Instruments of Professional Service"), only the signed and/or sealed versions shall be considered final. The original signed and/or sealed electronic file and/or hard copy version archived by TETRA TECH shall be deemed to be the original. TETRA TECH will archive a protected digital copy of the original signed and/or sealed version for a period of 10 years.

Both electronic file and/or hard copy versions of TETRA TECH's Instruments of Professional Service shall not, under any circumstances, be altered by any party except TETRA TECH. TETRA TECH's Instruments of Professional Service will be used only and exactly as submitted by TETRA TECH.

Electronic files submitted by TETRA TECH have been prepared and submitted using specific software and hardware systems. TETRA TECH makes no representation about the compatibility of these files with the Client's current or future software and hardware systems.

1.3 STANDARD OF CARE

Services performed by TETRA TECH for the Professional Document have been conducted in accordance with the Contract, in a manner consistent with the level of skill ordinarily exercised by members of the profession currently practicing under similar conditions in the jurisdiction in which the services are provided. Professional judgment has been applied in developing the conclusions and/or recommendations provided in this Professional Document. No warranty or guarantee, express or implied, is made concerning the test results, comments, recommendations, or any other portion of the Professional Document.

If any error or omission is detected by the Client or an Authorized Party, the error or omission must be immediately brought to the attention of TETRA TECH.

1.4 DISCLOSURE OF INFORMATION BY CLIENT

The Client acknowledges that it has fully cooperated with TETRA TECH with respect to the provision of all available information on the past, present, and proposed conditions on the site, including historical information respecting the use of the site. The Client further acknowledges that in order for TETRA TECH to properly provide the services contracted for in the Contract, TETRA TECH has relied upon the Client with respect to both the full disclosure and accuracy of any such information.

1.5 INFORMATION PROVIDED TO TETRA TECH BY OTHERS

During the performance of the work and the preparation of this Professional Document, TETRA TECH may have relied on information provided by third parties other than the Client.

While TETRA TECH endeavours to verify the accuracy of such information, TETRA TECH accepts no responsibility for the accuracy or the reliability of such information even where inaccurate or unreliable information impacts any recommendations, design or other deliverables and causes the Client or an Authorized Party loss or damage.

1.6 GENERAL LIMITATIONS OF DOCUMENT

This Professional Document is based solely on the conditions presented and the data available to TETRA TECH at the time the data were collected in the field or gathered from available databases.

The Client, and any Authorized Party, acknowledges that the Professional Document is based on limited data and that the conclusions, opinions, and recommendations contained in the Professional Document are the result of the application of professional judgment to such limited data.

The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this document, at or on the development proposed as of the date of the Professional Document requires a supplementary exploration, investigation, and assessment.

TETRA TECH is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.



1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, TETRA TECH has not been retained to explore, address or consider and has not explored, addressed or considered any environmental or regulatory issues associated with development on the subject site.

1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems, methods and standards employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historical environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional exploration and review may be necessary.

1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

Construction activity can impact structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques, and construction sequence are known.

1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, and the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

1.15 DRAINAGE SYSTEMS

Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function. Where temporary or permanent drainage systems are installed within or around a structure, these systems must protect the structure from loss of ground due to mechanisms such as internal erosion and must be designed so as to assure continued satisfactory performance of the drains. Specific design details regarding the geotechnical aspects of such systems (e.g. bedding material, surrounding soil, soil cover, geotextile type) should be reviewed by the geotechnical engineer to confirm the performance of the system is consistent with the conditions used in the geotechnical design.

1.16 DESIGN PARAMETERS

Bearing capacities for Limit States or Allowable Stress Design, strength/stiffness properties and similar geotechnical design parameters quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition used in this report. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions considered in this report in fact exist at the site.

1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.

1.18 APPLICABLE CODES, STANDARDS, GUIDELINES & BEST PRACTICE

This document has been prepared based on the applicable codes, standards, guidelines or best practice as identified in the report. Some mandated codes, standards and guidelines (such as ASTM, AASHTO Bridge Design/Construction Codes, Canadian Highway Bridge Design Code, National/Provincial Building Codes) are routinely updated and corrections made. TETRA TECH cannot predict nor be held liable for any such future changes, amendments, errors or omissions in these documents that may have a bearing on the assessment, design or analyses included in this report.

APPENDIX B

GEOTECHNICAL TESTHOLE LOGS



MOTI SOIL CLASSIFICATION TABLE

Major	Divisions	Symbol	Soil Type									
	d oils	GW	Well-graded gravels or gravel-sand mixtures, little or no fines									
oils	el an y Sc	GP	Poorly-graded gravels or gravel-sand mixtures, little or no fines									
d Sc	avell	GM*	Silty gravels, gravel-sand-silt mixtures									
ine	бË	GC*	Clayey gravels, gravel-sand-clay mixtures									
Gra	ls Is	SW*	Nell-graded sands or gravelly sands, little to no fines									
arse	Soi	SP	Poorly-graded sands or gravelly sands, little or no fines									
Cõ	Sand	SM*	Silty sands, sand-silt mixtures									
	ů ů	SC*	Clayey sands, sand-clay mixtures									
s	nd _<50	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity									
d Soil	Silts a ays Ll	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays									
ine	ū	OL	Organic silts and organic silt-clays of low palsticity									
le Gra	and .L>50	МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts									
Fir	ilts a ys L	СН	Inorganic clays of high plasticity, fat clays									
	S Cla	он	Organic clays of medium to high plasticity, organic silts									
Orga	anic Soils	Pt	Peat and other highly organic soils									
т	opsoil	TS	Topsoil with roots, etc.									
С	obbles	SB	Rock fragments and cobbles, particle size 75mm to 300mm diameter									
В	oulders	LB	Boulders, particle size over 300mm in diameter									
	*GP-GM ; G	P-GC; SP-SM;	SP-SC; 6-12% Passing #200 (0.075mm) Sieve									
	* GM1; GC	C1; SM1; SC	1; 12-20% Passing #200 (0.075mm) Sieve									
	* GM2; GC	2; SM2; SC	2; 20-30% Passing #200 (0.075mm) Sieve									
	* GM3; GC	03; SM3; SC	3; 30-40% Passing #200 (0.075mm) Sieve									
	* GM4; GC	C4; SM4; SC4	4; 40-50% Passing #200 (0.075mm) Sieve									

SOIL CLASSIFICATION

MoTI Soil Classification table from the document *Notes for Completion of Field Logs* (MoTI, 2016) uploaded to the MoTI website on September 14, 2016. This table was originally based on the MoTI *Geotechnical and Materials Engineering Standards for Bridge Foundation Investigations* (MoTI, 1991).



WOTI-SOIL-REV3 ENG.VGE003793-01 - KHRB (PHASE 1-2) 20220502.GPJ MOTI_DATATEMPLATE_REV3.GDT









	Sample Type:		ab ala		S -Sp	olit		- - //) -Oc	dex oter	۸ ۱		W -Wa	ash retur	<u>ت</u> ∏ ("		Shelb	y		Depth to Top of F	Rock: of 6
]ה	Legen	<u>d</u> A -Ai	uger	Π	B-Be	ecke	гП	Πc	-Co	re			G-Gr	ab	Г	_v-\	/ane			Final Depth of Hole: 52	.3 m
REV3 EN(- 60				•••																
G.VGE003793																					730
-01 - KHRB	-																				
(PHASE 1-2	-58																				731
) 20220502.(····						····									732
	- - - - -						 					 									
					•••	 						 									733-
ATE_REV3.G					•••		····			••••		· · · · · · · · · · · · · · · · · · ·						with BC Groundwater Protection regulations.			
DT 5/3/22								••••			· · · ·	÷						using a 63.5 kg (140 lb) SPT hammer. - Upon completion, the borehole was tremie grouted to 1.5 m depth with cement-bentonite grout and topped with bentonite chips to surface in accordance			734
	-54				•••							· · · · ·						combination with laboratory testing. Some variation through the interpreted soil layers is expected. - All SPTs in this borehole were completed			735-
	-53						····	••••			· · · ·	÷						elevations (+/- 0.03 m) are based on points collected from a survey completed by Geometric Surveys. - Soil descriptions are based on visual classifications and field observations in			736-
	- - - - -														ľ			End of borehole (target depth reached) at 52.3 m			
	-52	27 34 50		2	0		····			••••		. 84	•••••		G2:	3 21 10	0	SILT and SAND, low plastic, fine to medium sand, brown to grey, non-cohesive, V < PL, very stiff to hard - layer of fine to medium sand from 51.8 m to 52 0 m	ML	Sieve (Sa#G23) G:% S:% F:59%	737-
	51										•							51 5m			738
	- UC				•••		····					· · · · · · · · · · · · · · · · · · ·	.,					SAND and SILT, poorly graded, fine to medium sand, brown to grey, non-cohesive, moist to wet, dense to very dense (continued)			
	DEPTH (m)	DRILLING DETAILS		10 W _P 2	SP % 0 ⊢	7 "N	200 I" (Bl 40	LOV W%	300 400 WS/300 mm) ▲ WL% 60 WL%							RECOVERY (%	SOIL SYMBOL	SOIL DESCRIPTION	CLASSIFICATION	COMMENTS TESTING	ELEVATION (n
	Logge	ed by: KB	Re XP	eviev ocket	ved Per	by: netro	TO	er 🎗	Ele Sh	vati ear	on: Stre	78 ngth	39.0 (kPa	3 m) ш		(%	1_	Coordinates Surveyed September 17, 2021	Drill	ling Method: Sonic	Ê
	Prepa	ared by: 70 Tetra Tec	4-EN h Ca	G.VG anad	EO a In	0379 IC	3-01		Dat Nor	tum rthir	: U ng/E	TM Easti	11 (ng:	NAE 568	83) 3077	7.296	6,5	Alignment: 02544.763 Station/Offset:	Drill Drill	ler: Make/Model: Fraste Multidrill >	۲L
		5							Pro Loc	oje atio	ct: on:	Hi Gol	ghv den,	vay BC	95 - Ea	Kic st of	kin 10t	g Horse River Bridges h Ave N, north of river	Dat Cor	e(s) Drilled: 09/15/2021 to 09/1 npany: Mobile Augers and Res	7/2021 earch
TETRA TECH																Drill Hole #: BH21-01					



																		Drill Hole #: BH21-03						
	Ŧŧ	IE		K A		EC	н		Pi	roje	ect	: F	ligh	Wa	ay	95 I	Kic	kin	ıg	Horse River Bridges	Dat	Date(s) Drilled: 3/9/2021 to 3/12/2021 Company: Mobile Augers and Research		
Pre	epared	by: 70	4-EN	G.V	GEC	037	93-0)1	Da	atum	n: l	UTN	/ 11	(N/	AD8	- GO 33)	uia	S IS	lar	Alignment:	Dril	ller:	1	
	Tetr	a Tec	h Ca	ana	da lı	nc			No	orthi	ing/	Eas	sting	: 5	683	3006	.74	5,5	502	2559.621 Station/Offset:	Dril	I Make/Model: Fraste Multidrill XL		
Lo	gged by	y: CR	Re	evie locke	wec et Pe	l by: netro	: TI ome	M ter 3	Ele XS	evat hear	tion Str	: enat	788. h (kP	79 Pa)	m		5	2		Coordinates Surveyed March 18, 2021	Dril	ing Method: Sonic		
DEPTH (m)	DRILLING	DETAILS		1 W	100 ▲ SF P% 1	די די	200 \\" (E 40	BLO	3 WS/	00 /300 50	mm —I	400 ()▲ WL 9	/0		SAMPLE TYPE	SAMPLE NO	RECOVERV (%			SOIL DESCRIPTION	CLASSIFICATION	COMMENTS NO TESTING F		
- 10		2 7 11		11	8		·····			······································	· · · · · · · · · · · · · · · · · · ·			····	X	SPT7	7	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		SAND and GRAVEL, medium to coarse sand, well graded, fine to coarse subrounded to rounded gravel, some silt, trace subrounded to rounded cobbles up to 90 mm in diameter, light orange-brown, iron staining on gravel, cobbles, and in the matrix, non-cohesive, wet, dense. (<i>continued</i>) -becomes trace silt at 10.7 m		Sieve (Sa#SPT7) G:37% S:53% F:10% 77		
- 12 - - - - - - - - - - - - - - - - - - -	·	8 11 9			20		· · · · · · · · · · · · · · · · · · ·							····	X	SPT8	51				SW- SM / GW- GM	77		
14 14 14 15							•••••			· · · · · · · · · · · · · · · · · · ·	• • • •			••••						SILTY GRAVEL and SAND, fine to coarse		77	74	
ион рататемист – кеvз.		9 13 11	•••		24		••••••			•	• • • •				X	SPTS	7	0		subrounded to rounded gravel, well graded, medium to coarse sand, trace subrounded to rounded cobbles up to 90 mm in diameter, yellow-brown, iron staining on gravel, cobbles, and in the matrix, non-cohesive, wet, dense.		Sieve (Sa#SPT9) G:41% S:41% F:18% 77		
1492E 1-2) 20220902:GPJ 1 1 - 1 - 1 - 1 - 1 - 1 1 - 1 - 1 - 1 -							•••••••			· · · · · · · · · · · · · · · · · · ·	•			••••						-some cobbles from 16.8 to 17.6 m, up to 100 mm diameter	CMI	77	'2	
КЕV3 ЕNG.VGEOU3/93-01 - МПКВ (Г 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		5 9 10		1	9				·····	•				۲ ۲	X	SPT1	4				GM1 / SM1	77 76		
	gend [A-A	uger	Ţ	B -B	ecke	er [Ī	C -C	ore			G -0	Grab)	L]v-'	Vane	,			Final Depth of Hole: 64.0 m	١	
	Type: L#-Lab S-Split O-Odex Sample Spoon (air rotary)								ore ora Odex W-Wash rotary) (mud return)												Depth to Top of Rock: Page 2 of 7	: 7		



		<u> </u>								SU	IMMARY LOG		Drill Hole #: BH21-03		
		E TE		ECH	Projec	t: High	nway	95 I	Kick	king	Horse River Bridges	Date	ate(s) Drilled: 3/9/2021 to 3/12/2021		
	Prepar	red by: 704	I-ENG.VGEC	03793-01	Location Datum:	UTM 11	n, BC	- Go 083)	uld's	Islai	nd Alignment:	Con Drill	Driller:		
	T	etra Tech	n Canada lı	C	Northing	g/Easting	j: 568	3006	.745	, 50	2559.621 Station/Offset:	Drill	I Make/Model: Fraste Multidrill XL		
	Logge	d by: CR	Reviewed X Pocket Pe	by: TM	Elevatio	n: 788. trength (kF	.79 m Pa)		()		Coordinates Surveyed March 18, 2021	Drill	ling Method: Sonic	Ê	
	DEPTH (m)	DRILLING DETAILS	100 ▲ SF Wp% 20	200 PT "N" (BLC 40	300 DWS/300 mi %	400 ` m)▲ WL%	SAMPLE TYPI	SAMPLE NO	RECOVERY (%	SOIL SYMBOI	SOIL DESCRIPTION	CLASSIFICATION	COMMENTS TESTING	ELEVATION (n	
	_ 30										SILTY SAND, gravelly, medium to coarse ^{30.0m}			-	
	-31	7 19 16	St	35				SPT14	4		subrounded to rounded gravel, some subrounded to rounded cobbles up to 110 mm diameter, yellow-brown, non-cohesive, wet, very dense. <i>(continued)</i>		Sieve (Sa#SPT14) G:28% S:51% F:21%	758	
														757-	
	-32										-sand becomes fine to coarse at 32.4 m, some silt			- - - - -	
	-33													756	
												SM2		755	
77/2/2														754—	
AIE_KEV3.GL															
														753	
		17 26		31			\mathbf{X}	SPT1:	5 40		SAND, gravelly, fine sand, poorlv graded.			752-	
).ZUZUZUZUZ (-3/	5					····/	×			some silt, trace fine to coarse subrounded to rounded gravel, yellow-brown, non-cohesive, wet, dense. 37.5m	SP- SM			
5 (PHASE 1-2											some silt, trace fine to coarse subrounded to rounded gravel, yellow-brown, non-cohesive, moist, dense.	SP- SM		751-	
193-01 - KHKI											GRAVEL, sandy, fine to coarse subrounded to rounded gravel, well graded, fine to coarse sand, some silt, yellow-brown, non-cohesive, wet, dense.	GW- GM	-	750	
3 ENG.VGEOUS	-39	5	16					7			SILTY SAND, fine to medium sand, well graded, trace fine to coarse subrounded to rounded gravel, yellow-brown, non-cohesive, compact. -becomes trace gravel at 39.4 m	SM3			
> ЧЧ ЧЧ	- 40	9	17:	<u> </u>	· · · ·		/`	SPT1	6 90		39.9m		Sieve (Sa#SPT16)	0	
	<u>Legend</u> Sample Type:	A -Au [] ! ■ L# -Li	ger ∐B -B ab ⊠S -S	ecker 🚺 plit	C-Core	G-(Grab Wash] V -Va]T-Si	ane nelby			Depth of Hole: 64	.0 m Rock: of 7	
≥		oam - Sam	ne i spo		- (ali i0taiy)	====== (I])(uu retul	ш) ш и		5			raye 4		



	TETRA TECH											Drill Hole #: BH21-03							
		b]™	TF		EC	н	Pro	ject	: Hig	ghw	ay	95 H	Kicł	king	Horse River Bridges	Date	ate(s) Drilled: 3/9/2021 to 3/12/2021		
-	Prepa	red by: 70	4-FN	G VGF	00379	3-01	Loca Datu	ation um:	: Gol UTM	den, 11 (N		- Goi 83)	uld's	s Isla	nd Alianment:	Con Drill	npany: Mobile Augers and Rese er:	earch	
	• -	Tetra Tec	h Ca	anada	nc		Nort	hing	/Easti	ng:	568	3006	.745	5,50	2559.621 Station/Offset:	Drill	Drill Make/Model: Fraste Multidrill XL		
	Logge	d by: CR	R		d by:	TM	Elev	atior	n: 78	38.79) m				Coordinates Surveyed March 18, 2021	Drill	ing Method: Sonic		
	DEPTH (m)	DRILLING DETAILS	X F	100 ▲ S W _P % 20	PT "N	1" (BLC 40	X Sne 300 0WS/30	00 mm	engtn 400 n) ▲ WL %	(KPa)	SAMPLE TYPE	SAMPLE NO	RECOVERY (%	SOIL SYMBOL	SOIL DESCRIPTION	CLASSIFICATION	COMMENTS TESTING	ELEVATION (m)	
E	50									-								-	
	51					·····					-				50.5m SILTY SAND, fine sand, uniformly poorly graded, grey, non-cohesive, moist to wet, compact.			738	
	52	7 6 8		14								SPT20	0 100			SM2		737	
	53					·····					•				-grey silt inclusion, 30 mm diameter			736	
	54					·····					•				-bed of fine to medium sand from 53.45 to 53.65 m -dark grey silt inclusion, 40 mm diameter SAND and SILT, fine sand, uniformly poorly			735	
3.601 5/3/22	55											SH21	100		graded, grey, non-cohesive, moist to wet, compact. -attempted to take a Shelby tube sample from 54.86 to 55.32 m - no recovery	SM3		734-	
AIEMPLAIE_KEV	56	6 8 10		18 22		÷						SPT22	2 100		56.1m		Sieve (Sa#SPT22) G:0% S:65% F:35%	733	
	57		•••••							· · · · · ·	•				SANDY SIL I, low plastic silt, fine sand, uniformly poorly graded, grey, cohesive, wet, very stiff.	ML		732-	
						· · ·									57.1m SAND and SILT, fine sand, uniformly poorly graded, low plastic silt, grey, occasional dark grey silt inclusions up to 20 mm in diameter, non-cohesive, wet, compact.			731	
	58	4 6 13		19 2		·····						SPT2	3 100		-green-grey lamination of silt, 40 mm in thickness		Sieve (Sa#SPT23) G:% S:% F:46%		
	59				•••••	·····									green-grey lamination of slit, 100 mm in thickness			7 30 - - - - - - - - - 720-	
Ϋ́́	60			<u> </u>	:	:	<u> </u>	:	:	<u>:</u>							Final Donth of Llalas 04	0 m	
	<u>egend</u> ∭A-Auger ∭B-Becker Sample ∫ype: Sample Spoon				r 🛄	C -Core O -Ode (air rot	e ex tary)		G -Gra N -Wa <u>mu</u> d	ib sh returi	 	V- V T-S Tub	'ane helby be			Depth of Hole: 64	.0 m lock: of 7		

				SU	Drill Hole #: BH21-03															
	TE TE	TRA TECH	Project: Highway	95 Kicking	Horse River Bridges	Date(s) Drilled: 3/9/2021 to 3/12/2021														
			Location: Golden, BC -	- Gould's Islar	nd	Company: Mobile Augers and Research														
Prep	Tetra Tec	04-ENG.VGE003793-01 ch Canada Inc	Datum: UTM 11 (NAD8	83) 8006 745 502	Alignment: 2559 621 Station/Offset	Driller: Drill Make/Model: Fraste Multidrill XL														
Log	aed by: CR	Reviewed by: TM	Elevation: 788.79 m	, 000.7 40 , 002	Coordinates Surveyed March 18, 2021	Drilling Method: Sonic														
		× Pocket Penetrometer	r X Shear Strength (kPa) Ш	0L 0L		NO														
E T	ULS NLS	100 200		MBC NO	SOIL	COMMENTS O														
LT L	∎ ETALL			MPL - SΥ	DESCRIPTION															
B				SAI		CLAS														
- 60		20 40			SAND and SILT, fine sand, uniformly poorly															
Ē					graded, low plastic silt, grey, occasional dark grey silt inclusions up to 20 mm in															
-					diameter, non-cohesive, wet, compact.	SM4														
Ē					(continued)	728														
- 61																				
Ē																				
-																				
-62						121-														
Ē					some interhedding between more eilt, and															
-		<u>-</u>			sand-rich layers from 62.2 m to 64.0 m															
Ē						726-														
-63																				
-																				
Ę																				
Ē						725														
-64					End of borehole (target reached) at 64.0 m															
Ē					- Coordinates and ground surface elevations (+/- 0.03 m) are based on points															
					collected from a survey completed by															
					- Soil descriptions are based on visual	724-														
					classifications and field observations, in combination with laboratory testing. Some															
					variation through the interpreted soil layers is expected.															
					- All SPTs' in this hole were completed	723														
-66					- Upon completion, the borehole was															
					backfilled with bentonite grout to surface.															
			••••••																	
2						722-														
⁵ -67																				
						721–														
<u>-</u> -																				
2 – 69						120-														
>. -																				
2						719-														
2 70	<u>nd</u> [[5] <u>^</u> ^					Final Depth of Hole: 64.0 m														
Sam	<u>, u</u> ⊿l ^{a-A} ≣ ≣L# -I	Lab 578 -Split F	<u>ט-י-טופי</u> שוש-טופט שוש-U-U-U-U-U-U-U-U-U-U-U-U-U-U-U-U-U-U-	T-varie		Depth to Top of Rock:														
	San San	nple	(mud return			Page 7 of 7														
			_		_											50	IMMARY LOG		Drill Hole #: BH2'	1-04
---	--	----------------------------------	------------	---------------------------------------	-----------------	---------------	--------------------	-------------------------	---------------	---	---------	----------------	-------------	------------------	--------------------	-------------	---	----------------	--	---
-			TR	Α.	ΓĒ(СН		Pro	ojec	ct:	Hig	hwa	iy i	95 K	(ick	ing	Horse River Bridges	Dat	e(s) Drilled: 09/14/2021	
-	Prepare	ed bv: 704	4-EN	G.VG	EO03	3793-0	01	Loc	atio	n: (UT	Solde	en, B 1 (N/	AD8	· 10th 33)	I AV	϶N,	north of river Alianment:	Cor Drill	npany: Mobile Augers and Reseler:	earch
	Т	etra Tecl	h Ca	anada	Inc			Nor	thin	g/Ea	astinę	g: 56	683	262.	233	, 502	2408.265 Station/Offset:	Drill	Make/Model: Fraste Multidrill X	(L
	Logged	by: KB	Re	eview	ed b	y: T	G	Elev	vatio	on:	786	.65 r	m				Coordinates Surveyed September 17, 2021	Drill	ling Method: Sonic	
	DEPTH (m)	DRILLING DETAILS	A P	0000000000000000000000000000000000000		200 "N" (I	BLOV	300 300 WS/31	0 00 m	10 10 10 10 10 10 10 10 10 10 10 10 10 1	0 %		SAMPLE TYPE	SAMPLE NO	RECOVERY (%	SOIL SYMBOL	SOIL DESCRIPTION	CLASSIFICATION	COMMENTS TESTING	ELEVATION (m
	-1 -2 -3 -4 -5 -6 -7 -7 -8	18 12 12 12 12 12	P		24	40		<u>600</u>			D 			G1 G2 SPT2	100		ASPHALT (130 mm thick) 0.1m GRAVEL, sandy, gap graded, fine to coarse, subangular to subrounded gravel up to 60 mm in diameter, fine sand, some cobbles up to 100 mm in diameter, some silt to silty, light brown, non-cohesive, dry, compact to dense End of borehole (target depth reached) at 3.5m - Coordinates and ground surface elevations (+/- 0.03 m) are based on points collected from a survey completed by Geometric Surveys. - Soil descriptions are based on visual classifications and field observations, in combination with laboratory testing. Some variation through the interpreted soil layers is expected. - All SPTs in this borehole were completed using a 63.5 kg (140 lb) SPT hammer. - Upon completion, the borehole was backfilled with sand and bentonite chips in accordance with BC Groundwater Protection regulations. The road section was reinstated with cold patch asphalt	GW- GM	Sieve (Sa#SPT1) G:55% S:34% F:11% Sieve (Sa#SPT2) G:% S:% F:16%	ш 786 785 784 783 783 782 782 782 782 782 782 782 782 782
	10										,									777-
	Legend Sample	A-Au	ıger	∏ E	B-Bec	ker	0	C-Cor	re		G-	Grab			V-Va	ane			Final Depth of Hole: 3	6.5 m
	Type:	Sam	ab ple		S-Spli Spoor		Ξ_{0}^{\prime}	0 -Od (air ro	lex otary)		W (m	Wash ud re	า turn) [[[]	T-Sh Tube	nelby Ə			Page 1	of 1

																	รเ	IMMARY LOG		Drill Hole #: BH2	1-05
	T	ย™	TR	A	TE	C	Н	ſ	Pro	oje	ct:	Hi	ghv	/ay	95	Kic	kinę	Horse River Bridges	Date	ə(s) Drilled: 3/13/2021	
	Dranar	ad by 70		0.1/0		0070	22.04	4	Loc	atic	on:	Gol	den,	BC	- 101	h A	/e N	and Park Drive	Con	pany: Mobile Augers and Rese	earch
	Ргераг Т	ed by. 70 etra Tec	4-EN h Ca	anad	a In	0373 IC	93-0	'	Noi	rthir	. U 1a/E	East	ina:	VAL 568	03) 3377	'.47 <u></u> {	5.50	2346.306 Station/Offset:	Drill	er. Make/Model: Fraste Multidrill X	۲L
	Logged	d by: CR	Re	eviev	ved	by:	ТМ	Л	Ele	vati	on:	7	36.3	7 m			,	Coordinates Surveyed March 18, 2021	Drill	ing Method: Sonic	
	_	_	XP	ocket	t Per	netro	omet 200	er >	Sh 30	iear (Stre	ngth 100	(kPa)	Ш	0	(%)	DL		N		(L)
	ш т	ILS								-				Ł	Ž Ш	РЧ	MB	SOIL	CATIC	COMMENTS	NO
	E					- "A								РГЕ	4PL	2 E	∽	DESCRIPTION	SIFIC	TESTING	ATI (
	B			WP	х SP %		и (В	W%	1V5/3	300 n	nm) V	▲ V_ %		BAM	SA	Ŭ			CLAS		
	- 0	Ш		2	0	:	40		60)		80	:				\mathbb{X}	SILTY SAND and GRAVEL (FILL), fine to			ш.
	-		• 5												G1		\otimes	coarse sand, well graded, fine to coarse	SM1	Sieve (Sa#G1) G:% S:% F:19%	786-
	-					 		•••••		••••	••••	 	· · · · ·		G2		\bigotimes	occasional rootlets, non-cohesive, moist,	<u>GM1</u> /		-
	-																	very dense. -inclusions of black tar from 0.48 to 0.53m	0.44		-
	-1		•							• • • •		 	•••••		G3		\otimes	SAND (POSSIBLE FILL), fine to coarse	SIVI I	Sieve (Sa#G3)	-
	-		2			-				:							\mathbb{X}	sand, well graded, some silt, trace fine to coarse subangular to subrounded gravel, 1.4m-		0.70 0.70 1.1270	785-
	-	26								••••	7	8		$\overline{\nabla}$			\otimes	light brown (dry), occasional rootlets,			
		38 40	3			-	-			:			-	Ň	SPT	1 85	'	SILTY SAND and GRAVEL (POSSIBLE	SM2	G:38% S:42% F:20%	-
	-2									••••							\otimes	FILL), fine to coarse sand, well graded, fine	/ GM2		
						-											\bigotimes	light grey-brown, non-cohesive, dry, very			784-
	-					-	ł	÷	i		:	÷	÷		64		\bigotimes	becomes light grey at 1.9 m			
	-3										; ;		.;				\otimes	SAND (POSSIBLE FILL), gravelly, fine to	SW		
	-	20 20				÷	41	÷			:	÷	÷	\mathbb{N}	SPT	2 80		subangular to subrounded gravel, trace silt,			
	-	21					Ę.				: : · · ·	÷		\square			×	dignt grey-brown (dry), non-conesive, dry, dense.			783-
	-											-						-becomes very dense at 3.1 m			:
	-4									••••		÷		ł				- Coordinates and ground surface			-
	-									:								elevations (+/- 0.03 m) are based on points collected from a survey completed by			700
N	-					 						÷	••••••	+				Geometric Surveys.			102-
21212						-	ł	÷	i	:	:		÷					classifications and field observations, in			
פר	-5							· · :	 	••••	•••• •	÷	·•••••	1				variation through the interpreted soil layers			
つ 2 二 二 イ	-					ŀ	ł	÷	i		:	÷	÷					is expected. - All SPTs in this hole were completed			781-
Ц	-									• • • • •		 		1				using a 140 lb SPT hammer.			
MPL																		backfilled with cuttings and bentonite chips			-
IAIE	-0					-	-	ł				ł	-]				to surface.			
	-																				780-
D ≥	-					-	:	:			:	:	÷								
GL2	-7					; ;					: 										
						-	÷	į	:		-	-									
702 (-					<u>.</u>					: 	÷		-							779-
Ч 	-					-		÷	i			-	-								
2HH2	-8					 					 	 		+							
n N	-					-	-	-	-			-	-								778-
-							·	· · .		••••		÷	·	1							
193-U	-					-		÷			:	÷	÷								
2002	-9							•••••••••••••••••••••••••••••••••••••••	: :	••••	•••• •	÷ :	·:	1							
א קר י ע קר	-					-	÷	÷	:		-	-									777-
			••••	•••••		 				••••	••••	÷		1							
л 2	- 10					-		ł			-										
	Legend Sample	A -Au	iger	Ţ	B -Be	ecke	er [C-Co	ore			G -Gra	ab	L] v -\	/ane			Final Depth of Hole: 3	.5 m
	Type:	Sam	ab ple	\boxtimes	S -Sp	olit	[0 -Oc	dex otarv	<i>)</i>		W -Wa (mud	ish retur	n) [[[] ⊺ -8	Shelby De			Depth to Top of R Page 1	OCK: of 1
21		_ Jun	r		- 10		_	-1	11 11	y	1		uu	. J.ul	.,		. •	1			

[SU	IMMARY LOG		Drill Hole #: BH21	-06
	Т	₽™	TF	RA	Т	EC	H		P	roje	ect:	H	ighv	vay	9	5 K	ick	ing	Horse River Bridges	Date	e(s) Drilled: 09/14/2021	
	Prepar	ed by: 70	4-EN	G.VC	GEO	037	93-0)1	Lo	atum	on: า: เ		11 (, BC NAE	- 1 083	0th 5)	AV	εN,	Alignment:	Drill	npany: Mobile Augers and Rese	earcn
	Т	etra Tec	h Ca	anad	la Ir	IC			No	orthi	ng/l	East	ing:	568	31	96.7	7 69	, 50	2455.388 Station/Offset:	Drill	Make/Model: Fraste Multidrill X	L
	Logged	d by: KB	Re XF	eviev ocke	wed t Pe	by netr	: T ome	G eter	El S	evat hear	ion: Stre	7 ength	87.0 (kPa	7 m) ш			()		Coordinates Surveyed September 17, 2021	Drill	ing Method: Sonic	Ê
	DEPTH (m)	DRILLING DETAILS		10 W _F 2	00 ▲ SF 20 Ⅰ	יד די	200 N" (E 40	BLO W	3 WS/	00 /300 50	 mm) 	400)▲ ₩L% 80		SAMPLE TYPI		SAMPLE NU	RECOVERY (%	SOIL SYMBOI	SOIL DESCRIPTION	CLASSIFICATION	COMMENTS TESTING	ELEVATION (n
	- 0					-					:			-					ASPHALT (110 mm thick) 0.1m	AS		787
	- - - - - - - - - - - - - - - - - - -	28 42	2	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	······································			••••			82				G1 PT1	100		SILTY GRAVEL, sandy, gap graded, fine to coarse, subangular to subrounded gravel up to 40 mm in diameter, fine sand, some cobbles up to 90 mm in diameter, light brown, non-cohesive, dry, dense	0.14	Sieve (Sa#G1) G:% S:% F:15%	786
	-2	42			: 				••••					. 🏳	4					GMT		- - 785
		27 26 26	2	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·			52	· · · · · · · · · · · · · · · · · · ·					, SI	32 PT2	100				Sieve (Sa#G2) G:64% S:23% F:13%	784
	-	20	••••		•••• •	 	·		••••	 				· K					End of borehole (target depth reached) at 3.5m			-
																			3.5 m			
REV3.GUI 3/3/22					· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·				· · · · · · · · · · · · · · · · · · ·									 Coordinates and ground surface elevations (+/- 0.03 m) are based on points collected from a survey completed by Geometric Surveys. Soil descriptions are based on visual classifications and field observations, in combination with laboratory testing. Some variation through the interpreted soil layers is expected. All SPTs in this borehole were completed 			783
				· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·				••••	· · · · · · · · · · · · · · · · · · ·									using a 63.5 kg (140 lb) SPT hammer. - Upon completion, the borehole was backfilled with sand and bentonite chips in accordance with BC Groundwater Protection regulations. The road section was reinstated with cold patch asphalt			
2.65	-7								••••													780
	- - - - - - - - - - - - - - - - - - -			· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·				••••	· · · · · · · · · · · · · · · · · · ·												- - - - - - - - - - - - - - - - - - -
	-			: 		 			••••													-
REV3 ENG.VGEOU3/33-UI	- - - - - - - - - - - - - - - - - - -			· · · · · · · · · · · · · · · · · · ·					••••													778
, , ,	<u>Legend</u> Sample	A -Au	uger	Ţ	B -B	eck	er [C -C	ore			G -Gr	ab	[]	V -Va	ane			Final Depth of Hole: 3.	.5 m
	Type:	Sam	.ab iple	\boxtimes	S -S Spo	plit on	[0 -C (air)dex rotar	y)		W-Wa (mud	ash retu	_{rn)} [T -Sł Tub	nelby e			Page 1 (ock: of 1

ſ																		รบ	IMMARY LOG		Drill Hole #: BH21	1-08
	T	t∣™	TR	A	TE	EC	Н	Ī	Pr	roje	ect:	H	ighv	vay	95	K	ick	ing	Horse River Bridges	Dat	e(s) Drilled: 3/13/2021	
	Dress			0.1/0		0070			Lo	cati	ion:	Go	lden,	BC	- 1)th	Ave	e N,	north of existing bridge	Cor	npany: Mobile Augers and Rese	earch
	Prepar T	Fetra Tec	4-EN h Ca	anad	a In	0373 IC	93-0	'	No	aturi orthi	ina/	Fas	tina [.]	568	295 295	6 0	36	50	2521 106 Station/Offset	Dril	ier. I Make/Model: Fraste Multidrill X	1
	Logge	d by: CR	Re	eviev	ved	by:	Т	и	Ele	evat	tion	: 7	'89.5	2 m				,	Coordinates Surveyed March 18, 2021	Dril	ling Method: Sonic	-
Ī			XP		t Per	netro	omet	ter	KS ⊲	hear	Stre	ength	n (kPa)	Щ	6	5	(%)	Ч		Z		(E)
	<u> </u>	ILS ILS					200		0	00		400		Έ		ź	ž	MBO	SOIL	ATIC	COMMENTS	NC
	Ë	≣TA ∎												Г	Ę		Ň	SΥ	DESCRIPTION	SIFIC	TESTING	ATIC
	DE	H H P		WP	⊾SP %	א" די	√" (B	SLO W9	WS/	300	mm)▲ Wi%	, D	AMI			ы Ш	GL		ILAS) U
ł	0	T		2	0	:	40		6	<u>30</u> :		<u>8</u> 0 :	·····	S	4 0	1	R	ω ····································		ASPI	t	<u> </u>
ł						-	-			-	-							\bigotimes	SAND (FILL), gravely, fine to coarse sand.	SW		-
ł	-		••••							÷		• • • •				2		\bigotimes	well graded, fine to coarse subangular to	GW		789-
ł	-					÷	÷	÷		÷	÷	į						\bigotimes	non-cohesive, moist, dense.	┝──	-	-
Ē	-1		••••							÷	•	• • • •	•••••••••					\bigotimes	GRAVEL (FILL), sandy, fine to coarse			-
ł	-					-	÷	į		-	÷	ł	ł		6	3		\bigotimes	graded, fine to coarse sand, trace silt, grey,	SM4		-
Ē	-	G	••••			÷	40			<u>.</u>	•	·	••••					\bigotimes	inclusions of tar, non-cohesive, moist,			788-
ł	-	23		• 14		ł	40	i		÷	÷	į	-	Х	SF	T1	85	\bigotimes	SAND and SILT (POSSIBLE FILL),		Sieve (Sa#SPT1)	-
ļ	-2	17	••••			: :		••••	• • • •	 :	•	•	· · · · · ·	t	ľ			X	gravelly, fine to coarse sand, well graded,		0.1070 0.1070 1.11270	-
ł	-					-	÷	÷		-	÷		-					\bigotimes	gravel, light grey-brown, occasional rootlets,			-
ł	-		••••					••••		÷	•	•	••••••••		6	4		\bigotimes	-becomes light brown at 1.8 m			787-
ł	-					-	ł	į		-	÷	ł	-					\bigotimes	GRAVEL (POSSIBLE FILL), cobbley,	GW		-
ł	-3		••••						 20		••••	••••						\bigotimes	subrounded gravel, well graded, fine to			-
Ē	-	18 50				: 50			20					X	SF	Т2	0	\bigotimes	coarse sand, subangular to subrounded cobbles up to 90 mm diameter trace silt			-
ł	-		••••	••••					• • • •		•	•		1	ľ			KXX 	non-cohesive, moist, dense.		-	786-
	-					÷	÷	į		÷	÷	÷	÷						End of borehole (target reached) at 3.5 m			-
ł	-4		••••							÷	••••	• • • •	••••••••						- Coordinates and ground surface			-
	-					-	÷	÷		-	÷	÷	ł						collected from a survey completed by			-
7	-		••••	••••		:		••••		÷	•	• • • •	•••••••						Geometric Surveys. - Soil descriptions are based on visual			785-
5/3						ł	÷	÷			÷	÷	ł						classifications and field observations, in			-
3	-5		••••				-												variation through the interpreted soil layers			-
<u>х</u>	-					-	÷	÷		-	÷	÷	÷						is expected. - All SPTs in this hole were completed			
⊔ 	-		••••																using a 140 lb SPT hammer.			/84
Ц МГ						:	÷			-	÷	÷	÷						backfilled with cuttings and bentonite chips,			-
IAE																			and capped with compacted cold mix asphalt.			-
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r J	-7					; ;																-
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Ĩ	i jpe.	Sam Sam	ple	\bowtie	Spo	on	ŀ	<u> </u>	(air i	rotar	ïy)	Ĭ	(mud	retu	n) L	Шı	rube	:0:0y 9			Page 1	of 1



		C			_													ç	SU	IMMARY LOG		Drill Hole #: BH21-09
		Ŀ	TR	A	TE	C	Н		P	roje	ect	:	lig	hw	ay	95	Ki	cki	ing	Horse River Bridges	Date	∋(s) Drilled: 3/12/2021 to 3/13/2021
	Prepa	red by: 70	1-EN	G.VG	EO	0379	93-0	1	Lo	ocati atun	ion n:	: G UT	iold M 1	en, 1 (N	BC IAD	- Ku 83)	Ims	she	en F	Park, east of parking lot Alignment:	Con Drill	pany: Mobile Augers and Research
		Tetra Tec	n Ca	inad	a In	С			No	orthi	ing	/Ea	stin	g: {	568	295	7.34	48,	, 50	2626.001 Station/Offset:	Drill	Make/Model: Fraste Multidrill XL
	Logge	d by: CR	Re X Po	eviev	ved Per	by: netro	TI Dmet	M ter 2	El S	evat hear	tior Str	n: Tena	789 th (k	9.58 (Pa)	m		:	ন		Coordinates Surveyed March 18, 2021	Drill	ing Method: Sonic
	DEPTH (m)	DRILLING DETAILS		10 10 W _P	SP	T "N	200 \" (B	LO W	3 WS/	/300	mm	40 1) ▲ WL 80	%		SAMPLE TYPE	SAMPLE NO		RECOVERY (%	SOIL SYMBOL	SOIL DESCRIPTION	CLASSIFICATION	COMMENTS TESTING
0220502.GPJ MOTI_DATATEMPLATE_REV3.GDT 5/3/22	-10 11 	37 6 33 25 17 17 24 22 38		13			40		6 6	50 50		80				SPT	1	<u>r</u> (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)		SAND, gravelly, fine to coarse sand, well graded, fine to coarse subangular to rounded gravel, some silt, trace subangular to subrounded cobbles up to 100 mm in diameter, grey-brown, non-cohesive, wet, very dense. (continued) -becomes sandy at 10.6 m -becomes compact at 10.6 m -becomes dense at 12.2 m -becomes dense at 12.2 m -becomes very dense at 13.7 m -becomes very dense at 13.7 m -coordinates and ground surface elevations (+/- 0.03 m) are based on points collected from a survey completed by Geometric Surveys. - Soil descriptions are based on visual classifications and field observations, in combination with laboratory testing. Some variation through the interpreted soil layers is expected. -'SPT5' numbered SPT1 to SPT7 were completed using a 300 lb LPT hammer. SPT8 and SPT9 were completed using a 140 lb SPT hammer.	SW-SM	Ш 779- Sieve (Sa#SPT7) 778- 5:18% S:70% F:12% 778- 777- 777- 776- 777- 776- 775- 774- 773-
5003793-01 - KHRB (PHASE 1-2)	- 					· · · · · · · · · · · · · · · · · · ·			••••	· · · · · · · · · · · · · · · · · · ·					-					backfilled with bentonite chips to surface.		772-
SOIL-REV3 ENG.VGE	- - - - - - - - - - - - - - - - - - -	<u>=</u> []] A -Au	ıger		 В -Ве	ecke	 	···		ore	•]G	-Gra	b]v	'-Va	ne			Final Depth of Hole: 15.2 m
S-II-O	Sample Type:	, L# -L	ab	\square	S-Sp	olit	[0 -C)dex	2	2	W	-Wa	sh		 T	-Sh	elby			Depth to Top of Rock:
ĭ		L≝ Sam	ple	L ال	Spo	on	Ŀ	•	(air	rotai	ry)	12	″ ⊿(n	nud r	etur	u) Ш	ШŢ	ube				Page 2 of 2

		 _			_													ç	SU	IMMARY LOG		Drill Hole #: HV2	1-07
		שינש	TF	RA	TI	EC	H		P	roje	ect	: 	lig	hw	ay	95	Kie	cki	ng	Horse River Bridges	Dat	e(s) Drilled: 09/13/2021	
	Prepar	ed by: 70	4-EN	G.VC	GEO	037	'93-0)1	Lo	atun	ion n:		old M 1	en, 1 (N	IAD BC	- 10 83)	th A	we	S,	Alignment:	Cor Dril	npany: Kootenay Pumping Syst ler:	.ems
	Ť	etra Tec	h Ca	anad	la Ir	nc			No	orthi	ing	/Ea	stin	g: {	568	2820	0.61	3,	50	2578.518 Station/Offset:	Dril	I Make/Model:	
	Logged	d by: KB	R	eviev locke	wed t Pe	l by	r: T	G	El X S	evat hear	tior Str	n: Tena	788 th (k	3.4 r (Pa)	n I		[2		Coordinates Surveyed December 3, 2021	Dril	ling Method: Hydrovac	Ê
	DEPTH (m)	DRILLING DETAILS		10 W _F 2	SF	рт "	200 N" (E 40	BLC	ews	/300 /300	mm	400 1)▲ WL 80	%		SAMPLE TYPE	SAMPLE NO			SOIL SYMBOL	SOIL DESCRIPTION	CLASSIFICATION	COMMENTS TESTING	ELEVATION (m
	- 0					÷	÷			÷	÷	-				G1				ASPHALT (90 mm thick) 0.1m	AS		-
II_DATATEMPLATE_REV3.GDT 9/3/22			•													G2	:			 Subangular to subrounded gravel up to 40 mm in diameter, some subrounded cobbles up to 250 mm in diameter, some silt, brown to grey, non-cohesive, dry, dense End of hydrovac excavation (target depth reached) at 3.0 m Coordinates and ground surface elevations (+/- 0.03 m) are based on points collected from a survey completed by Geometric Surveys. Soil descriptions are based on visual classifications and field observations, in combination with laboratory testing. Some variation through the interpreted soil layers is expected. Upon completion, the borehole was backfilled with sand and bentonite chips in accordance with BC Groundwater Protection regulations. The road section was reinstated with cold patch asphalt 	GP- GM	Sieve (Sa#G2) G:61% S:34% F:6%	787 786 786 785 784 784 783
JN NC	-					-					÷												-
9.70G	-7			····	•••• •	· • • •				 	•	···	••••	•••	1								-
20220	-													••••	ļ								781-
2-I-Z						-	i				÷	i											-
PHAU	-8						· · · ·			÷	•	···		••••	ł								
NHKB	-																						780-
- 10-56																							-
-003/5	9				 					 	•			•••	{								-
פר אפו	_																						779-
/3 EN																							
Ц-ЧЦ	- 10 Legend				B D		: 		c c	: 	:	: 	: 	Gro	 h		 		16		<u> </u>	Final Depth of Hole: 3	<u> </u>
	Sample Type:	⊷ للکلا ^م -۹۱ L# -L	.ab		в-в S -S	eck plit	יים רי		0-0	Jore Odex		L (7)	v w	-Gra -Wa	sh	Ы	יי⊾ זות	She	elby			Depth to Top of R	lock:
P	2011 C	Sam	ple	M	Spo	on	ŀ	<u></u>	(air	rotar	ry)	12	Ø(n	nud r	retur	n) Ш	Ш⊤ι	lbe	,			Page 1	of 1

APPENDIX C

ENERGY MEASUREMENT REPORTS



Certificate of Test



MARL Technologies Inc. 5603 - 54 St. NW Edmonton, AB T6B 3G8

Client:

Mobile Augers 5603 - 54 st. Edmonton, Alberta

Test Details:

Description:	SPT Hammer Test Certificate
ASTM Testing Standard:	D4633-05
Test Date:	2021-03-15
Test Locaton:	MARL Technologies Inc.
Drill Rig #:	Unit 189
Hammer Type:	300lb Auto-Hammer
Hammer Manufacturer:	MARL Technologies Inc.
Hammer Serial #:	
Rod Type:	AW
Test Depth:	0.00 - 15.00 ft

Average ETR: 73.6 %

This certificate shows the **average** energy transfer for the hammer above on the above date at the above location.

Technician:				
	Name:	Scott Hughes	Date:	2021-03-16
	Signature:			

MARL Technologies Inc. does not guarantee that this represents the average value at different locations, with different operators, under different weather conditions and different rig configurations.

Certificate of Test



MARL Technologies Inc. 5603 - 54 St. NW Edmonton, AB T6B 3G8

Client:

Mobile Augers 5603 - 54 st. Edmonton, Alberta

Test Details:

Description:	SPT Hammer Test Certificate
ASTM Testing Standard:	D4633-05
Test Date:	2021-03-15
Test Locaton:	MARL Technologies Inc.
Drill Rig #:	Unit 189
Hammer Type:	140lb Auto-Hammer
Hammer Manufacturer:	MARL Technologies Inc.
Hammer Serial #:	
Rod Type:	AW
Test Depth:	0.00 - 15.00 ft

Average ETR: 75.4 %

This certificate shows the **average** energy transfer for the hammer above on the above date at the above location.

Technician:				
	Name:	Scott Hughes	Date:	2021-03-16
	Signature:			

MARL Technologies Inc. does not guarantee that this represents the average value at different locations, with different operators, under different weather conditions and different rig configurations.

APPENDIX D

iBPT RESULTS



PRESENTATION OF SITE INVESTIGATION RESULTS

Kicking Horse River Bridge

Prepared for:

Tetra Tech Canada Inc.

ConeTec Job No: 21-02-21991

Project Start Date: 15-Mar-2021 Project End Date: 20-Mar-2021 Report Date: 30-Mar-2021



Prepared by:

ConeTec Investigations Ltd. 201-8327 Eastlake Drive Burnaby, BC V5A4W2

Tel: (604) 273-4311 Fax: (604) 273-4066 Toll Free: (800) 567-7969

ConeTecYVR@conetec.com www.conetec.com www.conetecdataservices.com



Introduction

The enclosed report presents the results of the site investigation program conducted by ConeTec Investigations Ltd. for Tetra Tech Canada Inc. at Kicking Horse River bridge in Golden, BC. The program consisted of one instrumented Becker Penetration Test (iBPT). Please note that this report, which also includes all accompanying data, are subject to the 3rd Party Disclaimer and Client Disclaimer that follow in the 'Limitations' section of this report.

Project Information

Project	
Client	Tetra Tech Canada Inc.
Project	Kicking Horse River Bridge
ConeTec project number	21-02-21991

An aerial overview from Google Earth including the iBPT test location is presented below.



Rig Description	Deployment System	Test Type
Becker drill rig (AP 1000)	ICE 180 diesel pile driving hammer	iBPT



Coordinates		
Test Type	Collection Method	EPSG Number
iBPT	Consumer grade GPS	32611

Instrumented Becker Pene	tration Test (iBPT)
Depth reference	Depths are referenced to the existing ground surface at the time of each test.
Additional information	Tabular results are provided in Excel format files in the data release package. Plots with iBPT results and Harder & Seed (1986) results are provided in the appendices.



Closure

Thank you for the opportunity to work on this project. The equipment used and the field procedures followed complied with current accepted practice standards. This report has been prepared under my supervision and I have reviewed and approved the content.

ConeTec Investigations Ltd.



Ilmar Weemees, P.Eng.



Limitations

3rd Party Disclaimer

This report titled "Kicking Horse River Bridge", referred to as the ("Report"), was prepared by ConeTec for Tetra Tech Canada Inc.. The Report is confidential and may not be distributed to or relied upon by any third parties without the express written consent of ConeTec. Any third parties gaining access to the Report do not acquire any rights as a result of such access. Any use which a third party makes of the Report, or any reliance on or decisions made based on it, are the responsibility of such third parties. ConeTec accepts no responsibility for loss, damage and/or expense, if any, suffered by any third parties as a result of decisions made, or actions taken or not taken, which are in any way based on, or related to, the Report or any portion(s) thereof.

Client Disclaimer

ConeTec was retained by Tetra Tech Canada Inc. to collect and provide the raw data ("Data") and to provide analysis of the raw data to extract Becker Test tip and head residual energy, which is included in this report titled "Kicking Horse River Bridge", which is referred to as the ("Report"). ConeTec has collected and reported the Data in accordance with current industry standards. No other warranty, express or implied, with respect to the Data is made by ConeTec. In order to properly understand the Data included in the Report, reference must be made to the documents accompanying and other sources referenced in the Report in their entirety. Any analysis, interpretation, judgment, calculations and/or geotechnical parameters (collectively "Interpretations") included in the Report, including those based on the Data, are outside the scope of ConeTec's retainer and are included in the Report as a courtesy only. Other than the Data, the contents of the Report (including any Interpretations) should not be relied upon in any fashion without independent verification and ConeTec is in no way responsible for any loss, damage or expense resulting from the use of, and/or reliance on, such material by any party.



The instrumented Becker Penetration Test (iBPT) system was developed to obtain direct measurements of force and acceleration at the drill tip required to compute the energy delivered to the soil beneath the drill tip during penetration (DeJong et al. (2017)). Coupled with the measured blow counts, this provides an energy normalized blow count based on direct measurements. This direct measurement based method eliminates the long-standing challenge of estimating, and then correcting for, the shaft friction that develops along the drill string length which absorbs hammer energy.

The iBPT is an instrumentation and data acquisition system that provides automated measurements of force and acceleration at the tip and head of a drill string, in addition to measurements of bounce chamber pressure and drill string position. The system is comprised of two 12" long instrumented pipe sections located at the head and tip of the drill string that contain redundant acceleration and strain measurements. The redundant instrumentation enables continued penetration even if one sensor malfunctions.

Data from the instrumented tip section is acquired by a data acquisition module located in the tip module while data from the head section is acquired in the iBPT trailer. The instrumented pipe sections are mechanically integrated within the conventional 6 5/8" outer diameter Becker drill system. Dynamic measurements are obtained during hammer impacts as the drill string is advanced into the ground. Continuous measurements of air pressure in the hammer bounce chamber and the string potentiometer position are also obtained during operation.

The dynamic measurements of strain and acceleration at the head and tip are processed to compute the energy delivered to each respective location. The acceleration records are integrated to obtain velocity, and then integrated a second time to obtain displacement. The force measurements account for both the dynamic force imparted during dynamic hammer blows as well as the residual locked-in force that develops at the tip due to shaft friction. Temperature correction is also applied to the tip force measurements.

Following conventional pile dynamic analysis (e.g. Rausche et al. (1972)) and the ASTM standard for energy measurement of dynamic penetrometers (D4633-16, (2016)), energy is calculated following:

$$E(kJ) = \int FVdt$$

The energy computed and reported is the residual energy measured at the tip, as this measurement is representative of the total energy absorbed by the soil. This is different than the maximum energy delivered at the head as is conventionally used with SPT and Sy and Campanella's (S&C) Becker analysis. The iBPT energy efficiency is computed as a percentage of the theoretical hammer energy as:

$$E(\%) = \frac{\int FVdt}{11.0 \text{ kJ}}$$

The energy normalization of penetration resistance is consistent in form with SPT (ASTM D4633-16) and BPT (Sy and Campanella (1994)) methods.



The BPT measured blow counts, N_B, are then normalized to a reference hammer energy of 30% using:

$$N_{B30} = N_B \frac{E(\%)}{30}$$

The normalized to 30% hammer energy is a typical efficiency for double acting diesel hammers and is consistent with Sy and Campanella (1994).

The delivered energy ratio (DER), the ratio of the residual energy delivered to the tip of the drill string, normalized to the residual energy delivered to the head of the drill string, provides an indication of the amount of energy absorbed along the drill string due to the shaft friction. In general, the DER decreases with penetration depth and when underlying soft layers are encountered (DeJong et al. (2017)).

It is noted that DER values larger than 100% are possible as the residual energy at the tip is computed based on both the dynamic and locked-in force components. Small DER values (e.g. less than about 10%) indicate that practically all of the hammer energy (and thus penetration resistance) is being absorbed by shaft resistance along the drill string; this occurs when shaft friction is very high relative to tip resistance. This can occur in competent soils when shaft friction is high and/or in very weak soils when the soil resistance below the tip is very small. When the DER is very small (e.g. < 10%), the absolute tip measurements of force and acceleration used to compute energy can be near the performance limit, resulting in a very small ratio of E(%)/30.

When minimal energy is delivered to the tip a significant normalization factor is required to compute N_{B30} . For data reduction, a threshold limit of 2% is set for the tip residual energy, which corresponds to a normalization factor of 15. Due to the proportional form of the equation, a tip residual energy of 3% significantly reduces the normalization factor to 10 while a tip residual energy of 1% significantly increases the normalization factor to 30. Currently, application of the equation down to tip residual energies of 2% is reasonable though confidence increases rapidly with greater tip residual energy. Application of the equation to measurements with tip residual energies less than 2% extends the developed correlation beyond the database upon which it was developed.

The correlation developed by Ghafghazi et al. (2017a) is used to estimate equivalent SPT N_{60} values as follows:

N_{60} =1.8 × N_{B30}

The median correlation factor of 1.8 is the best fit to the data collected and analyzed by Ghafghazi et al. (2017a), producing an estimate close to the expected 50th percentile value. This correlation value was determined based on comparisons of 364 pairs of side-by-side SPT N₆₀ and iBPT N_{B30} measurements in soils that are free of gravel (Ghafghazi et al. (2017a)). More conservative estimates of SPT N₆₀ values corresponding to a 33^{rd} percentile value could be obtained using a correlation factor of about 1.6 or a 16th percentile value using a correlation factor of about 1.3. As detailed in Ghafghazi et al. (2017a), most of the scatter in the developed correlation is attributed to lateral spatial variability commonly present in fluvial depositional environments. As a result, and as evident in the published correlation plots, it is reasonable to observe penetration resistance differences between two adjacent soundings/borings performed at a spacing of 10 to 20 feet, regardless of whether it is two iBPT N_{B30} soundings, two SPT N₆₀ borings, or two cone penetration test (CPTu) corrected tip resistance (q_t) soundings.



Additional analysis was performed to compute equivalent SPT N_{60} values based on the Harder and Seed (1986) method. This method requires measured blows per foot (N_B), average hammer bounce chamber pressure (BCP), and correction of the measured air pressure for project elevation. The effect of shaft friction is not explicitly accounted for; instead, the accumulation of shaft friction with depth is embedded in the correction of N_B to N_{BC} even though the accumulation of shaft friction with depth depends on stratigraphy and soil type. Further discussion of the Harder and Seed method and the differences compared to the iBPT method are discussed in Ghafghazi et al. (2017b).

A summary of the iBPT soundings along with test results and individual plots are provided in the relevant appendices.

References

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Harder L.F.Jr., and Seed H.B., 1986, "Determination of penetration resistance for coarse-grained soils using the Becker Hammer Drill", College of Engineering, University of California, Berkeley, Report No. UCB/EERC-86/06, May 1986.

Rausche F., Moses F., and Goble G., 1972, "Soil resistance predictions from pile dynamics", ASCE, Journal of the Soil Mechanics and Foundation Division, 98(SM9): 917-937. DOI: 10.1061/40743(142)24.

Sy A. and Campanella R.G., 1994, "Becker and standard penetration tests (BPTSPT) correlations with consideration of casing friction", Canadian Geotechnical Journal, 31(3): 343-356. DOI: 10.1139/T94-042.



The appendices listed below are included in the report:

- Instrumented Becker Penetration Test Borehole Summary and Tabular Results
- Instrumented Becker Penetration Test Plots with Energy Results and iBPT Equivalent N₆₀
- Instrumented Becker Penetration Test Plots with Harder & Seed (1986) Results



Instrumented Becker Penetration Test Borehole Summary and Tabular Results



	Job No:	21-02-21991
CONFTEC	Client:	Tetra Tech Canada Inc.
CONTRACT	Project:	Kicking Horse River Bridge
	Start Date:	15-Mar-2021
	End Date:	20-Mar-2021

		iBl	PT BOREHOI	LE SUMI	MARY				
Borehole ID	File Name	Date From	Date To	Predrill Depth (ft)	Start Depth (ft)	Final Depth (ft)	Northing ¹ (m)	Easting ¹ (m)	Refer To Notation
iBPT21-02	21-02-21991_BH02	15-Mar-2021	20-Mar-2021	0.0	0.0	156.0	5682958	502572	

1. Coordinates were acquired using consumer grade GPS equipment, datum: WGS 1984 / UTM Zone 11 North.



 Job No:
 21-02-21991

 Client:
 Tetra Tech Canada Inc

 Project:
 Kicking Horse River Bridge

 Start Date:
 15-Mar-2021

 End Date:
 20-Mar-2021

Borehole ID: iBPT21-02 Rig: AP 1000

				INSTR	UMEN	TED BEO	CKER PENETR	RATION TEST	RESULTS						
Start Depth (ft)	End Depth (ft)	Start Depth (m)	End Depth (m)	Average Bounce Chamber Pressure (BCP) (psi)	N_{BC}^{1}	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
0.0	1.1	0.0	0.3	9.0	5	5	7	10.2	10.1	5.4	5.3	52.8	1	2	
1.1	2.0	0.3	0.6	11.0	7	8	15	38.4	37.1	26.2	25.9	69.7	13	23	
2.0	3.0	0.6	0.9	15.6	20	19	37	34.6	30.7	25.7	25.5	83.0	31	57	
3.0	4.0	0.9	1.2	19.5	42	35	57	29.0	26.7	25.1	24.0	89.9	46	82	
4.0	5.0	1.2	1.5	19.8	43	36	58	25.2	21.0	24.4	23.3	110.9	45	81	
5.0	6.0	1.5	1.8	19.3	38	33	51	28.5	22.4	23.9	23.0	102.8	39	70	
6.0	7.3	1.8	2.2	18.6	31	28	43	24.3	18.5	23.3	22.4	121.1	32	58	
7.3	8.0	2.2	2.4	17.7	33	29	53	21.0	14.3	21.4	20.6	143.7	36	65	
8.0	9.0	2.4	2.7	18.5	48	39	81	19.5	13.1	19.3	17.9	136.6	48	87	
9.0	10.0	2.7	3.1	22.6	94	67	135	25.9	16.6	20.9	17.8	107.2	80	144	
10.0	11.0	3.1	3.4	24.0	145	97	212	26.9	17.0	20.0	17.0	100.0	120	216	
11.0	12.0	3.4	3.6	23.3	171	113	276	28.6	18.2	18.4	15.3	84.3	141	254	
27.2	28.0	8.3	8.5	14.9	19	18	39	29.6	27.4	24.0	23.1	84.4	30	54	
28.0	29.0	8.5	8.8	15.1	20	19	40	29.8	26.6	24.8	23.7	89.0	32	57	
29.0	30.0	8.8	9.1	15.6	21	20	40	30.0	26.3	24.7	23.8	90.6	32	57	
30.0	31.0	9.1	9.5	16.5	25	23	44	31.6	27.4	24.6	23.7	86.5	35	62	
31.0	32.0	9.5	9.8	16.0	24	22	44	33.1	29.8	23.4	22.4	75.1	33	59	
32.0	33.0	9.8	10.1	16.1	24	23	45	29.5	25.5	23.2	21.9	86.0	33	59	
33.0	34.0	10.1	10.4	15.1	20	19	41	27.5	22.9	22.6	21.3	93.4	29	52	
34.0	35.0	10.4	10.7	14.4	17	17	37	28.7	27.2	24.0	23.9	87.5	29	53	
35.0	36.0	10.7	11.0	16.3	24	22	43	29.6	26.1	23.6	23.0	88.0	33	59	
36.0	37.0	11.0	11.3	16.5	25	23	45	31.7	28.7	22.9	21.8	75.7	33	59	
37.0	38.0	11.3	11.6	17.1	18	18	25	33.0	30.6	22.5	21.3	69.7	18	32	
38.0	39.0	11.6	11.9	16.5	26	24	47	30.1	26.1	20.5	19.9	76.4	31	56	
39.0	40.0	11.9	12.2	17.0	39	33	70	31.1	24.4	19.5	19.1	78.0	44	80	
40.0	41.0	12.2	12.5	16.8	37	32	68	31.7	25.2	20.2	19.8	78.7	45	81	
41.0	42.0	12.5	12.8	17.8	47	38	85	29.7	20.1	18.9	18.1	90.0	51	92	
42.0	43.0	12.8	13.1	19.2	51	41	81	35.8	24.8	20.5	19.5	78.7	53	95	
43.0	44.0	13.1	13.4	17.8	45	37	80	31.8	23.1	19.4	18.4	79.5	49	88	
44.0	45.0	13.4	13.7	17.8	41	35	67	33.8	27.8	20.4	19.4	70.0	43	78	
45.0	46.0	13.7	14.0	18.4	41	35	63	34.7	28.1	21.1	20.1	71.6	42	76	
46.0	47.0	14.0	14.3	17.8	37	32	59	34.0	28.3	21.7	20.6	72.7	40	73	
47.0	48.0	14.3	14.6	16.3	22	20	37	31.9	26.8	21.7	20.7	77.4	26	46	
48.0	49.0	14.6	14.9	15.7	21	19	38	35.6	32.4	23.3	23.2	71.5	29	53	
49.0	50.0	14.9	15.2	15.6	22	20	42	33.7	30.3	22.1	21.9	72.1	31	55	



 Job No:
 21-02-21991

 Client:
 Tetra Tech Canada Inc

 Project:
 Kicking Horse River Bridge

 Start Date:
 15-Mar-2021

 End Date:
 20-Mar-2021

Borehole ID: iBPT21-02 Rig: AP 1000

				INSTR	UMENT	TED BE	CKER PENETR	ATION TEST	RESULTS						
Start Depth (ft)	End Depth (ft)	Start Depth (m)	End Depth (m)	Average Bounce Chamber Pressure (BCP) (psi)	N_{BC}^{1}	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
50.0	51.0	15.2	15.5	14.9	19	18	40	32.2	29.2	20.9	20.7	70.8	28	50	
51.0	52.0	15.5	15.9	14.5	19	18	41	31.1	27.2	19.5	18.7	68.9	26	46	
52.0	53.0	15.9	16.2	14.2	17	16	37	30.2	27.9	18.5	18.1	65.1	22	40	
53.0	54.0	16.2	16.5	15.2	20	19	41	32.9	29.7	18.8	18.4	62.0	25	45	
54.0	55.0	16.5	16.8	16.8	29	26	51	35.6	30.8	19.2	18.5	60.2	31	57	
55.0	56.0	16.8	17.1	17.1	29	26	49	36.0	30.7	20.2	19.4	63.1	32	57	
56.0	57.0	17.1	17.4	15.5	25	23	52	32.8	27.2	19.1	18.4	67.6	32	57	
57.0	58.0	17.4	17.7	16.3	22	20	37	34.7	28.8	19.6	19.1	66.2	24	42	
58.0	59.0	17.7	18.0	17.9	31	28	48	40.6	33.9	22.0	21.8	64.1	35	63	
59.0	60.0	18.0	18.3	17.2	31	27	52	36.3	29.7	19.5	19.2	64.7	33	60	
60.0	61.0	18.3	18.6	16.6	27	25	48	35.2	29.9	18.8	18.4	61.7	29	53	
61.0	62.0	18.6	18.9	16.1	21	20	36	36.9	33.1	22.0	21.2	63.9	25	46	
62.0	63.0	18.9	19.2	15.1	18	17	33	34.8	32.1	21.8	21.3	66.5	23	42	
63.0	64.0	19.2	19.5	14.7	18	17	36	32.2	29.7	19.7	19.4	65.2	23	42	
64.0	65.0	19.5	19.8	14.3	18	17	41	31.1	28.7	18.0	17.6	61.4	24	43	
65.0	66.0	19.8	20.1	14.2	20	19	47	30.8	27.9	17.4	17.0	61.1	27	48	
66.0	67.0	20.1	20.4	14.1	18	17	41	31.1	28.6	17.8	17.5	61.1	24	43	
67.0	68.0	20.4	20.7	14.8	17	17	33	30.4	26.1	16.9	16.7	64.3	18	33	
68.0	69.0	20.7	21.0	16.5	41	35	82	32.8	23.6	15.0	14.8	62.7	40	73	
69.0	70.0	21.0	21.3	17.7	63	48	142	33.5	20.0	13.3	12.4	62.1	59	106	
70.0	71.0	21.3	21.6	17.9	75	55	182	34.7	19.5	13.9	12.6	64.7	77	138	
71.0	72.0	21.6	21.9	18.6	93	66	221	35.2	18.3	13.7	12.4	68.0	91	165	
72.0	73.0	21.9	22.3	18.9	91	65	203	38.8	22.4	14.6	14.0	62.5	94	170	
73.0	74.0	22.3	22.6	18.3	74	55	165	36.0	17.1	14.0	12.5	73.2	69	124	
74.0	75.0	22.6	22.9	18.2	56	44	107	36.3	22.4	16.2	15.5	69.2	55	99	
75.0	76.0	22.9	23.2	18.5	51	41	89	37.0	24.8	16.6	15.8	63.7	47	84	
76.0	77.0	23.2	23.5	17.7	59	46	129	35.3	22.8	15.8	14.5	63.4	62	112	
77.0	78.0	23.5	23.8	18.2	69	51	148	36.2	24.8	14.0	12.8	51.6	63	114	
78.0	79.0	23.8	24.1	19.4	90	65	186	38.6	26.4	15.8	13.3	50.4	83	149	
79.0	80.0	24.1	24.4	19.2	83	60	169	38.5	26.3	15.6	14.6	55.6	82	148	
80.0	81.0	24.4	24.7	17.5	57	45	124	35.9	26.0	14.6	13.3	51.2	55	99	
81.0	82.0	24.7	25.0	17.7	56	44	118	37.0	26.9	14.9	14.2	52.8	56	100	
82.0	83.0	25.0	25.3	18.1	57	45	114	36.6	27.4	14.7	13.5	49.4	51	93	
83.0	84.0	25.3	25.6	19.0	78	57	159	39.0	27.1	17.0	16.0	59.1	85	153	
84.0	85.0	25.6	25.9	17.3	73	54	198	35.9	25.0	16.8	14.9	59.7	98	177	



 Job No:
 21-02-21991

 Client:
 Tetra Tech Canada Inc

 Project:
 Kicking Horse River Bridge

 Start Date:
 15-Mar-2021

 End Date:
 20-Mar-2021

Borehole ID: iBPT21-02 Rig: AP 1000

				INSTR	UMENT	TED BEO	CKER PENET	RATION TEST	RESULTS						
Start Depth (ft)	End Depth (ft)	Start Depth (m)	End Depth (m)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
85.0	86.0	25.9	26.2	17.8	76	56	191	34.8	27.1	13.3	12.0	44.4	77	138	
86.0	87.0	26.2	26.5	19.0	71	53	140	37.5	26.5	15.4	14.0	52.9	65	118	
87.0	88.0	26.5	26.8	17.2	43	36	80	37.9	30.8	15.8	15.4	50.1	41	74	
88.0	89.0	26.8	27.1	18.8	70	53	140	38.8	32.0	17.3	17.2	53.7	80	145	
89.0	90.0	27.1	27.4	19.0	102	72	239	38.1	28.5	15.5	15.4	53.8	122	220	
90.0	90.8	27.4	27.7	19.4	130	88	313	37.6	26.1	14.0	13.9	53.3	145	261	
112.0	113.0	34.1	34.4	13.8	39	33	160	25.1	17.9	12.7	12.0	67.3	64	115	
113.0	114.0	34.4	34.7	12.7	27	25	120	23.6	19.5	12.9	12.4	63.4	49	89	
114.0	115.0	34.7	35.1	13.0	23	22	86	25.7	22.5	14.3	13.7	61.2	39	71	
115.0	116.0	35.1	35.4	14.2	24	22	63	30.9	29.5	17.8	17.7	59.9	37	67	
116.0	117.0	35.4	35.7	15.2	34	30	81	31.9	26.1	17.0	16.5	63.3	45	80	
117.0	118.1	35.7	36.0	16.4	34	30	66	33.9	26.8	17.1	16.4	61.0	36	65	
118.1	119.0	36.0	36.3	16.9	37	32	67	37.1	33.1	16.0	15.5	46.8	35	62	
119.0	120.0	36.3	36.6	17.4	55	43	119	39.8	32.9	14.2	13.8	41.8	55	98	
120.0	121.0	36.6	36.9	18.1	96	68	253	39.6	25.8	13.8	13.2	51.0	111	200	
121.0	122.0	36.9	37.2	16.5	59	46	170	33.9	25.5	13.6	13.3	52.1	75	136	
122.0	123.0	37.2	37.5	17.2	69	52	187	40.6	31.1	14.9	14.3	45.8	89	160	
123.0	124.0	37.5	37.8	17.2	81	59	243	45.0	37.2	13.6	11.4	30.7	92	166	
124.0	125.0	37.8	38.1	16.5	74	55	255	43.4	30.4	14.3	12.2	40.1	103	186	
125.0	126.0	38.1	38.4	17.2	66	50	172	39.1	34.1	16.7	15.5	45.4	89	160	
126.0	127.0	38.4	38.7	17.1	33	29	58	47.6	42.8	22.0	21.2	49.6	41	74	
127.0	128.0	38.7	39.0	16.9	30	27	53	53.1	49.7	21.8	20.6	41.4	36	65	
128.0	129.0	39.0	39.3	16.5	30	27	56	43.2	43.1	20.6	19.3	44.9	36	65	
129.0	130.0	39.3	39.6	16.8	40	34	74	36.2	33.9	17.3	16.1	47.5	40	71	
130.0	131.0	39.6	39.9	17.2	61	47	152	34.3	27.9	11.9	10.7	38.5	54	98	
131.0	132.0	39.9	40.2	16.2	53	42	145	29.1	24.9	11.1	10.3	41.1	50	89	
132.0	133.0	40.2	40.5	15.5	50	40	157	30.6	25.0	10.7	10.1	40.3	53	95	
133.0	134.0	40.5	40.8	15.8	41	34	91	36.9	33.3	11.6	11.0	32.9	33	60	
134.0	135.0	40.8	41.1	16.2	45	37	102	36.3	32.9	13.6	13.4	40.7	45	82	
135.0	136.0	41.1	41.5	17.4	55	44	119	38.1	33.3	12.5	12.1	36.4	48	86	
136.0	137.0	41.5	41.8	17.8	64	49	144	38.5	34.5	10.3	10.0	29.0	48	86	
137.0	138.0	41.8	42.1	17.9	80	58	202	38.3	33.1	9.7	9.1	27.4	61	110	
138.0	139.0	42.1	42.4	18.0	69	52	156	39.6	34.5	9.9	9.4	27.2	49	88	
139.0	140.0	42.4	42.7	17.9	83	60	211	39.5	33.8	8.7	8.0	23.7	56	101	
140.0	141.0	42.7	43.0	18.4	100	70	259	33.7	28.1	8.2	6.8	24.2	59	106	



Job No:21-02-21991Client:Tetra Tech Canada IncProject:Kicking Horse River BridgeStart Date:15-Mar-2021End Date:20-Mar-2021

Borehole ID: iBPT21-02 Rig: AP 1000

				INSTR	UMEN	TED BE	CKER PENETH	RATION TEST	T RESULTS						
Start Depth (ft)	End Depth (ft)	Start Depth (m)	End Depth (m)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
151.1	152.0	46.1	46.3	13.8	44	37	220	23.9	19.5	12.8	9.4	48.5	69	125	
152.0	153.0	46.3	46.6	15.1	57	45	255	33.6	26.2	13.7	12.0	46.0	102	184	
153.0	154.0	46.6	46.9	15.4	57	45	227	30.8	25.1	13.5	12.1	48.1	91	164	
154.0	155.0	46.9	47.2	16.3	60	46	181	34.5	26.0	10.8	10.1	38.9	61	110	
155.0	156.0	47.2	47.5	16.9	81	59	256	35.3	26.3	13.7	11.6	44.0	99	178	

1. Harder & Seed, 1986.

Instrumented Becker Penetration Test Plots with Energy Results and iBPT Equivalent $N_{\rm 60}$







Instrumented Becker Penetration Test Plots with Harder & Seed (1986) Results







APPENDIX E

LABORATORY TEST RESULTS

Particle Size Distribution Test Reports

SIEVE ANALYSIS REPORT

						ŀ	ASTMC	136, C11	1							
Project:	Kick	ing Ho	rse Bi	idges	;				Sam	ple No.:	:	SPT 2				
Client:	Mini	stry of	Trans	porta	tion and	d Infra	astructu	ure	Date	e Sample	ed:	March,	202	1		
Project No.:	704	-ENG.\	/GEO	03793	3-01.10)7			Sam	pled By	:	TTC In	C.			
Attention:									Date	e Tested	:	April 2	7, 202	21		
Description:	GRA	AVEL a	ind SA	ND, s	some s	ilt			Test	ed By:	MS	La	b: (Calga	ry	
Source:	BH2	21-03							No.	Crushed	l Face	es:				
Location:	Gold	den, BO	2						Mois	sture Co	ntent:	6.	1%			
Specification	n:															
Sieve Size F	Percent Passing		¹⁰⁰ [1	1		T	[TT		<u> </u>	$\top T$		
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150 mm		-														
100 mm		Р	80										+			
80 mm		e r	70								∕⊥⊢		_			
50 mm		с														
40 mm	100	e n	60										+			
25 mm	92	t	50										_			
20 mm	92	Р														
16 mm	82	а	40										+			
12.5 mm	73	S S	30										\perp			
10 mm	67	i														
5 mm	52	n a	20										-			
2.5 mm	45	Ŭ	10										_			
1.25 mm	36															
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315 µm	25	-	×		፵ ; - (um)	31	³³	1.2	5	(n	$\frac{2}{2}$	6 4	- ič	⁸ 0	20 20	
160 µm	10 5			•	V . 7				Sieve	Size	1111)					
80 µm	18.5															
Remarks:																
							F	Reviewe	ed By:_	5	-	M	1	_	_P.Er	ng.

Data presented hereon is for the sole use of the stipulated client. Tetra Tech is not responsible, nor can be held liable, for use made of this report by any other party, with or without the knowledge of Tetra Tech. The testing services reported herein have been performed to recognized industry standards, unless noted. No other warranty is made. These data do not include or represent any interpretation or opinion of specification compliance or material suitability. Should engineering interpretation be required, Tetra Tech will provide it upon written request.



SIEVE ANALYSIS REPORT

Ki	cking Ho	orse E	Bridges	5				Sam	ple No.	.:	SP	Γ7			
Mi	nistry of	Tran	sporta	tion an	d Infra	structu	ire	Date	e Samp	led:	Mai	rch, 2	2021		
.: 70	4-ENG.	VGE	00379	3-01.10)7			Sam	pled By	y:	TTC	C Inc.			
								Date	e Testeo	d:	Apr	il 27,	2021		
n: SA	AND and	GR/	VEL,	some s	silt			Test	ted By:	MS		Lab:	: Ca	algary	
BH	121-03							No.	Crushe	d Face	es:	_			
Go	olden, B	С						Mois	sture Co	ontent:	:	10.6	6%		
on:															
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		90									4				
										И					
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											_				
	Kiu Mi Mi 100 Percer Passin 100 96 93 87 83 78 63 54 42 32 20 13 10.3	Kicking Ha Ministry of 704-ENG. BH21-03 Golden, B on: Percent Passing 100 96 100 96 100 96 100 96 100 96 100 93 87 83 78 63 9 42 32 20 13 10.3	Kicking Horse BMinistry of Trans704-ENG.VGEOSAND and GRABH21-03Golden, BCCon:100Percent Passing1009090919092909398780939878093987883307810093987883307810093993987883307810093910099399398788798798399399399399399399399399399399399399399399399499599699399499599599699399499599599699799899	Kicking Horse BridgesMinistry of Transporta704-ENG.VGE003793n:SAND and GRAVEL, iBH21-03Golden, BCon:PercentPassing90919293949593969697989893949593939495939394959393949593959393949595939495959395939495	Kicking Horse BridgesMinistry of Transportation an 704-ENG.VGEO03793-01.10n:SAND and GRAVEL, some s BH21-03 Golden, BCon: 100 90 	Kicking Horse Bridges Ministry of Transportation and Infra 704-ENG.VGE003793-01.107 n: SAND and GRAVEL, some silt BH21-03 Golden, BC con: 90 Percent 90 Passing 100 90 90 Percent 70 Con: 90 Passing 100 90 90 91 90 92 9 93 87 83 78 63 9 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10 9 10	Kicking Horse Bridges Ministry of Transportation and Infrastructure 100 SAND and GRAVEL, some silt BH21-03 Golden, BC on: Percent Passing 100 90 91 90 91 92 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 91 92 93 93 94 95 95 96 97 98 99 90 90 91 92 93 93 <td>Kicking Horse Bridges Ministry of Transportation and Infrastructure 704-ENG.VGEO03793-01.107 n: SAND and GRAVEL, some silt BH21-03 Golden, BC on: Percent Passing 100 96 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 91 10.3</td> <td>Kicking Horse Bridges Sam Ministry of Transportation and Infrastructure Date 704-ENG.VGE003793-01.107 Date Image: SAND and GRAVEL, some silt Tesl BH21-03 No. Golden, BC Mois Image: Sand No. Image: Sand No. Image: Sand No. Golden, BC Mois Image: Sand No. Image: Sand Image: Sand Image: Sand Image: Sand</td> <td>Kicking Horse Bridges Sample No Ministry of Transportation and Infrastructure Totale Sampled Bill Totale Tester Sampled Bill BH21-03 Colden, BC Cont Totale Sampled Bill Parsing 100 Percent 9 No Percent Passing 100 Percent 9 No Percent Passing 100 Percent 9 No Percent Passing 100 Percent 9 No 9<td>Kicking Horse Bridges Sample No.: Ministry of Transportation and Infrastructure Date Sampled: 704-ENG.VGEO03793-01.107 Date Tested: T SAND and GRAVEL, some silt Date Tested: BH21-03 Colden, BC Date Tested: Golden, BC Date Tested: Tested By: Ms Passing 100 <t< td=""><td>Kicking Horse Bridges Sample No.: SP Ministry of Transportation and Infrastructure Date Sampled By: Mailed By: Total asymptotic Sample No.: Sample No.: SP Date Tested By: MS asymptotic Solden, BC No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: m: Percent 100 9 9 No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 0</td><td>Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2 704-ENG.VGE003793-01.107 Date Sampled By: TC Inc. i: SAND and GRAVEL, some silt Date Sampled By: TC Inc. BH21-03 Golden, BC Date Sampled By: Inc. on: </td><td>Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Tested By: MS Lab: Ca No. Crushed Faces: Moisture Content: 10.6% On: 9 9 9 Passing 9 9 9 9 Passing 9 9 9 9 9 Passing 9 <t< td=""><td>Kicking Horse Bridges Sample No:: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 No: Output Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:</td></t<></td></t<></td></td>	Kicking Horse Bridges Ministry of Transportation and Infrastructure 704-ENG.VGEO03793-01.107 n: SAND and GRAVEL, some silt BH21-03 Golden, BC on: Percent Passing 100 96 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 93 87 83 78 63 91 10.3	Kicking Horse Bridges Sam Ministry of Transportation and Infrastructure Date 704-ENG.VGE003793-01.107 Date Image: SAND and GRAVEL, some silt Tesl BH21-03 No. Golden, BC Mois Image: Sand No. Image: Sand No. Image: Sand No. Golden, BC Mois Image: Sand No. Image: Sand Image: Sand Image: Sand Image: Sand	Kicking Horse Bridges Sample No Ministry of Transportation and Infrastructure Totale Sampled Bill Totale Tester Sampled Bill BH21-03 Colden, BC Cont Totale Sampled Bill Parsing 100 Percent 9 No Percent Passing 100 Percent 9 No Percent Passing 100 Percent 9 No Percent Passing 100 Percent 9 No 9 <td>Kicking Horse Bridges Sample No.: Ministry of Transportation and Infrastructure Date Sampled: 704-ENG.VGEO03793-01.107 Date Tested: T SAND and GRAVEL, some silt Date Tested: BH21-03 Colden, BC Date Tested: Golden, BC Date Tested: Tested By: Ms Passing 100 <t< td=""><td>Kicking Horse Bridges Sample No.: SP Ministry of Transportation and Infrastructure Date Sampled By: Mailed By: Total asymptotic Sample No.: Sample No.: SP Date Tested By: MS asymptotic Solden, BC No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: m: Percent 100 9 9 No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 0</td><td>Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2 704-ENG.VGE003793-01.107 Date Sampled By: TC Inc. i: SAND and GRAVEL, some silt Date Sampled By: TC Inc. BH21-03 Golden, BC Date Sampled By: Inc. on: </td><td>Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Tested By: MS Lab: Ca No. Crushed Faces: Moisture Content: 10.6% On: 9 9 9 Passing 9 9 9 9 Passing 9 9 9 9 9 Passing 9 <t< td=""><td>Kicking Horse Bridges Sample No:: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 No: Output Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:</td></t<></td></t<></td>	Kicking Horse Bridges Sample No.: Ministry of Transportation and Infrastructure Date Sampled: 704-ENG.VGEO03793-01.107 Date Tested: T SAND and GRAVEL, some silt Date Tested: BH21-03 Colden, BC Date Tested: Golden, BC Date Tested: Tested By: Ms Passing 100 0 <t< td=""><td>Kicking Horse Bridges Sample No.: SP Ministry of Transportation and Infrastructure Date Sampled By: Mailed By: Total asymptotic Sample No.: Sample No.: SP Date Tested By: MS asymptotic Solden, BC No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: m: Percent 100 9 9 No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 0</td><td>Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2 704-ENG.VGE003793-01.107 Date Sampled By: TC Inc. i: SAND and GRAVEL, some silt Date Sampled By: TC Inc. BH21-03 Golden, BC Date Sampled By: Inc. on: </td><td>Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Tested By: MS Lab: Ca No. Crushed Faces: Moisture Content: 10.6% On: 9 9 9 Passing 9 9 9 9 Passing 9 9 9 9 9 Passing 9 <t< td=""><td>Kicking Horse Bridges Sample No:: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 No: Output Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:</td></t<></td></t<>	Kicking Horse Bridges Sample No.: SP Ministry of Transportation and Infrastructure Date Sampled By: Mailed By: Total asymptotic Sample No.: Sample No.: SP Date Tested By: MS asymptotic Solden, BC No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: m: Percent 100 9 9 No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 No. Crushed Faces: No. Crushed Faces: No. Crushed Faces: mister Content: 100 9 9 0	Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2 704-ENG.VGE003793-01.107 Date Sampled By: TC Inc. i: SAND and GRAVEL, some silt Date Sampled By: TC Inc. BH21-03 Golden, BC Date Sampled By: Inc. on:	Kicking Horse Bridges Sample No.: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: SPT 7 Date Sampled: March, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Sample No.: March, 2021 Sample No.: April 27, 2021 Tested By: MS Lab: Ca No. Crushed Faces: Moisture Content: 10.6% On: 9 9 9 Passing 9 9 9 9 Passing 9 9 9 9 9 Passing 9 <t< td=""><td>Kicking Horse Bridges Sample No:: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 No: Output Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:</td></t<>	Kicking Horse Bridges Sample No:: SPT 7 Ministry of Transportation and Infrastructure Date Sampled: March, 2021 Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: SPT 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:: Set 7 No: Output Date Sample No:: Set 7 Date Sample No:: Set 7 Date Sample No:

Data presented hereon is for the sole use of the stipulated client. Tetra Tech is not responsible, nor can be held liable, for use made of this report by any other party, with or without the knowledge of Tetra Tech. The testing services reported herein have been performed to recognized industry standards, unless noted. No other warranty is made. These data do not include or represent any interpretation or opinion of specification compliance or material suitability. Should engineering interpretation be required, Tetra Tech will provide it upon written request.



SIEVE ANALYSIS REPORT

							ŀ	ASTM C1	136, C11	/							
Project:		Kickir	ng Ho	rse B	sridges	3				Sam	ple No	.:	SPT	9			
Client:		Minis	try of	Tran	sporta	tion an	nd Infra	astructu	re	Date	e Samp	led:	Marc	:h, 2(021		
Project No	0.:	704-E	ENG.\	/GEC	00379	3-01.1	07			Sam	pled B	y:	TTC	Inc.			
Attention:										Date	e Teste	d:	April	27, 2	2021		
Descriptio	on:	SANE) and	GRA	VEL,	some s	silt			Test	ted By:	MS	I	∟ab:	Cal	lgary	
Source:		BH21	-03							No.	Crushe	d Face	es:				
Location:		Golde	en, BO)						Mois	sture Co	ontent		6.7%	, D		
Specificat	ion:												_				
	T																
Sieve Size	Perc	cent		100					1		1						7
200 mm	1 43	Sing		00								/					
200 mm				90													
100 mm			Р	80								/		++		+	_
80 mm			e	70							/						
50 mm			C	70													
40 mm			e	60		+	+	<u> </u>			/			++	+	+	_
25 mm			t	50													
20 mm	10	00	D	50													
16 mm	9	3	a	40		+	+	<u> </u>						++		+	-
12.5 mm	8	7	S	30													
10 mm	8	0	i	30													
5 mm	5	9	n	20		\sim	+							++	—	+	_
2.5 mm	4	9	y	10													
1.25 mm	3	8		10													
630 µm	3	0		0													
315 µm	2	5		Ċ	∞ ∕	(um)	315	630	1.25	C.7	ς 2	12.5 12.5 16	25	50 50	80	100	200
160 µm	2	1				— (μm)) ——		<u> </u>	Sieve	Size (_mm) -					7
80 µm	18	8.0															
Remarks:	:																
								R	eviewe	d By:						P	.Eng.
																	5

Data presented hereon is for the sole use of the stipulated client. Tetra Tech is not responsible, nor can be held liable, for use made of this report by any other party, with or without the knowledge of Tetra Tech. The testing services reported herein have been performed to recognized industry standards, unless noted. No other warranty is made. These data do not include or represent any interpretation or opinion of specification compliance or material suitability. Should engineering interpretation be required, Tetra Tech will provide it upon written request.


						A	STM C	136, C11	7								
Project:	Ki	cking Ho	orse E	Bridges	6				Sam	ple No.	:	SP	T 14				
Client:	М	inistry of	Tran	sporta	tion an	d Infra	structu	ure	Date	Sample	ed:	Ма	rch, 2	2021			
Project No.	.: 70)4-ENG.'	/GE(20379	3-01.10)7			Sam	pled By	<i>'</i> :	TT	C Inc				
Attention:									Date	Tested	ł:	Apr	·il 27,	, 202	1		
Descriptior	n: S/	AND, gra	velly	, silty					Test	ed By:	MS		Lab): C	algar	у	
Source:	BI	H21-03							No. (Crushed	d Face	es:	-			-	
Location:	G	olden, B	С						Mois	sture Co	ontent		9.2	%			
Specificatio	on:																
· 																	
Sieve Size	Percer Passin	nt	100		1	1			1	r r		 _					
200 mm		9	00								X						
200 mm		_	90														
100 mm		Р	80			-	-					_	+	\vdash		++	
80 mm		e r	70														
50 mm		c c	70														
40 mm		e n	60				-						_			++	
25 mm		t	50					1									
20 mm	100	Р	50														
16 mm	96	a	40									-	+	+		+	
12.5 mm	92	S S	30			1							\perp				
10 mm	87	i	20														
5 mm	72	n a	20				_									+	
2.5 mm	64		10														
1.25 mm	56	_															
630 µm	4/		0	 0	 	<u>ر</u>	0	י <u>ה</u> י	<u>ເ</u>	ν ο ο	م بہ ہ	نہ 	 ; o	 		 。 。	
315 µm	30 26	_		~	≝ — (µm)	31	<u>وی</u>		N	(1	' <u>2</u> ' - mm) -	6		u) 0	10	\bigvee_{20}^{15}	
80 um	20 21 २	_			• •		•	-	Sieve	Size						-	
00 µm	21.0																
Remarks:																	
) Oviour-	d Dur				24	1		р г.,	
							ŀ	(eviewe	а ву:_	/	20					P.Er	ıg.



				AS	TM C136, C	117							
Project:	Kicking Ho	orse Brido	jes			San	nple No.	:	SP	Г 16			
Client:	Ministry of	Transpo	rtation and	d Infrast	ructure	Date	e Sampl	ed:	Mai	rch, 2	2021		
Project No.:	704-ENG.	VGEO03	793-01.10)7		San	npled By	/:	TTC	C Inc.			
Attention:						Date	e Testeo	d:	Apr	il 27,	2021		
Description:	SAND, silt	y				Tes	ted By:	MS	<u> </u>	Lab	: Ca	algary	
Source:	BH21-03	-				No.	Crushe	d Fac	es:				
Location:	Golden, B	С				Moi	sture Co	ontent		17.0)%		
Specification:													
Sieve Size Pe	ercent Issing	100											
200 mm		90 —		\square					_				
150 mm													
100 mm	P	80 —											
80 mm	r e	70 —	/										
50 mm	C												
40 mm	n e	60	+/-										
25 mm	t	50 —	_/										
20 mm	Р		Λ										
16 mm	a	40											
12.5 mm	S	30							_	_			
10 mm	100 i												
5 mm	100 n 100 g	20											
2.5 mm	100	10 —								_			
1.25 mm	99												
315 um	80	0 _		30 -		i.	ر م	16 15	25			38 8	
160 um	47	←	(μm)	ო დ 	$\rightarrow \overleftarrow{\leftarrow}$	0	(mm) ⁻					\rightarrow
80 µm 3	31.4					Sieve	Size						
Remarks:													
										_			
					Revie	wed By:	~	5	i	H		F	P.Eng.
					-	,							J



					ASTM (C136, C11	7							
Project:	Kicking	Horse Bri	idges				Sam	ple No	.:	SP	'T 19			
Client:	Ministry	of Transp	portation a	and Inf	rastruct	ure	Date	e Samp	led:	Ma	irch, 2	2021		
Project No.:	704-EN	G.VGEO)3793-01.	107			Sam	pled B	y:	ΤT	C Inc			
Attention:							Date	e Teste	d:	Ар	ril 27,	, 202 [,]	1	
Description:	SAND, s	silty, trace	eclay				Test	ed By:	M	S	Lab	: C;	algar	у
Source:	BH21-03	3					No.	Crushe	d Fa	ces:				
Location:	Golden,	BC					Mois	sture C	onten	t:	24.	5%		
Specification	า:													
Sieve Size F	Percent Passing	¹⁰⁰												
200 mm		90 -		Д—										<u> </u>
150 mm														
100 mm		P 80												
80 mm		r 70 -	-/-											
50 mm		c												
40 mm		e ₆₀ n	1											+
25 mm		t 50	/	\perp							\rightarrow			
20 mm		Р												
16 mm		a 40		+										
12.5 mm		s s 30 -												
10 mm		i												
5 mm		n 20 a												
2.5 mm	100	10												
1.25 mm	100													
630 µm	100	0 L		<u>ې</u>		<u>د</u>	ν.	· در	لـــــ س در	, vo		 。 。	 > 0	 。。
315 µm	96	~	<u>ور</u> ۱۳) <u>ح</u>	m)			6		- <u>5</u> - (mm)	- 7	4	<u>v</u> »	10 °	\bigvee_{20}^{15}
160 μm	19			,			Sieve	Size	(IIIII)					-
60 µm	40.0													
Remarks:														
											201			
					I	Reviewe	ed By:		>	and a	1			P.Eng



M C136 C117

	ASTM C13	ю, С117
Project:	Kicking Horse Bridges	Sample No.: SPT 22
Client:	Ministry of Transportation and Infrastructure	Date Sampled: March, 2021
Project No.:	704-ENG.VGEO03793-01.107	Sampled By: TTC Inc.
Attention:		Date Tested: April 27, 2021
Description:	SAND, silty, trace clay	Tested By: MS Lab: Calgary
Source:	BH21-03	No. Crushed Faces:
Location:	Golden, BC	Moisture Content: 21.8%
Specification:		
Sieve Size Pe Pa	ercent 100	
200 mm	90	
150 mm		
100 mm	P 80	
80 mm	r 70	
50 mm	C	
40 mm	n 60	
25 mm	t 50	
20 mm	P	
16 mm	a 40	
12.5 mm	s 30	
10 mm		
5 mm	g g	
2.5 mm	10	
1.25 mm	100	
215 um		25 25 10 10 10 10 10 10 10 10 10 10 10 10 10
160 um	$\frac{30}{64} \qquad \qquad$	(mm)
80 µm 3	34 7	Sieve Size
Remarks:		
	P .	and
	Re	Viewed By: P.Eng.



						A	ASTM C	136, C11	7								
Project:	Kic	king Ho	orse B	ridges	3				Sam	nple No	.:	SP	T 1				
Client:	Mi	nistry of	Trans	sporta	tion an	d Infra	astructu	ire	Date	e Samp	led:	Ma	rch, 2	2021			
Project No.	.: 70	4-ENG.	VGEC	0379	3-01.10	07			Sam	npled B	y:	ТТ	C Inc				
Attention:									Date	e Teste	d:	Ap	ril 27,	202 ⁻	1		
Descriptior	n: SA	ND and	I GRA	VEL,	some s	silt			Tes	ted By:	MS	5	Lab	: C	algary	/	
Source:	BH	121-05							No.	Crushe	d Fac	es:					
Location:	Go	lden, B	С						Mois	sture Co	ontent	t:	3.19	%			
Specificatio	on:																
·																	
Sieve Size	Percen Passing	t	100								гтт						
200 mm		5	00														
150 mm			90														
100 mm		P	80								И		_			+	
80 mm		e r	70														
50 mm		c	/0														
40 mm		e n	60							1						+	
25 mm	100	t	50														
20 mm	93	Р	00														
16 mm	85	a	40					1									
12.5 mm	80	s s	30														
10 mm	77	i				1											
5 mm	62	n a	20														
2.5 mm	53	ĭ	10														
1.25 mm	42																
630 µm	34	_	0		。	<u>м</u>	 0	ν ν	رب م	<u>г</u> ой	с с v o	С			 > 0		
315 µm	28		с	。 ←—	9 — (µm)		²³		6.	(- <u></u> mm)	-0	4 1	n o	10	\bigvee_{20}^{15}	
100 µm	10.7						-		Sieve	Size	,)					-	
ου μπ	19.7																
Remarks:																	
							R	eviewe	ed By:	5		2	Z			P.En	g.
									-			_					-



		ASTMC	136, C117		
Project:	Kicking Ho	orse Bridges	5	Sample No.:	SPT 1
Client:	Ministry of	f Transportation and Infrastruct	re D	Date Sampled:	March, 2021
Project No.:	704-ENG.	VGEO03793-01.107	5	Sampled By:	TTC Inc.
Attention:				Date Tested:	April 27, 2021
Description:	SAND, silt	ty, some gravel		Tested By: MS	Lab: Calgary
Source:	BH21-08			No. Crushed Fac	es:
Location:	Golden, B	С	N	Moisture Content	13.6%
Specification:					
·					
Sieve Size Pe	rcent	100	<u> </u>		
200 mm	33119	00			
150 mm		90			
100 mm	Р	80			
80 mm	e	70			
50 mm	c				
40 mm	e	60			
25 mm	t	50			
20 mm	Р				
16 mm 1	100 a	40			
12.5 mm	91 s	30			
10 mm	88 i	30			
5 mm	82 n	20			
2.5 mm	76 ⁹	10			
1.25 mm	68				
630 µm	61				
315 µm	55	23 31, 16 ∞ 23 31, 16 ∞	2.5	12, 12, 12, 12, 14, 14, 14, 14, 14, 14, 14, 14, 14, 14	200 200 200 200 200 200 200 200 200 200
160 µm	49		Sie	eve Size	
80 µm 4	1.5				
Remarks:					
		F	eviewed I	By:	P.Eng.



M C136 C117

						F	AS TIVI C	136, C11	17								
Project:	Ki	cking Ho	orse B	ridges	5				San	nple No.	:	SP	Т2				
Client:	M	inistry of	Tran	sporta	tion an	d Infra	struct	ure	Date	e Sampl	ed:	Ma	rch, 2	021			
Project No	o.: <u>70</u>	4-ENG.	VGEC	00379	3-01.10)7			San	npled By	/:	TT	C Inc.	1			
Attention:									Date	e Testec	d:	Apr	il 27,	2021			
Description	n: Gl	RAVEL	and S	AND,	trace si	ilt			Tes	ted By:	MS		Lab:	Ca	lgary		
Source:	Bł	H21-09							No.	Crushee	d Fac	es:					
Location:	G	olden, B	С						Moi	sture Co	ontent	:	8.1%	6			
Specificati	on:																
· · · · · · · · · · · · · · · · · · ·		1															
Sieve Size	Percer Passin	nt g	100														
200 mm			90								++	\mathbb{H}			-		
150 mm			90														
100 mm		P e	80														
80 mm		r	70				_			+	++				+		
50 mm		C e	(0)														
40 mm		n	60														
25 mm	100	t	50			-		-							+		
20 mm	88	Р	10						1								
16 mm	84	a	40														
12.5 mm	80	S	30					1			++	_			+		
10 mm	75	i	20				1										
5 mm	59	g	20			ſ											
2.5 mm	47		10					_			++	_					
1.20 mm	25		0														
315 um	20	_	0	 08	 09	15	30	25		<u>ہ</u>	10 16	25	40			00	
160 um	16			←	_ (μm)	ς. 	<u>و</u>		(1	(1	되 mm) ·					$\overset{\sim}{\sim}$	
80 µm	13.7								Sieve	Size							
Remarks:																	
							F	Reviewe	ed By:		- A	1	Ð		F	P.En	J.



							A	STM C	136, C11	17								
Project:	ł	Kickiı	ng Ho	rse E	Bridge	S				Sam	ple No.	:	SP	Т7				
Client:		Minis	try of	Tran	sporta	ation an	nd Infra	struct	ure	Date	e Sampl	ed:	Ma	rch, 2	2021			
Project No	o.: 7	704-E	ENG.\	/GEC	00379	3-01.1	07			Sam	pled By	/:	тт	C Inc				
Attention:	_									Date	Tested	d:	Apr	il 27,	, 202	1		
Descriptio	n: S	SANI	D, sor	ne gr	avel,	some s	silt			Test	ed By:	MS		Lab): C	algar	Y	
Source:	I	BH21	-09							No.	Crushe	d Face	es:	_				
Location:	(Golde	en, BO)						Mois	sture Co	ontent:		8.4	%			
Specificati	ion:																	
·	-	1																
Sieve Size	Perc Pass	ent ing		100										\neg				
200 mm				90					_			1		\perp	<u> </u>	\square		
150 mm																		
100 mm			P	80			1						+	+	1	\parallel	++	
80 mm			e r	70						<u> </u>			_	\perp	<u> </u>	\square		
50 mm			C															
40 mm			e n	60					\boldsymbol{X}					+	1			
25 mm			t	50				-/	′					\perp	<u> </u>	\square		
20 mm	10	0	Р															
16 mm	98	3	а	40				\boldsymbol{X}	_					+	\square			
12.5 mm	93	3	s s	30			+/							\perp				
10 mm	92	2	i				\boldsymbol{V}											
5 mm	82	2	n g	20										+	1			
2.5 mm	74	+		10		1			_					\perp				
1.25 mm	20	1		0														
215 um	21	,		0	ـــــــ ∞	00	15			i i	v c	ور به او او نه او	25		<u> </u>	<u>3</u> 8	<u> </u>	
160 um	14	1			~	 (μm))	<u> </u>	<u> </u>	0	(1	≌ mm) -					$\stackrel{\scriptstyle 5}{\leftarrow}$	
80 um	11.	.8								Sieve	Size							
00 µ		U																
Remarks:	-																	
														2			-	
								F	Review	ed By:	2		N	2			_P.Er	ng.



PERCENT FINER THAN # 200 SIEVE FORM

ASTM C117 Aggregate, ASTM D1140 Soils

Project:	Kicking Horse Bridges	Sample Number:	Various
Client:	Ministry of Transportation and Infrastructure	_Sampled By:	TTC Inc.
Location	Golden, BC	Date Tested:	27-Apr
Project N	lumber: 704-ENG.VGE003793-01.107	_Tested By:	MS/IB

Borehole:	BH21-03	BH21-03	BH21-05	BH21-05	BH21-09		
Depth	SPT 18	SPT 23	G1	G3	SPT 1		
Wet Mass & Tare (g)	995.4	670.3	1823.4	2512.8	324.3		
Tare Mass (g)	489	0	0	0	0		
Wet Mass (g)	506.4	670.3	1823.4	2512.8	324.3		
-							
Initial Mass & Tare (g)	412.7	537.96	1730.1	2466	307.1		
Tare Mass (g)	0	0	0	0	0		
Initial Mass (g)	412.7	537.96	1730.1	2466	307.1		
						-	
Washed Mass & Tare (g)	715.6	779.1	1396.2	2160.9	227.5		
Tare Mass (g)	489	488.7	0	0	0		
Washed Mass (g)	226.6	290.4	1396.2	2160.9	227.5		
Moisture Content (%)	22.7	24.6	5.4	1.9	5.6		
Mass Passing #200 (g)	186.1	247.56	333.9	305.1	79.6		
Percent Passing #200	45.1	46.0	19.3	12.4	25.9		

Remarks:

Reviewed By:

P.Eng.

Sai



						ASTM	C136,	C117								
Project:	Kickir	ng Hoi	rse Brid	ges					Sam	ple No	.:	S6				
Client:	Minis	try of	Transpo	ortation	and Inf	rastru	cture		Date	Samp	led:	Sep	otemb	er 17	, 202 [.]	ł
Project No.:	704-E	ENG.V	GEO03	3793-01	.203				Sam	pled By	y:	KB				
Attention:									Date	Teste	d:	Oct	ober 1	7, 202	21	
Description:	SAN) and	GRAVE	EL, som	ne silt				Test	ed By:	MS		Lab:	Ca	lgary	
Source:	BH21	-01							No. (- Crushe	d Fac	es:				
Location:	Golde	en. BC	;						Mois	ture Co	ontent		8.0%	, o		
Specification	:	,						+2						-		
								-								
Sieve Size P	ercent assing		100													-
200 mm			90 -									┛	_ -	_		_
150 mm											ИI					
100 mm		Р	80													-
80 mm		r	70 -								┝╌╽╌╎					_
50 mm		C								\mathbf{V}						
40 mm		n	60						7							
25 mm	100	t	50 -								╎╾╎━╎				┥╾┥	_
20 mm	91	Р														
16 mm	91	а	40								╞╼┼╸					
12.5 mm	86	5 5	30			\mathcal{A}		-+			┝┼╌┼		_ -	_	$\left - \right $	
10 mm	80	i														
5 mm	61	g	20													-
2.5 mm	49		10 -					-+			╞╌┼╌┼	_	_	\rightarrow	+	_
1.25 mm	40															
315 um	25		0 5		15	e e	5	5	1	<u> </u>	0 5 0	25	6 ç		8 8	8
160 µm	18		←		um) —	igi G	→ _<		•	(드 (mm)					- ~i >
80 µm	14.6							:	Sieve	Size						
Remarks:							Revi	iewe	d By:_	Q	101	U	ller	lue	2F	P.Eng.



ASTM C136, C117

Project:	ĸ	lickin	a Hoi	se Bri	dges					Sar	nple No).:	S2				
Client:	N	linist	rv of	Transi	ortat	ion an	d Infra	astruct	ure	Dat	e Sam	oled:	Ser	otemb	er 17	. 20	21
Project No.	.: 7	04-E	NG.V	GEO)3793	3-01.20)3			Sar	npled E	ly:	KB			-	
Attention:										Dat	e Teste	ed:	Oci	ober '	7, 20	21	
Description	n: S) and	GRA	EL. s	ome s	silt			Tes	sted Bv:	MS	;	Lab:	Са	Igan	v
Source:	"≥≓ B	H21	-01		, -					No.	Crush	ed Fac	es:				·
Location:	р= С	Golde	n. BC							Mo	isture C	onten		6.2%	, D		
Snecificatio		.0.40	,														
opecificatio							_										
Sieve Size	Perce Passi	ent ng		¹⁰⁰ [1	
200 mm				90				_		_		- -	-{-				
150 mm			_														
100 mm			P	80								N					
80 mm			r	70				_	_	-		H			<u> </u> .		+
50 mm		_	C														
40 mm	100)	n	00 -							X						
25 mm	94		t	50					_	+		+	_		-+		
20 mm	84		P	40													
16 mm	80		8	40					X								
12.5 mm	. 73		5	30					-			┥┥┥	-+		-+		
10 mm	70 57		i n	20													
5 mm	57		9	20													
1.05 mm	40			10				_	_			┨╾┼╼╂			-+	+-	+
630 um	28																
315 um	23			202		09	315	130	:25	2.5	5	10 25	25	40	8 8	88	150
160 µm	19			-		_ _ (μm) —		→ ~			(mm)					\rightarrow
80 µm	16.	2								Sieve	e Size						
Remarks:	10.	~							D i-		Ø	1,10	m	100	11	0-	PEn



ASTM C136, C11		
	7	
	Sample No.:	S9
n and Infrastructure	Date Sampled:	September 17, 2021
1.203	Sampled By:	КВ
	Date Tested:	October 7, 2021
ce silt	Tested By: MS	Lab: Calgary
	No. Crushed Face	es:
	Moisture Content:	5.9%
		X
	+ $+$ $+$ $//$	
	<u>├ </u>	
	┼╱┼──┼┼┼	
	╉──┼─┼┼┼	
630 (1.25	2.5 5 12.5 12.5 166	25 50 100 150 200 200 200
(km)	Sieve Size	
	(μm)	Sampled By: Date Tested: Tested By: MS No. Crushed Face Moisture Content:



				SI	EVE A	NALY	'SIS RE	PORT							
					A	SIMC	136, C11	17							
Project:	Kicking	Horse	Bridge	S				Sam	ple No.	•	S-16	j			
Client:	Ministry	of Tra	nsporta	ition an	d Infra	structu	Ire	Date	samp	led:	Sept	emb	er 17,	2021	
Project No.:	704-EN	G.VGE	00379	3-01.20)3			Sam	pled By	/:	KB				
Attention:								Date	Teste	d:	Octo	ber 7	, 202	1	
Description:	GRAVE	Land	SAND,	silty, so	ome cla	ay		Test	ed By:	MS		Lab:	Cal	jary	
Source:	BH21-0	1						No.	Crushe	d Fac	es:				
ocation:	Golden,	BC						Mois	sture Co	ontent		4.8%)		
Specification:															
Sieve Size Pe	ssing	100					Τ					Π			1
200 mm		90	<u> </u>	_						- -		4		- -	-
150 mm		_ 00													
100 mm		р 80 е													1
80 mm		r 70				-	-					_ - -			-
50 mm		C C													
40 mm	100	e 60							1						1
25 mm	80	t 50	,			_									-
20 mm	78	P													
16 mm	75	a ⁴⁰													
12.5 mm	72	s 30		\leftarrow	1		_			$\left \cdot \right $		_ - -		_	-
10 mm	69	i													
5 mm	60	n 20 g)				-								1
2.5 mm	52	10	,									_ - -		-	-
1.25 mm	45														
630 µm	39	0			2	<u> </u>	52	<u>ن</u>	<u>.</u>	م رہ <u>د</u>	5	<u> </u>		<u> </u>	2
160 um	33		~	≝ — (µm))		⊡ ←	2	(-≌- mm)	C1	-1 47		= <u>*</u>	ਸ •
	27.6					-	-	Sieve	Size	/					
80 μm 2 Remarks:	27.6								0						



Proiect:							01111-0	100, 01									
Jaan	Kickir	ng Ho	rse Brid	ges					Sar	nple No	o.:	S	S23				
Client:	Minis	try of	Transpo	ortatio	on and	l Infra	structu	Ire	Dat	e Sam	oled:	S	epten	nber	17, 2	021	
Project No.:	704 - E	ENG.V	/GEO03	3793-	01.20	3			Sar	npled E	By:	KI	3				
Attention:									Dat	e Teste	ed:	October 7, 2021					
Description:	GRA	VEL, s	sandy, s	ome	silt				Tes	sted By	: M	s	La	b:	Calga	ary	
Source:	BH21	-01							No.	Crush	ed Fa	ces:					
Location:	Golde	en, BC)						Moi	isture C	onter	nt:	6.	9%			
Specification:																-	
Siovo Sizo Pe	ercent		100														
Pa	issing																
200 mm			90 -	-+							┼┼╴		A		╶┼╾┝╸		
150 mm		D	80														
100 mm		e															
80 mm		r	70											-			
50 mm	100	e	60						_		К						
40 mm	94	n +									1						
20 mm	74		50 -			<u>87</u>								-	-[-]-	- -	
16 mm	67	P	40				<u> </u>	_		_	\parallel						
12.5 mm	63	S															
10 mm	59	s i	30					1						1			
5 mm	48	n	20 -					_	_	_				_ _	_ _ -		
2.5 mm	40	g		_													
1.25 mm	34		10				1										
630 µm	28		0 L					1						<u> </u>			
315 µm	21		80	160		cle	630	1.25	2.5	ŝ	12.5	01	(); ()	50	80 100	, 150 200	
160 µm	15		~		· (µm)		\rightarrow	\leftarrow	Sieve	Size	(mm)					\rightarrow	
	12.6								01676	. 0.20							



ASTM C136, C117

								701													
Project:	k	licking	Но	rse Br	idges	5					S	amı	ple No).:	-	S28					
Client:	Ν	/linistry	of	Trans	porta	tion a	nd Inf	rastr	uctur	e	D	ate	Samp	oled:	1	Sept	temb	ber 1	17, 2	021	
Project No.	.: 7	'04-EN	G.\	/GEO	0379	3-01.2	203				S	am	pled B	y:	1	KB					
Attention:											D	ate	Teste	d:	(Octo	ber	7, 2	:021		
Description	n: G	RAVE	L, s	sandy,	som	ne silt					Т	este	ed By:	N	IS		Lab:	:C	alga	гу	
Source:	E	3H21-0	1								N	lo. C	Crushe	ed Fa	ices	s:					
Location:	G	Golden,	BC								N	loisi	ture C	onte	nt:		5.6%	%			
Specificatio	on:																				
Sieve Size	Perce Passi	ent ing		¹⁰⁰ [Τ	17		ТГ		
200 mm				90								_			┢	-	-#		┼┼	_	
150 mm			_	00													Δ				
100 mm			P	8U												Z					
80 mm			r	70		-								+		4_	-+	┣—	┼┼		ł
50 mm	100	D	C							ļ					V						
40 mm	85		n	60											1						
25 mm	76		t	50			_			<u> </u>	_			4-	┢			┣—	++-		
20 mm	69		Ρ																		
16 mm	61		a	40		-					1										
12.5 mm	55		S	30			-	\rightarrow	_	[-	_		┼╌┼╼		- -	_ -				
10 mm	52		i																		
5 mm	44	, 	g	20																	
2.5 mm	30	•		10		-	_							$\left \cdot \right $	-			┢			
1.25 mm	 																				
315 um	22			U L	3	. 09	15	30		<u>ุ</u> ม	5.5	ų	n	10	16	25	40	20	00	50	3
160 um	16				<	 (μn	n) —	Ú	\rightarrow	$\stackrel{\sim}{\leftarrow}$	C1			<u></u> (mm)) —					\rightarrow	4
80 µm	12.	7									Sie	ve s	Size								
Dementer																					
Remarks:	-																				
						0.0															
													A					,			
									R	eview	ed E	3v:-	U	w	n	U	le	U	0	P.B	Ena
																				_	



ASTM C136, C117

Proiect:	Kickina Horse Bridaes	Sample No.: S34
Client:	Ministry of Transportation and Infrastructure	Date Sampled: September 17, 2021
Project No :	704-ENG VGE003793-01.203	Sampled By: KB
Attention:	104 2103.102000700 01.200	Date Tested: October 7, 2021
Description	SAND silty some clay	Tested By: MS Lab: Calgary
Source:	BH21-01	No Crushed Faces:
Location:	Golden BC	Moisture Content: 19.2%
Specification		
opecification		
Sieve Size P	ercent 100	
200 mm	90	
150 mm		
100 mm	P 80	
80 mm	r 70	
50 mm	c	
40 mm	e 60 n	
25 mm	t 50	
20 mm	P	
16 mm	a 40	
12.5 mm	s 30	
10 mm	i	
5 mm	n 20	
2.5 mm	100 10	
1.25 mm	100	
630 µm		
315 µm	² ² ² ² ³ ² ² ³ ² ² ³ ² ³ ² ³ ² ³ ² ³	(mm) (mm) (mm) (mm)
	60	Sieve Size
	20.3	
Remarks:		
		0.
	Re	viewed By: Montelle P.Eng.
		inclusion and in



							ASTM C	136, C11	7							
Project:	Kic	king Ho	rse B	ridges					Sar	nple N	lo.:	S2				
Client:	Min	nistry of	Trans	portat	ion an	d Infra	structu	ire	Dat	te San	npled:	Se	ptemb	per 17	7, 202	21
Project No	.: 704	I-ENG.	/GEO	03793	3-01.20)3			Sar	mpled	By:	KB				
Attention:									Dat	te Tes	ted:	Oc	tober	7, 20	21	
Description	n: GR	AVEL, s	sandy	, som	e silt				Tes	sted B	y: <u>M</u> S	S	Lab	: Ca	algary	/
Source:	BH	21-04							No.	. Crusl	ned Fad	es:				
Location:	Go	lden, BC	>						Мо	isture	Conten	t:	0.79	%		
Specificati	on:															
		1														
Sieve Size	Percent Passing		100		<u> </u>		1								Т	
200 mm]	90							-	_ - -	+				
150 mm			00									/				
100 mm		P	80													
80 mm		_ r	70				_									
50 mm		C e	60													
40 mm		_ n	00													\square
25 mm	100	- t	50				_			+					-	+
20 mm	90	_ P	40													
16 mm	76	a	40													\square
12.5 mm	60	- s	30				_		<u> </u>		_ - - -	-				+
	20	- i	20					1								
25 mm	35	g	20				1									
1 25 mm	26		10					_				-+				$\left - \right $
630 um	21	-	0													::
315 µm	16		0	2	160	315	\$30	-25	2.5	Ś	10	32 25	40	0, 0	20	150
160 µm	14	t.		<u> </u>	— (μm))	\rightarrow	~			(mm)					\rightarrow
80 µm	11.4								Sieve	e Size						
Remarks:					0.0											

Reviewed By: Monuclue P.Eng.



ASTM C136, C117

^o roject:	Kickin	ig Ho	rse Brid	ges					Sam	ple No	* *	S3				
Client:	Minist	ry of	Transpo	ortation	and In	nfrastr	ucture	~	Date	Samp	led:	Sep	otemb	per 1	7, 20	21
Project No.:	704-E	NG.	/GEO03	8793-01	.203				Sam	pled By	y:	KB				
Attention:									Date	Teste	d:	Oct	ober	7, 20)21	
Description:	GRAV	/EL, s	sandy, s	ome sil	it				Test	ed By:	MS	3	Lab:	Ca	algar	у
Source:	BH21	-06							No. (Crushe	d Fac	es:	-			
ocation:	Golde	en, BC)						Mois	ture Co	onten	t:	1.6%	6		
Specification:																
Sieve Size	ercent		100					·····								
200 mm			90 L													
150 mm																
100 mm		Ρ	80				-+	-+				//			_	+-
80 mm		e r	70									Λ				
50 mm		c														
40 mm	100	e	60					-+			⊢					+-1
25 mm	76	t	50								Ζ					
20 mm	70	Р							!							
16 mm	61	а	40 -					-+								
12.5 mm	52	S	30													
10 mm	46	i						1								
5 mm	36	n	20			\rightarrow	$ \frown$								_	
2.5 mm	29	3	10													
1.25 mm	23															
630 µm	19															
315 µm	17		8	<u> </u>	- (m) 	630	1:25	2.5	u	· · ·	12.5	5	40		00	200
160 µm	15			— (þ	, —		~ ~	s	ieve S	(Size	mm)					~
80 µm	2.8															
Remarks:							_									
										0				0		
									_	101	IN	11	101	11	2/	



ASTM C136, C117

	Kickin	g Ho	rse Brid	jes				Sam	ple No.	•	S2				
Client:	Minist	ry of	Transpo	rtation a	and Inf	rastruct	ure	Date	Sampl	ed:	Se	ptemb	oer 17	7, 202	21
Project No.:	704-E	NG.\	/GE003	793-01.	203			Sam	pled By	Γ.	KB				
Attention:								Date	Tested	d:	Oc	tober	7, 20	21	
Description:	GRAV	/EL, s	sandy, tr	ace silt				Test	ed By:	MS)	Lab	Ca	lgary	,
Source:	BH21	-07						No.	Crushe	d Fac	es:				
Location:	Golde	n, BC	>					Mois	sture Co	ontent	4	2.5%	%		
Specification:															
Sieve Size Pe Pa	rcent ssing		100								Λ				
200 mm			90				_	<u> </u>			\square			_	
150 mm															
100 mm		P	80							- -/					
80 mm		е Г	70				_	<u> </u>		- /-				_	Ц
50 mm		C								X					
40 mm		e n	60												
25 mm 1	00	t	50				_								
20 mm	92	Р													
16 mm	76	a	40								-+			-	
12.5 mm	65	S	30			_					_				
10 mm	59	i						1							
5 mm	39	n a	20		-										
2.5 mm	28	Ĵ	10				_								
1.25 mm	20														
630 µm	14		0 5			- Q	<u>رہ</u>	<u>и</u>	v c	یں ہو م			e e		ب
315 µm	7		~	≌ µ) —	m) —			ri		<u>ຕ່</u> mm)	(1)	44	n 60	0	→ 2 22
160								Sieve	Size)					-
160 µm	in, in, i														

