

CANADA - BRITISH COLUMBIA

FRASER RIVER FLOOD CONTROL 1968 AGREEMENT

Inventory and Engineering Branch
Ministry of Environment
Victoria, British Columbia

Water Planning and Management Branch
Inland Waters Directorate
Pacific Region
Department of the Environment
Vancouver, British Columbia

DISTRICT OF SURREY

Operation and Maintenance Instructions
Flood Control Works

Volume 3

As Constructed Works

Crippen Consultants
1605 Hamilton Avenue
North Vancouver, B.C.
V7P 2L9

District of Surrey

**Operation and Maintenance Instructions
Flood Control Works**

THE MANUAL

The operation and maintenance instructions for the flood control works for the District of District of Surrey are provided in two volumes:

VOLUME I GENERAL INSTRUCTIONS

VOLUME 3 AS-CONSTRUCTED WORKS

District of Surrey
Operation and Maintenance Instructions
Flood Control Works

Volume 3

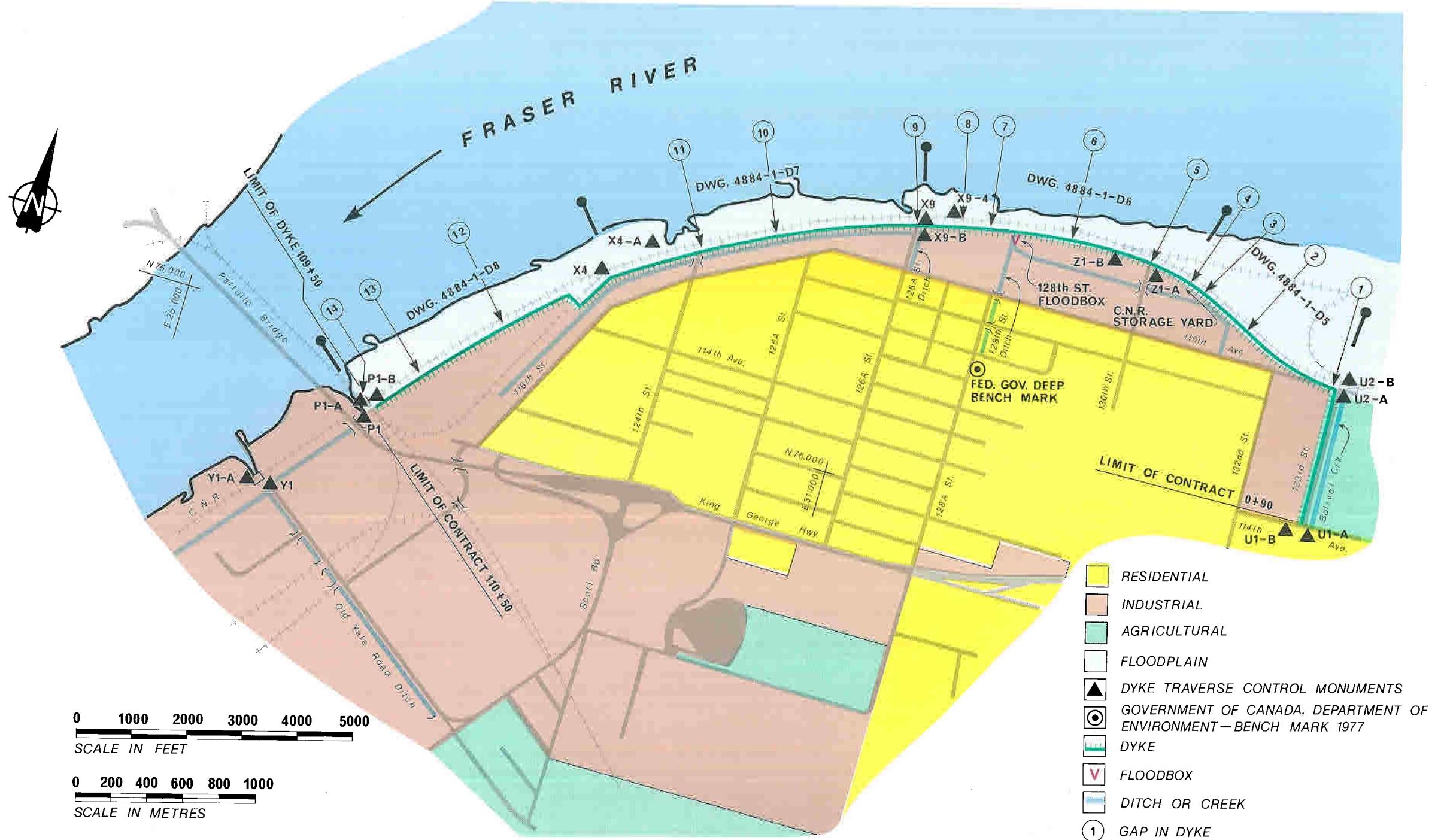
As Constructed Works

Contents

| | <u>Page</u> |
|-------------------------------|------------------------------------|
| Title Page | i |
| Record of Amendments | ii |
| The Manual | iii |
| Volume 3 - Contents | iv |
| Volume 3 - List of Appendices | v |
| Volume 3 - Key Plan | vi |
| | |
| 1. | DESCRIPTION OF CONSTRUCTED WORKS |
| 1.1 | Dyke |
| 1.1.1 | General |
| 1.1.2 | Embankment |
| 1.1.3 | Concrete Wall |
| 1.2 | Outlet Works 128th Street Floodbox |
| 1.2.1 | Foundation |
| 1.2.2 | Concrete |
| 1.2.3 | Pipe |
| 1.2.4 | Riprap |
| 1.2.5 | Inlet Channel |
| | 3 |
| | 4 |
| | 4 |
| | 5 |
| | 5 |
| | 5 |
| | 5 |
| 2. | As Constructed Drawings |
| 2.1 | As-Constructed Drawing List |
| | 2 |

LIST OF APPENDICES

| | | No. of Pages |
|------------|-----------------------|--------------|
| APPENDIX 1 | Concrete Test Results | 10 |
| APPENDIX 2 | Dyke Materials | 5 |
| APPENDIX 3 | Instrumentation | 5 |



DESCRIPTION OF CONSTRUCTED WORKS

I. DESCRIPTION OF CONSTRUCTED WORKS

I.I Dyke

I.I.I General

Arrangements were made for the removal of the Canadian National Telecommunications (CNT) pole line from along the Canadian National Railways (CNR) right-of-way prior to the start of the dyke construction. The CNT pole line was replaced with the buried cable installed approximately 3 feet offset from the end of the ties on the land side of the south track.

Because of the type of sub-surface soil conditions (peat, silt and clay) varying amounts of settlement was anticipated along the dyke. In order to monitor the settlement and detect lateral movements during construction, settlement pipes and slope indicator gauges (lateral movement gauges) were installed prior to commencing dyke construction. Total settlements and lateral movements recorded at completion of construction are shown in Appendix 3 - Instrumentation.

I.I.2 Embankment

a. Dyke Embankment

The dyke embankment was constructed from screened sand obtained from the Hillside pit near Victoria. The dyke fill was delivered to the site by barges to the Construction Aggregates dock at Musqueam Drive, Bridgeview, Surrey, B.C.

Typical gradation curves and maximum dry (Proctor) density analyses are shown in Appendix 2 - Dyke Materials.

Filter cloth - "Permaliner-Type I" was used under the filter gravel to prevent loss of dyke material by piping through the dyke fill.

b. Station 0+26 to 12+36 (Bolivar Creek)

In this area unsuitable fill from building demolitions and other sources was stockpiled in the area of the new dyke. The Contractor removed this unsuitable material and wasted it in the adjoining property with the Owner's approval. Stripping of this unsuitable material from the new dyke location revealed deposits of hog fuel. The hog fuel to the west of the dyke embankment was removed to a depth of 3 feet and backfilled with dyke fill. River sand from the structural excavation was used in this area for building the bottom 2 feet of the dyke while the remaining part of the dyke was built with the standard dyke fill material.

Varying amounts of settlement up to 5 feet was expected in this area, therefore, the dyke crest was built up to El 19 feet in the centre and 18 feet at both ends. The preload has been left in

place to allow settlement to occur with time. Settlement to the end of construction has been shown in Appendix 3 - Instrumentation.

c. Station 13+00 to 19+80 and 23+00 to 33+95

The dyke in this area was previously constructed by the C.N.R. for the Inventory and Engineering Branch (formerly Water Investigations Branch). Reconstruction of the dyke in this area was mainly to bring the dyke crest up to grade with dyke fill material plus topping with 6 inches of dyke surfacing.

d. Station 19+80 to 23+00

Stripping of the overburden and structural excavation for the dyke wall in this area revealed unstable ground due to a high water table and seepage out of the bank. To overcome this problem a very clean free draining material (Modified Filter Gravel) was placed on the landside slope of the dyke.

e. Station 33+95 to 48+29

The dyke consists of a low embankment built with typical dyke fill and filter cloth covered with filter gravel.

f. Station 48+29 to 56+09

The dyke in this area was built with typical dyke fill and filter cloth covered with filter gravel. The existing drainage ditch parallelling the dyke in this area was filled with typical dyke fill material after the bottom of the ditch had been cleaned of all soft unsuitable material.

g. Station 56+09 to 57+29

A gap has been left in the construction of the dyke in this location to facilitate construction of a pumphouse at this location at a later date. The dyke and dyke wall will be constructed under Contract No. 3.

h. Station 57+29 to 76+80

The dyke embankment was constructed with typical dyke fill material and filter cloth covered with filter gravel. A new ditch was dug to the south of the old ditch which was backfilled with dyke fill material.

New culverts were installed across 126 A Street, Domtar east crossing and Domtar west crossing.

i. Station 76+80 to 89+10A

The dyke embankment was constructed to typical dyke fill and filter cloth covered with filter gravel. A new ditch was ex

cavated to the south of the existing ditch and the existing ditch was backfilled with dyke fill. A new corrugated asphalt coated metal culvert was installed at Station 78+03 in order to save purchasing additional right-of-way at Station 78+13. The gap of 140' in the concrete wall from Station 76+17 to Station 77+57 for the future 124 St. Floodbox was brought up to Dyke wall elevation with dyke fill material and left in place as preload.

A bagged concrete wall was installed from Station 79+38 to 80+62 to save purchasing additional right-of-way at this location.

j. Station 89+10B to 109+73

The low dyke embankment in this area was constructed of typical dyke fill and filter cloth covered with filter gravel. A drainage ditch was dug on the adjacent property as a replacement for the shallow V ditch which was filled in by the dyke embankment.

1.1.3 Concrete Wall

a. Foundation

Stripping and structural excavation for the concrete wall foundation revealed generally good foundation conditions. The concrete wall is generally founded on compacted dyke fill material.

Settlement plates and slope indicator gauges were installed to monitor the settlement and movement of the dyke embankment. Appendix 3 - Instrumentation shows the results of the monitoring.

No excessive settlement or movement was noticed during construction of the embankment and no excessive movement is expected after construction.

b. Concrete

The Contract called for three different classes of concrete based on strength. A tabulation of the concrete design requirements and concrete mix designs are shown in Table I.

CONCRETE MIX - REQUIREMENTS & DESIGN

| <u>Specifications</u> | <u>Class I</u> | <u>Class II</u> | <u>Class III</u> |
|-------------------------------|----------------|-----------------|------------------|
| Strength | 4000 | 3000 | 2000 |
| Cement (lbs per cu yd) | 395 | 345 | 311 |
| Fly Ash " " " " | 92.4 | 92.4 | 92.4 |
| Concrete Sand (lbs per cu yd) | 1378 | 1446 | 1479 |
| Coarse Aggregate: | | | |
| 10 mm x 5 mm (lbs/cu/yd) | 336 | 336 | 336 |
| 20 mm x 10 mm " " " | 1680 | 1647 | 1647 |
| Slump (inches) | 3"+1" | 3"+1" | 6"+1-1/2" |
| Admixture | Pozzolith | Pozzolith | Pozzolith |
| Air Content (%) | 5±1 | 5±1 | 5±1 |
| Maximum W/C | 0.45 | 0.50 | 0.60 |

c. Joints

The expansion joints in the concrete dyke wall were constructed using water stops, joint filler, joint sealant, bondbreaker and joint primer.

The water stop was a PVC Durajoint type 7C. The joint filler was Rodofoam GR grade, while the joint sealant was Duoflex non-sag polysulphide. The bond breaker between the joint filler and joint sealant was ordinary masking tape. The joint primer used was Polyprime 2.

I.2 Outlet Works 128th Street Floodbox

I.2.1 Foundation

The inlet, outlet and concrete culvert pipe installed in open trench was founded on dense Fraser River sand. A section of the concrete culvert pipe which was jacked under the railroad embankment was jacked through sandy, silty soil. The borehole information and the information obtained from the excavation for the inlet, outlet and trench for the concrete pipe shows that the same dense sand deposit would form the foundation for the culvert pipe jacked under the railroad.

Monitoring of the settlement of the pipe was documented and the results are shown in Appendix 3 - Instrumentation.

1.2.2 Concrete

Class I concrete was used for the construction of the inlet and outlet parts of the structure and has been tabulated in Table I of Sub-section 1.1.3 (b).

The concrete test cylinder results as shown in Appendix I - Concrete Test Results showed that the concrete had adequate strength.

1.2.3 Concrete Pipe

The reinforced concrete pipe used for the outlet structure was manufactured to meet the requirements of the current specification ASTM C76 series. Class V pipe was required within the CNR right-of-way while Class III was installed outside the CNR right-of-way. The concrete pipe had tongue and groove type joints which accepted rubber gaskets and thereby were waterproof.

The rubber gaskets were in accordance with the current specification ASTM C433 series.

1.2.4 Riprap

The riprap material was limited to a maximum size of 18 inch and had 50 to 75 percent of the total number of pieces having an average dimension of 12 inch or more and not more than 10 percent of the total number of pieces had an average dimension of less than 3 inches.

Filter cloth Polyfelt TS400 and Fibretex #300 was used under the riprap to prevent loss of foundation material by piping through the voids in the riprap.

1.2.5 Inlet Channel

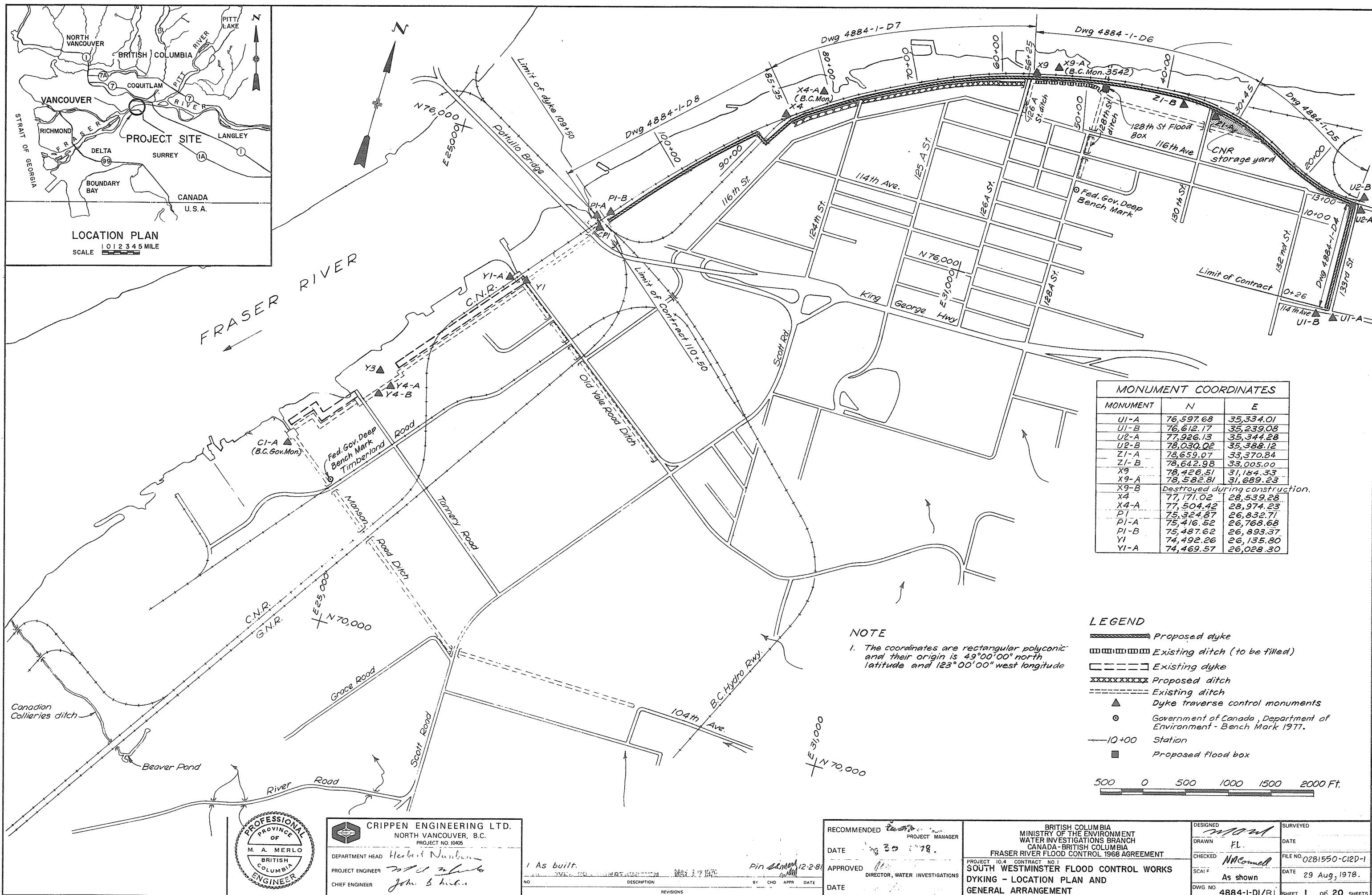
The inlet channel was constructed to connect with the existing 128th Street ditch. Should there be reconstruction of the ditch in this area at a future date the ditch should be constructed to approach the flood box in a straight line.

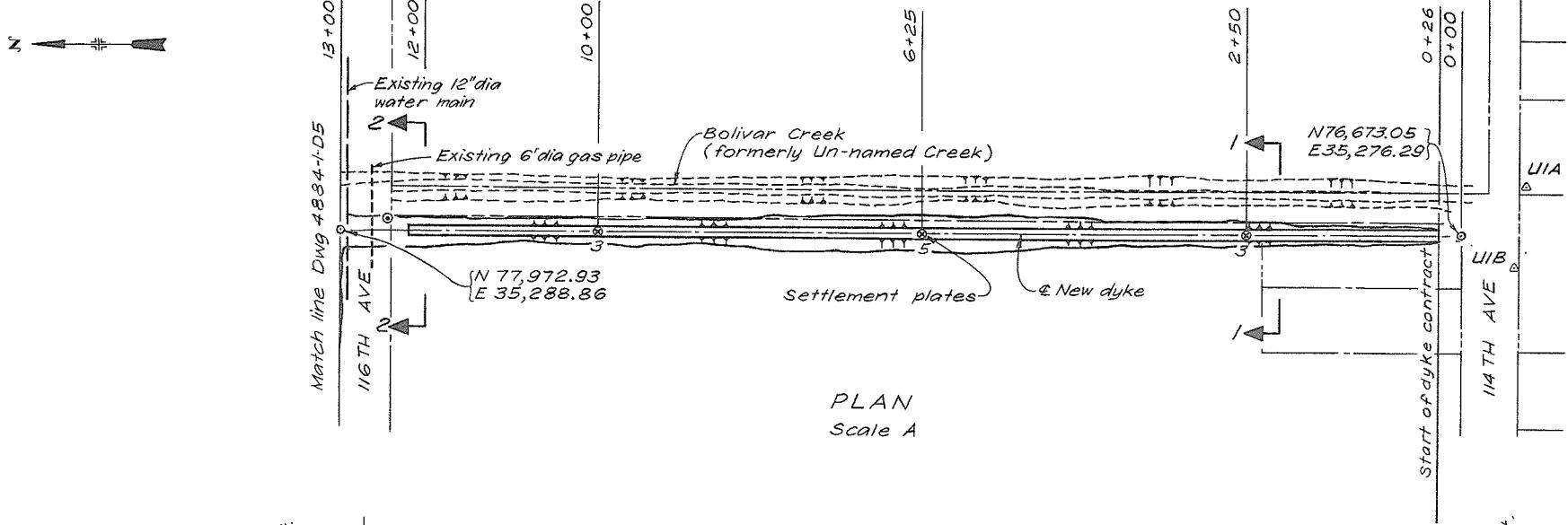
2. AS-CONSTRUCTED DRAWINGS

2.1 As-Constructed Drawing List

| Drawing No. | Title | Provincial 105 mm Negative Number |
|---------------|--|--|
| 4884-I-D1/R1 | Dyking - Location Plan and General Arrangement | 105764 |
| 4884-I-D4/R2 | Dyke - Station 0+90.00 to 12+99.94 Plan, Profile and Sections | 105765 |
| 4884-I-D5/R3 | Dyke - Station 12+99.94 to 30+45 Plan, Profile and Sections | 105766 |
| 4884-I-D6/R2 | Dyke - Stations 30+45 to 56+25 Plan, Profile and Sections | 105767 |
| 4884-I-D7/R4 | Dyke - Stations 56+25 to 85+35 Plan, Profile and Sections | 105768 |
| 4884-I-D8/R4 | Dyke - Stations 85+35 to 110+50 Plan, Profile and Sections | 105769 |
| 4884-I-D3/R2 | Settlement Plate & Piezometer Details | 105770 |
| 4884-I-D16/R1 | Mill & Timber Spurline Dyke Retaining Walls - Plan & Section | 105771 |
| 4884-I-D15/R2 | Wesco Spurline - Dyke Retaining Walls Plan, Sections and Details | 105772 |
| 4884-I-D2/R3 | Miscellaneous Concrete Details | 105773 |
| 4884-I-D18/R2 | Road Crossings and Stoplog Wall Details | 105774 |
| 4884-I-D9/R3 | Dyke Wall - Concrete Outline and Reinforcement | 105775 |
| 4884-I-D10/R3 | 128th Street Floodbox - General Arrangement & Work Areas | 105776 |
| 4884-I-D11/R2 | 128th Street Floodbox - Excavation & Backfill - Sheet 1 of 2 | 105777 |
| 4884-I-D12/R2 | 128th Street Floodbox - Excavation & Backfill - Sheet 2 of 2 | 105778 |

| Drawing No. | Title | Provincial 105 mm Negative Number |
|---------------|---|--|
| 4884-I-D13/R2 | I28th Street Floodbox - Concrete Outline | 105779 |
| 4884-I-D14/R2 | I28th Street Floodbox - Reinforcement | 105780 |
| 4884-I-D17/R1 | I28th Street Floodbox - Flood gate | 105781 |
| 4884-I-D19/R1 | I28th Street Floodbox - Miscellaneous Metalwork | 105782 |
| 4884-I-D20/R1 | I28th Street Floodbox Trashracks | 105783 |

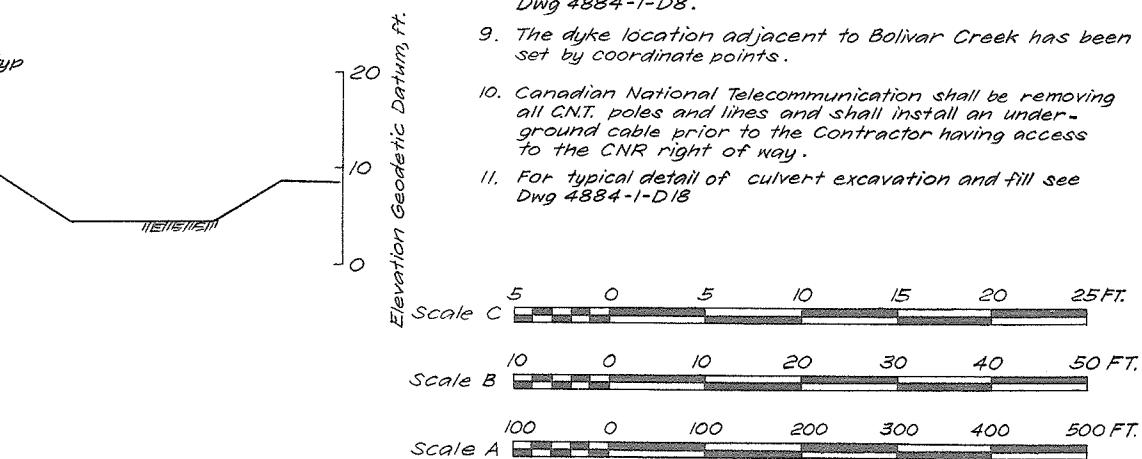
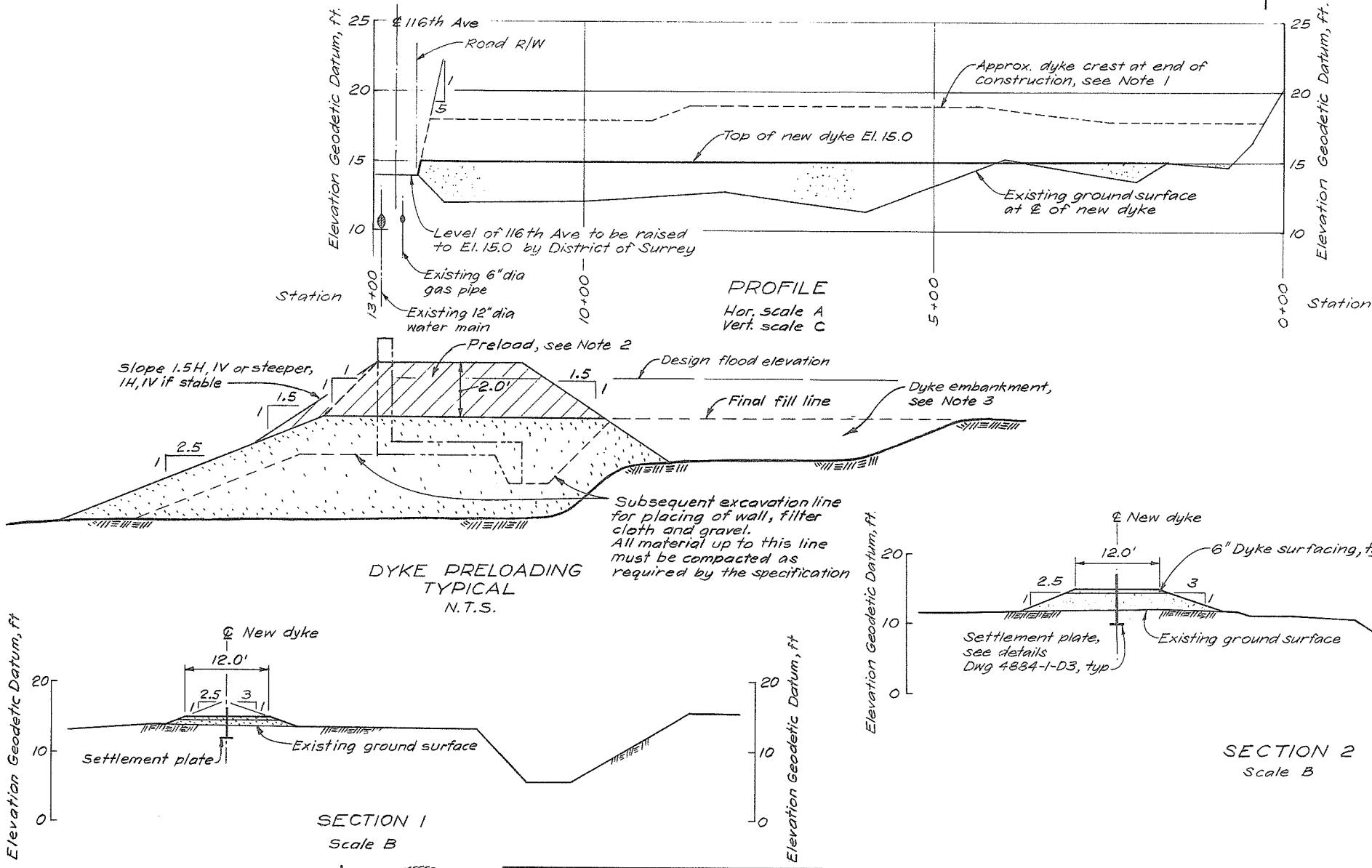




| | |
|-------|--|
| € | Centreline |
| CN(R) | Canadian National(Railroad) |
| R/W | Right of way |
| — | Railroad |
| ■ | Asphaltic concrete pavement |
| W.O. | Works out (site adjustment) |
| CN-ML | CN-Main Line |
| CN-SL | CN-Switching Lead |
| CN-MW | CN-Maintenance walk |
| △UIA | Monument |
| ○ | Lateral movement gauge location (slope indicator casing on plan) |
| (E) | Lateral movement gauge in section (slope indicator casing in section) |
| — | Bottom El. of casing |
| — | Canadian National Telecommunication pole |
| 5 | Settlement plate and piezometer on plan, nominal depth of piezometer below existing ground level in ft |
| — | Settlement plate and piezometer in section |
| — | Base of rail, south main line |
| — | New ditch invert (Land side) |

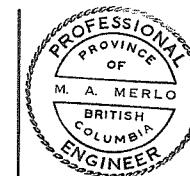
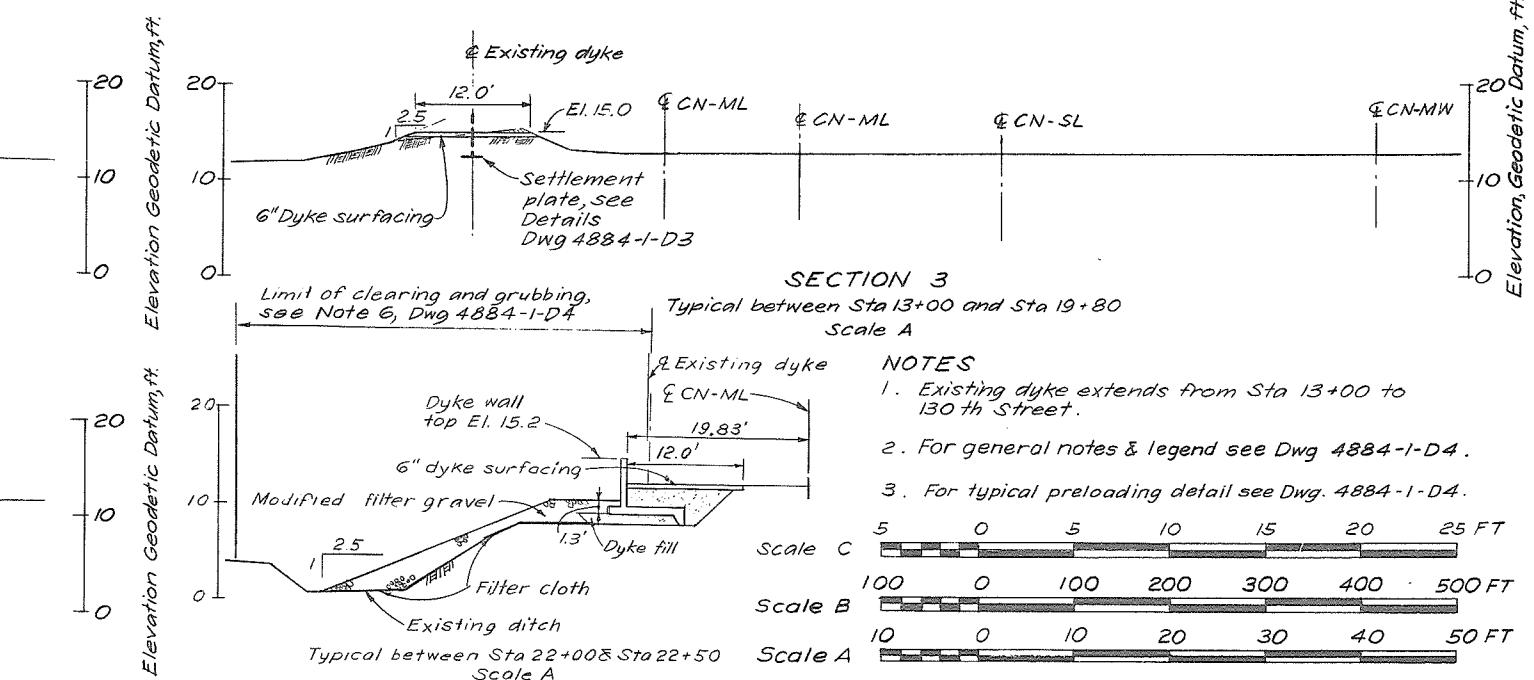
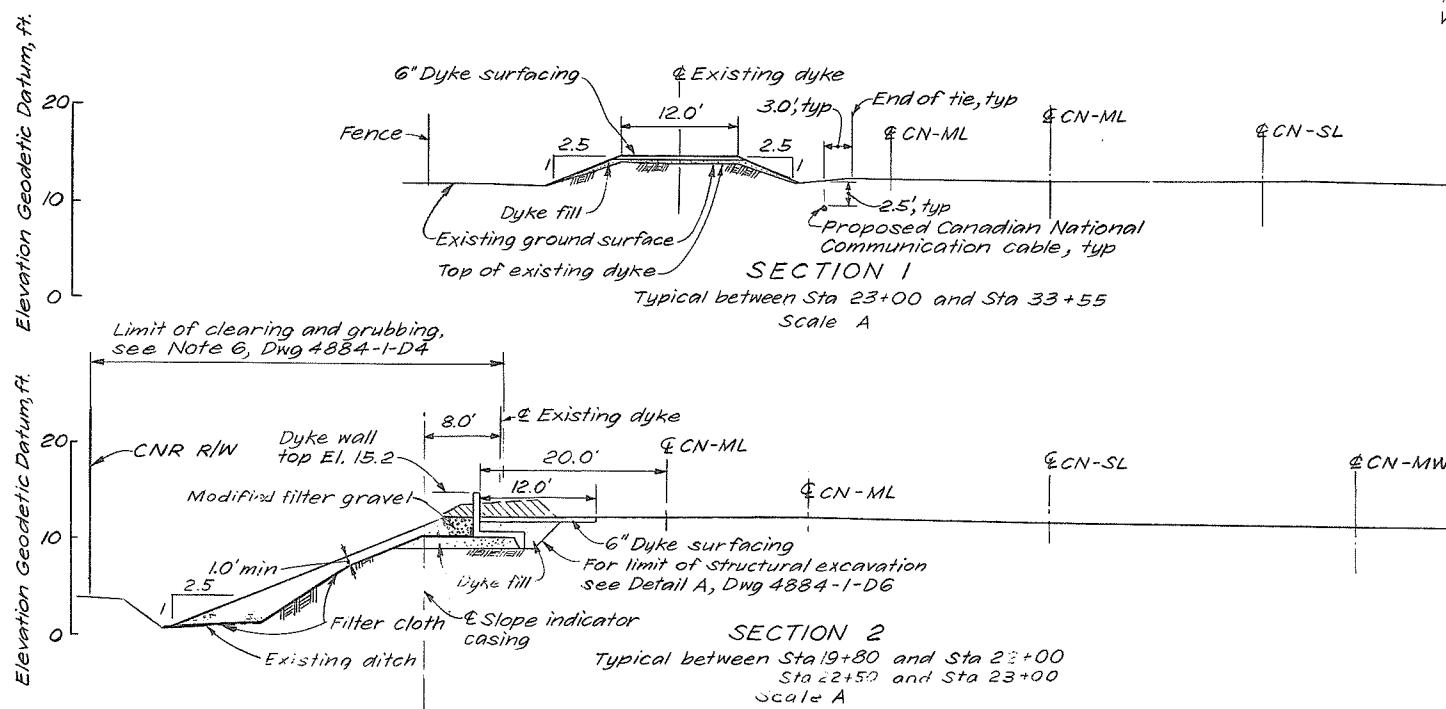
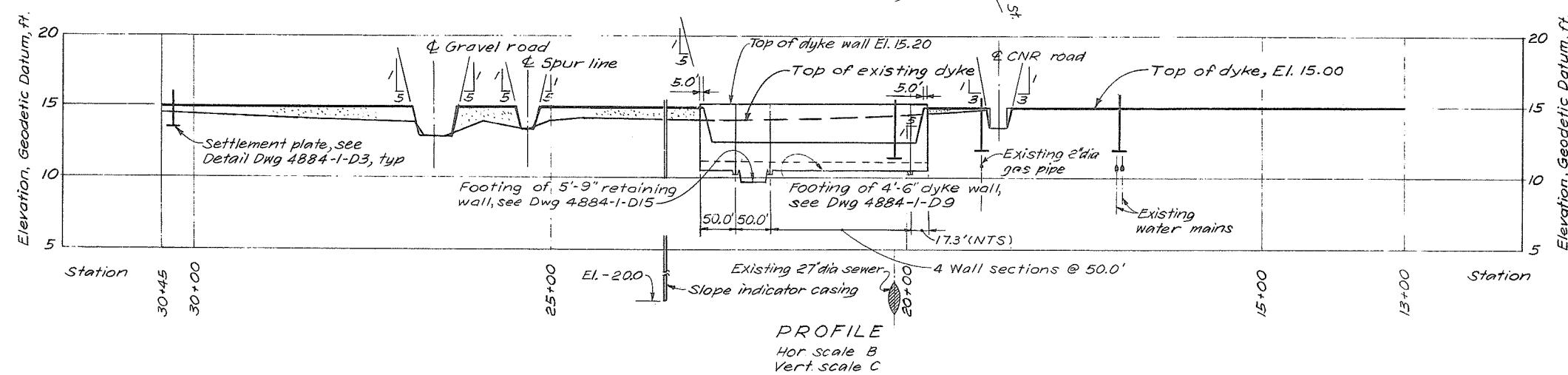
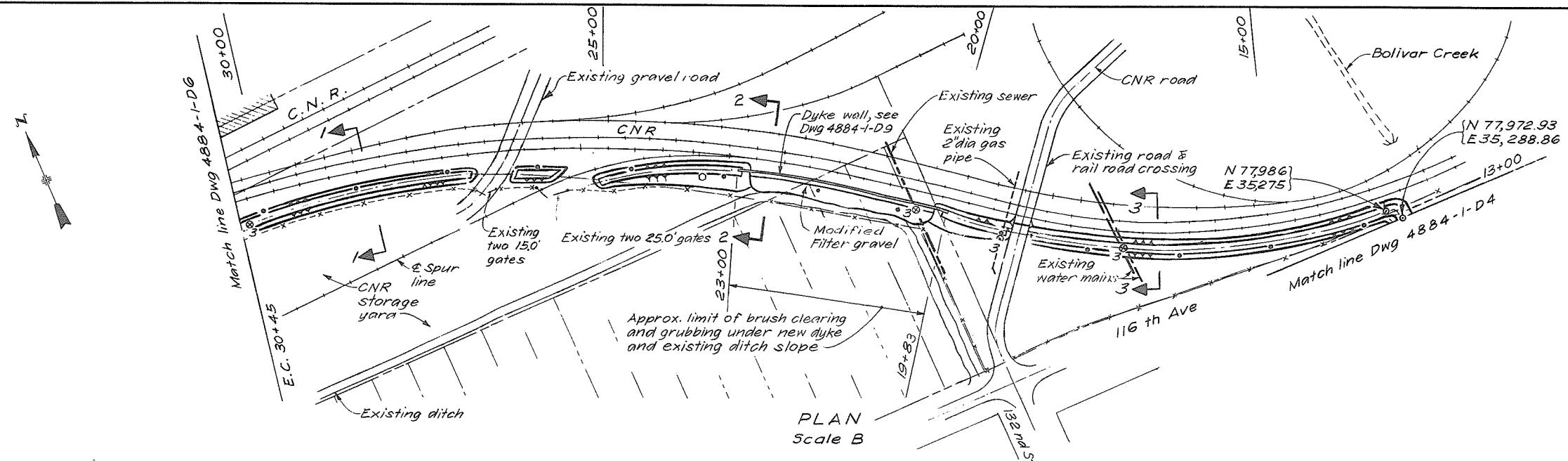
NOTES

- The crest elevation at end of construction will depend to some extent on the rate of construction. A settlement of about 5 ft is expected at Sta. 6+00. Final crest elevation is 15.0. Applies to dyke from Sta. 0+26 to 12+40.
- The 2.0 ft of preload shall be placed at all wall locations except where existing ground elevation at the dyke wall location is at or above the proposed surcharge elevation. The duration of preload shall be determined in field by the Engineer.
- The preload shall be constructed with dyke fill and at the Contractor's option the preload material may be incorporated as part of the dyke embankment.
- Unless otherwise indicated, all dyke fill and ditch backfill, except 6" thick dyke surface course, toe drain and gravel filter, to consist of dredged Fraser River fine sand or acceptable fill in accordance with the specifications.
- Dyke surface course to be 6" thick layer of well compacted gravel-sand mixture as specified. Toe drain and gravel filter to consist of select granular material meeting filter requirements or acceptable fill in accordance with the specifications.
- Limits of clearing and grubbing as shown on sections are typical for areas as indicated on plans and are maximum limits. Actual limits will be determined in field by the Engineer.
- Dyke and dyke wall alignment have been set by coordinate points and by offsets measured at right angles to and from centreline of south railroad track adjacent to dyke. Exact layout of dyke shall be determined by the Contractor and shall comply with the minimum clearances specified.
- For filter cloth placement, excavation and structural fill payment limit adjacent to dyke wall see Detail Y and Z Dwg 4884-1-D8.
- The dyke location adjacent to Bolivar Creek has been set by coordinate points.
- Canadian National Telecommunication shall be removing all CNT poles and lines and shall install an underground cable prior to the Contractor having access to the CNR right of way.
- For typical detail of culvert excavation and fill see Dwg 4884-1-D18



| | | | | |
|-------------|------------------|---|----------|-------------------------|
| RECOMMENDED | PROJECT MANAGER | BRITISH COLUMBIA | DESIGNED | SURVEYED |
| DATE | MAY 17 1978 | MINISTRY OF THE ENVIRONMENT WATER INVESTIGATIONS BRANCH CANADA-BRITISH COLUMBIA | DRAWN | DATE |
| APPROVED | Aug 30 1978 | FRASER RIVER FLOOD CONTROL 1968 AGREEMENT | FL | |
| NO. | BY CHD APPR DATE | PROJECT 10.4 CONTRACT NO. 1 SOUTH WESTMINSTER FLOOD CONTROL WORKS DYKE STATIONS 0+90.00 TO 12+99.94 PLAN, PROFILE AND SECTIONS | CHECKED | FILE NO. 0281550-C12D-1 |

| | | | | |
|---|--|---|-----------------|-----------------|
| PROFESSIONAL OF BRITISH COLUMBIA ENGINEER | CRIPPEN ENGINEERING LTD. NORTH VANCOUVER, B.C. PROJECT NO. 10405 | APPROVED FOR CONSTRUCTION MAY 17 1978 | RECOMMENDED | RECOMMENDED |
| M. A. MERLO | DEPARTMENT HEAD Herbert Müssbaum | 1. General Note 11 added. General Note 8 revised Minor revisions as shown. | PROJECT MANAGER | PROJECT MANAGER |
| CHIEF ENGINEER | PROJECT ENGINEER | 2. As built. | DATE | DATE |
| | | APPROVED | FL | FL |
| | | for DIRECTOR, WATER INVESTIGATIONS | 20-279 | 20-279 |
| | | DATE | Aug 27 1978 | Aug 27 1978 |
| | | REVISIONS | | |



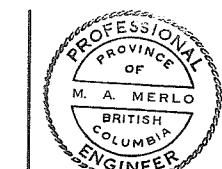
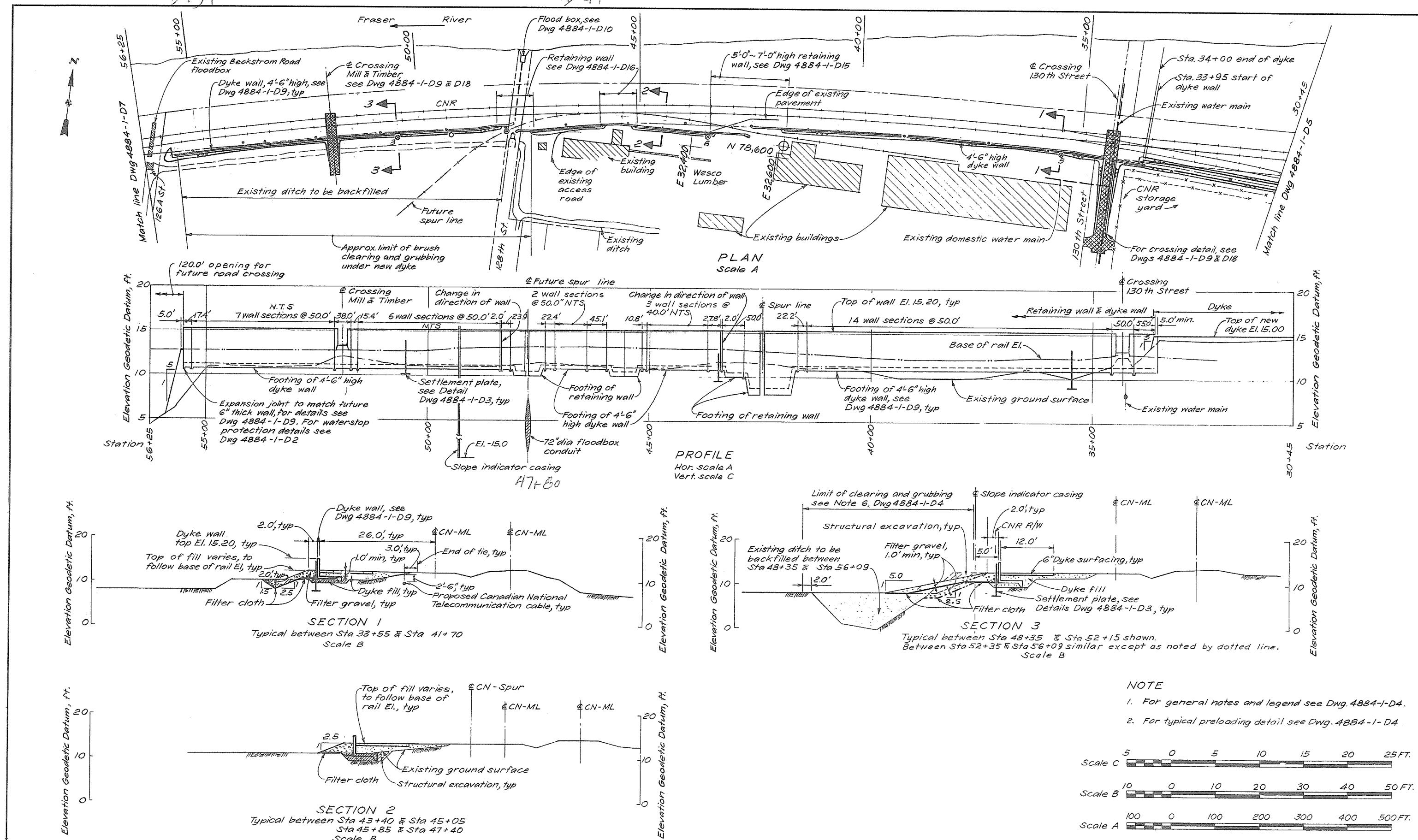
CRIPPEN ENGINEERING LTD.
NORTH VANCOUVER, B.C.
PROJECT NO. 10405
DEPARTMENT HEAD: *Hurt*
PROJECT ENGINEER: *P. J. G.*
CHIEF ENGINEER: *J. H. S.*

3. As built.
2 Structural excavation note added to Section 2
and minor revision 78 shown.
1. Slope indicator casing bottom tip revised
NO. *100* FL *100* MM *100* IN *100* APPR *100* DATE
DESCRIPTION BY CHD APPROVED DATE
REVISIONS

RECOMMENDED *As built*
PIN *12-281* PROJECT MANAGER
DATE *Aug 31 1978*
APPROVED *100* DIRECTOR, WATER INVESTIGATION
FL *100* MM *100* IN *100* APPR *100* DATE
DESCRIPTION BY CHD APPROVED DATE
REVISIONS

BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT
PROJECT 10.4 CONTRACT NO. 1
SOUTH WESTMINSTER FLOOD CONTROL WORKS
DYKE STATIONS: 12+99.94 TO 30+45
PLAN, PROFILE & SECTIONS

DESIGNED *M. Merlo* SURVEYED
DRAWN *P.S.K., H.N. Chee* DATE
CHECKED *R. C.* FILE NO. *0281550-C12D-1*
SCALE *As shown* DATE *29 Aug 1978*
DWG. NO. *4884-1-D5/R* SHEET *3* OF *20* SHEETS



2 As built
APPROVED FOR CONSTRUCTION MAY 17 19

1. Detail A added. Structural excavation details.
Notes added at Sta 55+50 & minor revision.

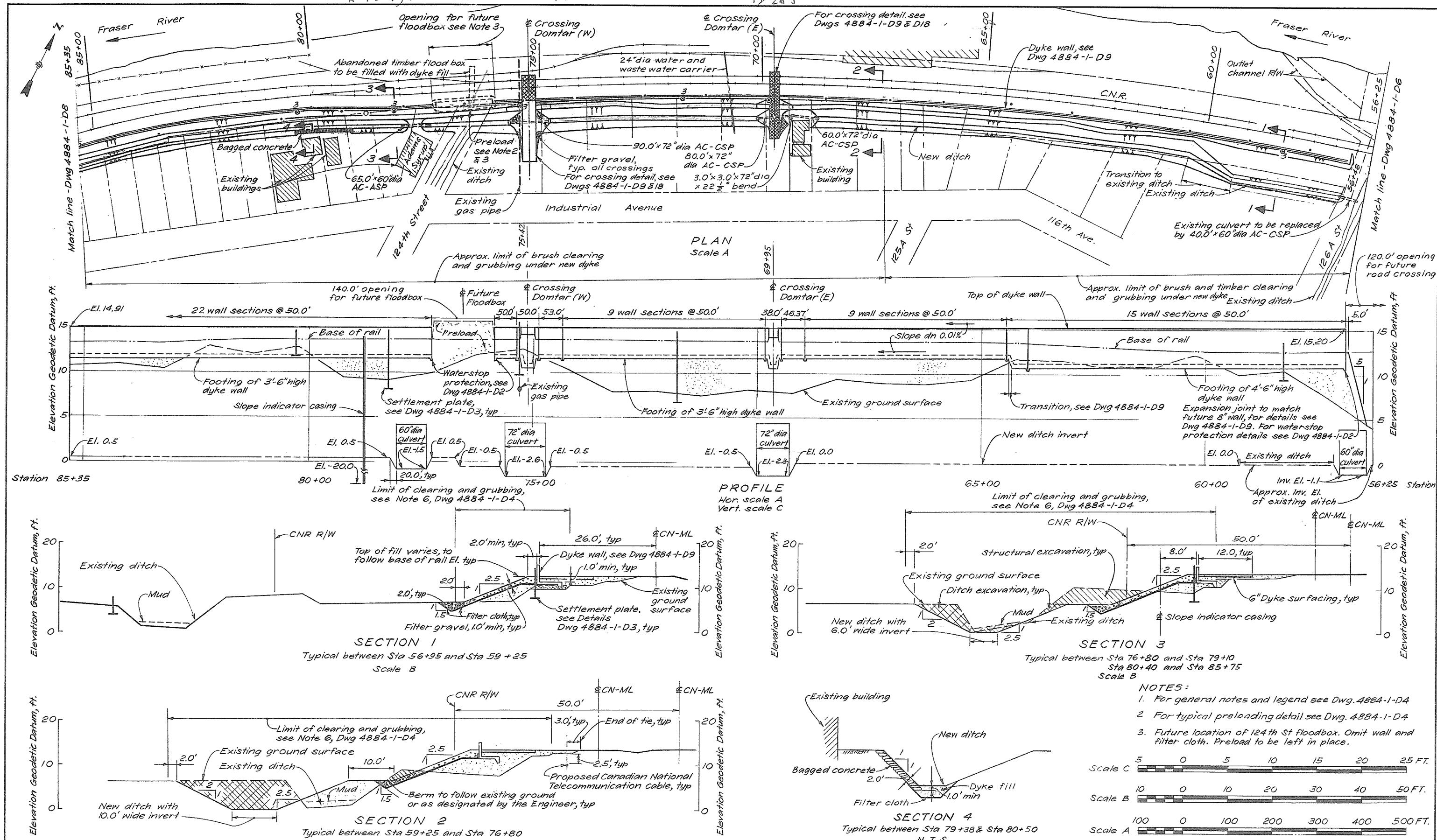
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|-----------------|----------|--|
| RECOMMENDED | | <i>Lee Cook</i> |
| PROJECT MANAGER | | |
| 12-2-81 | DATE | Aug 30 1978 |
| 20-2-79 | APPROVED | <i>Bob Miller</i> for DIRECTOR, WATER INVESTIGATION |
| R DATE | DATE | E 8/28/82 |

BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT

PROJECT 10.4 CONTRACT I
SOUTH WESTMINSTER FLOOD CONTROL WORKS
DYKE STATIONS : 30 +45 TO 56 + 25
PLAN, PROFILE & SECTIONS

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| DESIGNED | <i>M.W.M.</i> | SURVEYED |
| DRAWN | FL, P.S.K. | DATE |
| CHECKED | | FILE NO. |
| SCALE | As shown | DATE |
| DWG. NO. | 4884-1-D6K2 | Sheet 4 of 20, CHARTED |



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|---|---|
|  | CRIPPEN ENGINEERING LTD. NORTH VANCOUVER, B.C. PROJECT NO. 10405 |
| DEPARTMENT HEAD | H. J. MacLean |
| PROJECT ENGINEER | T. M. Smith |
| CHIEF ENGINEER | J. L. Johnson |

4. As built.

3. Road crossing details revised to conform to Dug
section numbers revised. Ditch excavation sys-
at Section 3 revised.

APPROVED FOR CONSTRUCTION MAY 17 1978

2. Ditch invert El. revised. Structural & ditch ex-
defined and minor revisions as shown & n.

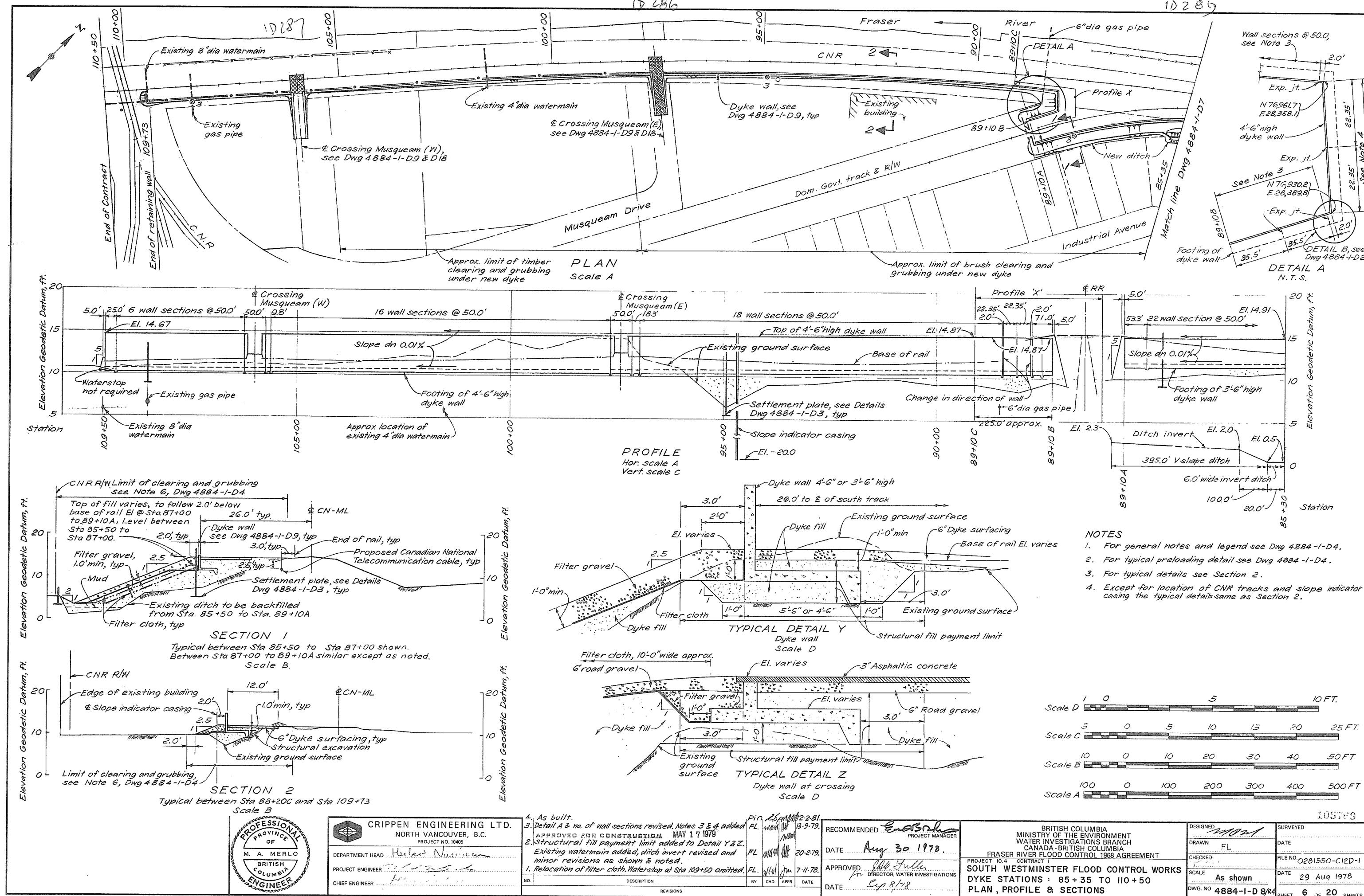
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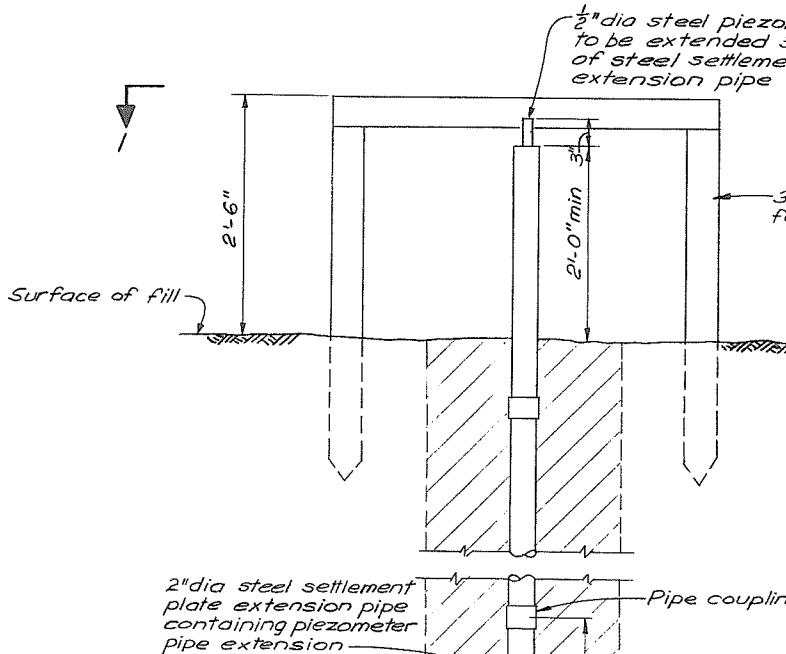
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| 8. Wall bol vation ed. | Pin | SL | MM | 6-2-81 | RECOMMENDED PROJECT MANA DATE Aug 30 1978 |
| | FL | MM | MM | 19-9-75 | |
| | FL | MM | MM | 20-2-79 | |
| | FL | MM | JH | 7-11-78 | |
| BY | CHD | APPR | DATE | APPROVED <i>J.W. Miller</i> for DIRECTOR, WATER INVESTIGATE | |

**BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT**

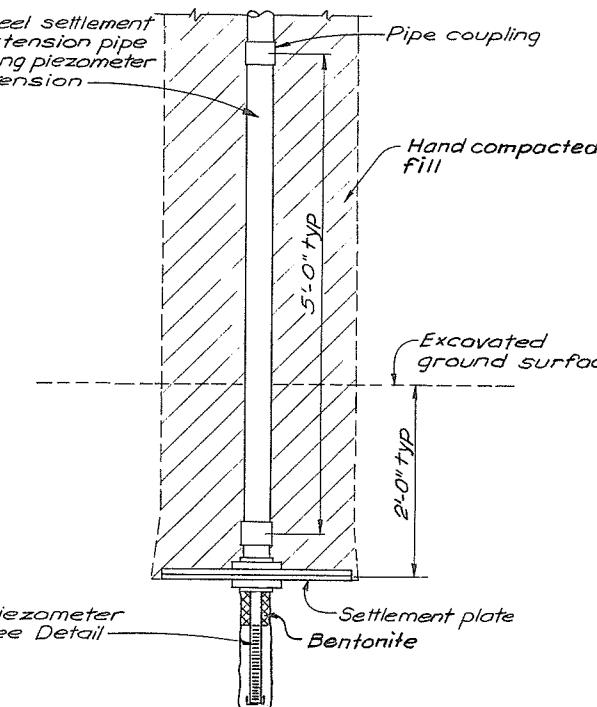
PROJECT ID. 4 CONTRACT I
**SOUTH WESTMINSTER FLOOD CONTROL WORKS
DYKE STATIONS: 56 + 25 TO 85 + 35
PLAN, PROFILE AND SECTIONS**

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| DESIGNED <i>M. W. M.</i> | SURVEYED |
| DRAWN FL | DATE |
| CHECKED | FILE NO. 0281550-C12D-1 |
| SCALE As shown | DATE 29 Aug 1978 |
| DWG. NO. 4884-1-D7/R4 | SHEET 5 OF 20 SHEETS |

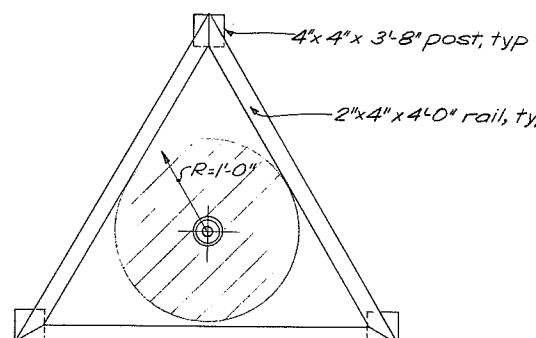




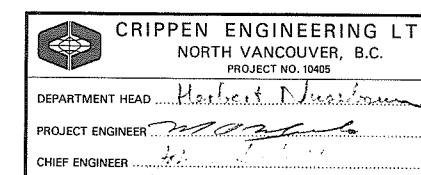
INSTALLATION OUTSIDE FI



INSTALLATION AT DYKE

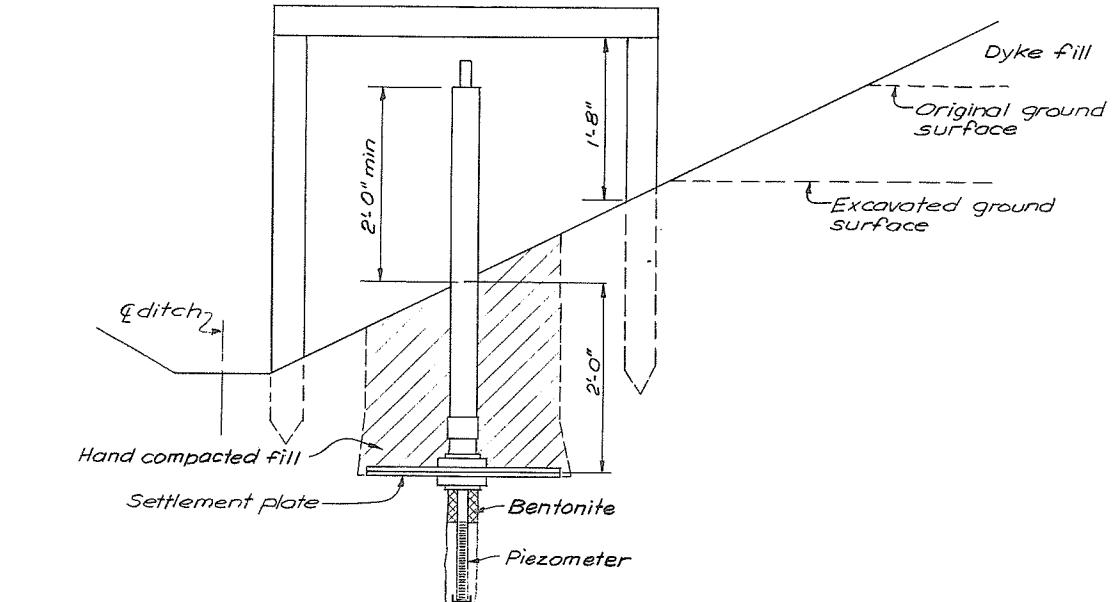


PLAN

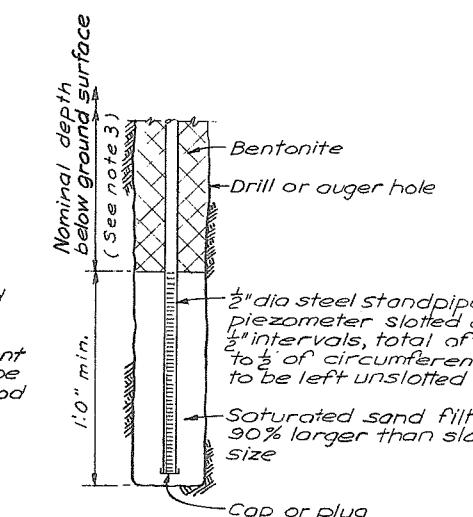


2. As built
APPROVED FOR CONSTRUCTION MAY 17 1979
1. Piezometer detail dimension added

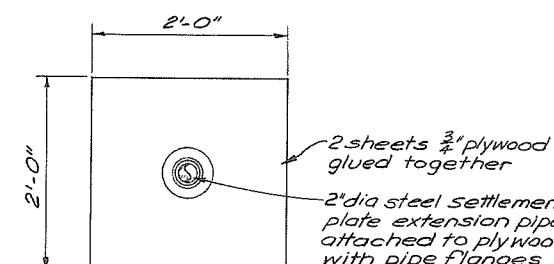
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|--|-------------|--------------------------------------|
| 1. Piezometer detail dimension added | | |
| NO. | DESCRIPTION | BY CHD APPR |
| | | |



INSTALLATION NEAR DITCH



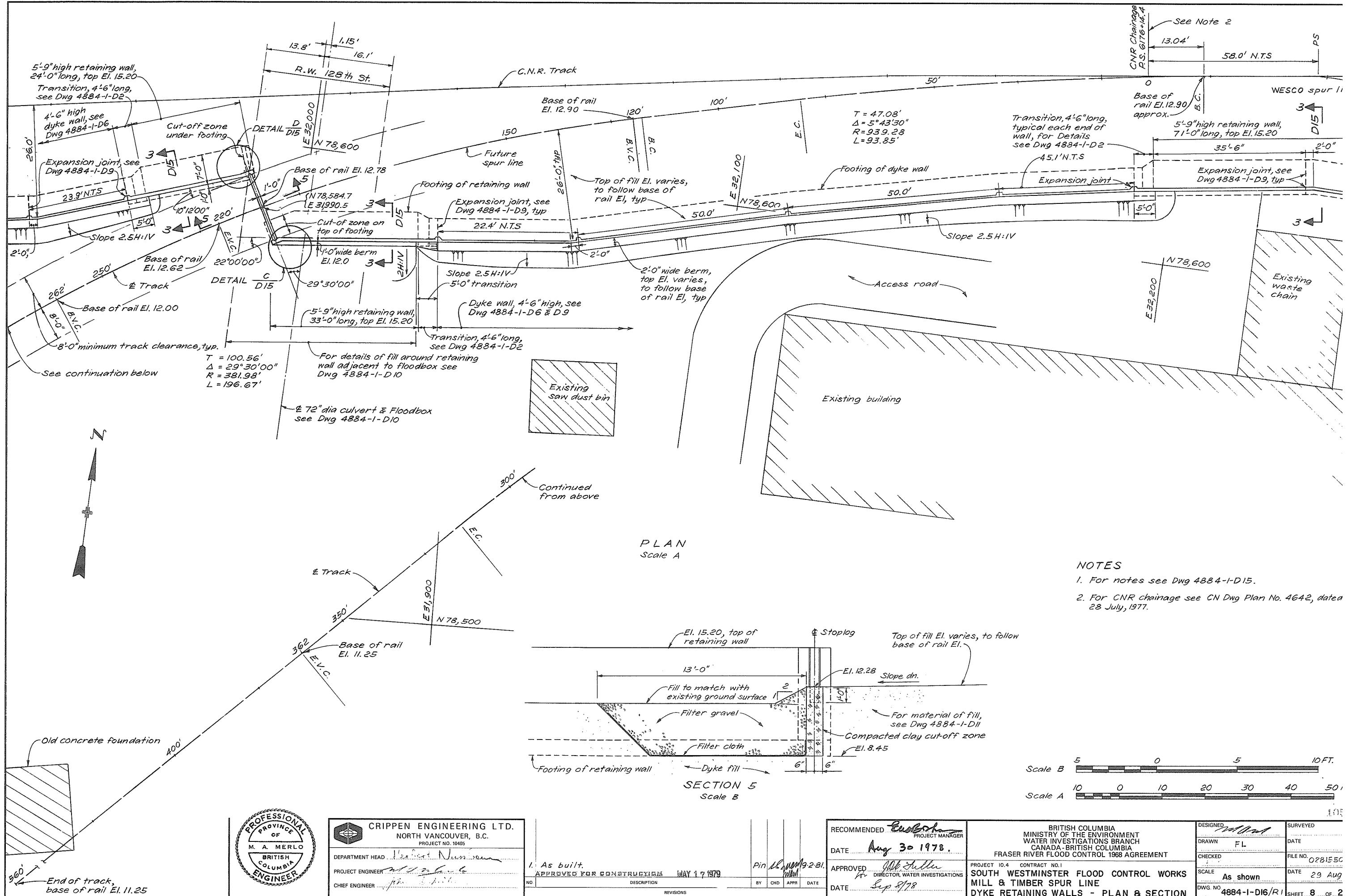
PLAN 2
TYPICAL SETTLEMENT PLAT

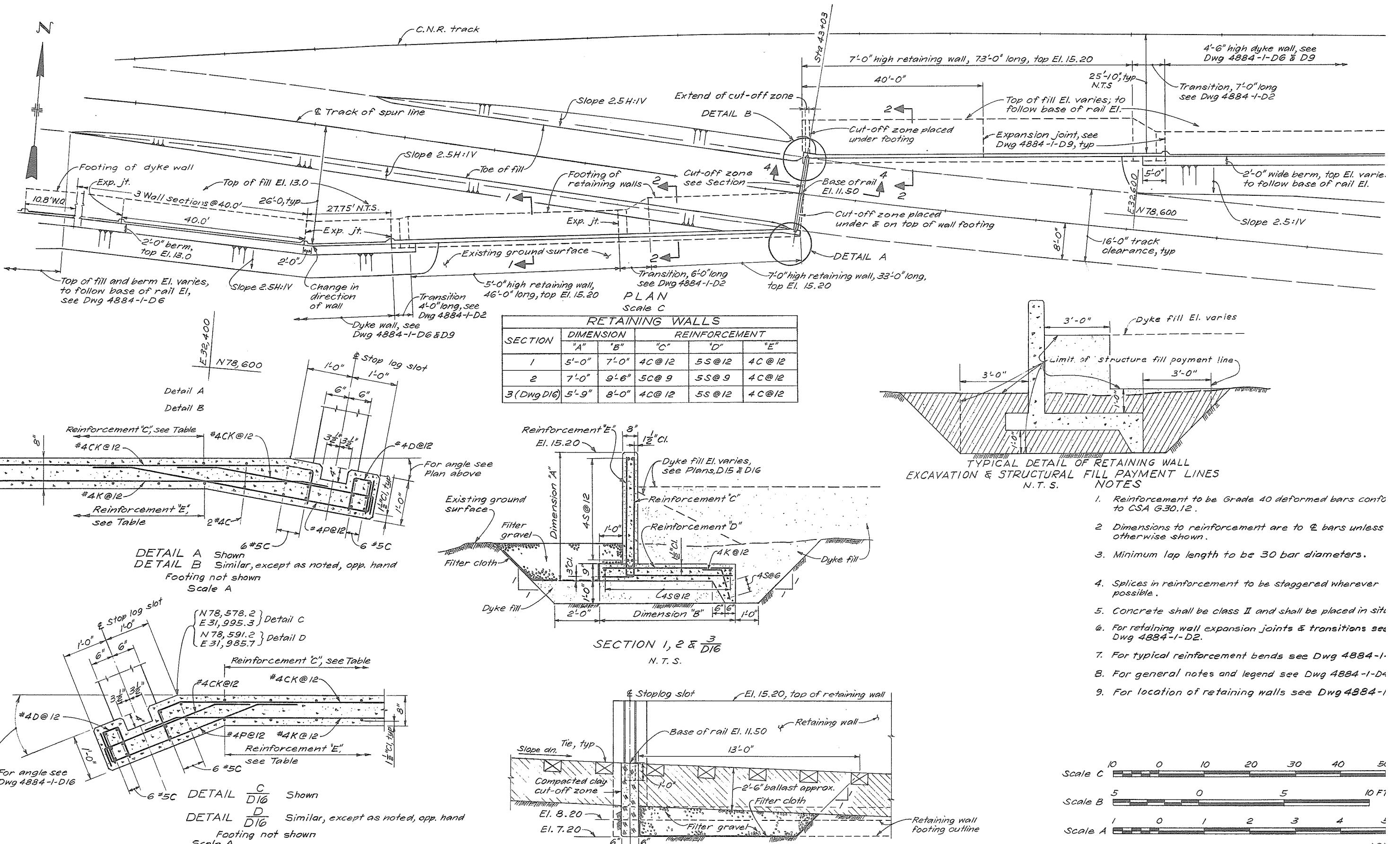


PIEZOMETRE DETAIL

- NOTES**

 1. Surface settlement plate to be installed at approx 2'-0" depth below ground level.
 2. Protective fence to be installed prior to any fill placement & raised as fill surface rises.
 3. Installation depths and locations of piezometer - settlement plate units are shown on Dwg 4884-1-L D5, D6, D7 and D8.
 4. All extension pipes to be clearly identified.





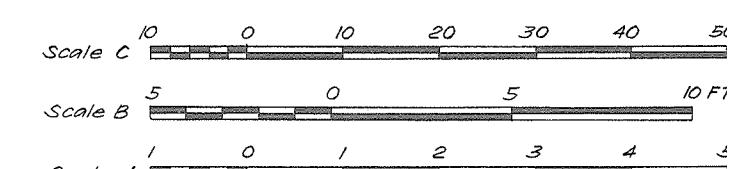
CRIPPEN ENGINEERING LTD.
NORTH VANCOUVER, B.C.
PROJECT NO. 10405
DEPARTMENT HEAD
PROJECT ENGINEER
CHIEF ENGINEER

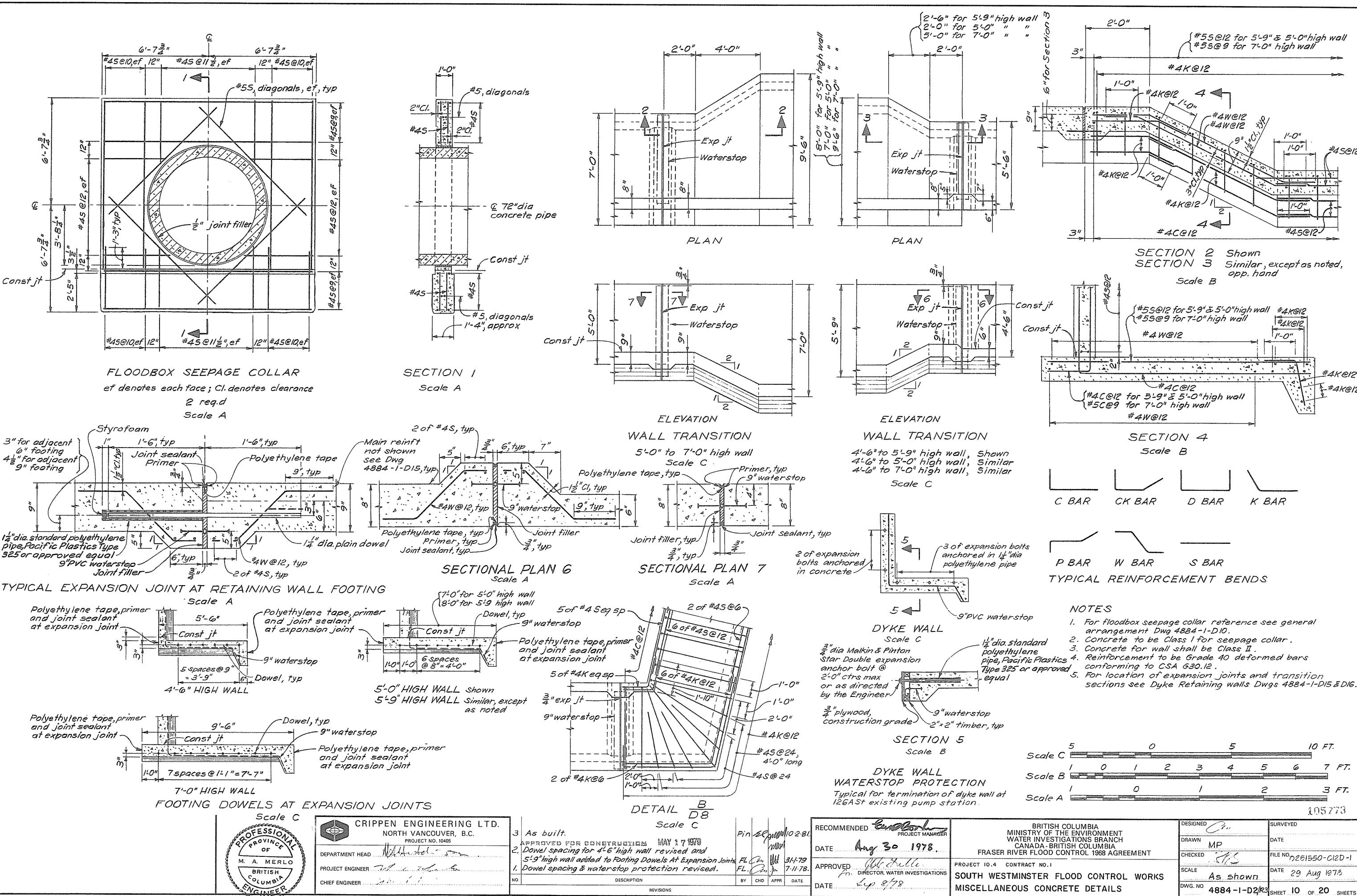
| | |
|-----------------------------------|-------------|
| RECOMMENDED | Aug 30 1978 |
| DATE | 12-28-78 |
| APPROVED | Aug 30 1978 |
| BY DIRECTOR, WATER INVESTIGATIONS | APPR DATE |
| NO. | DESCRIPTION |
| | BY CHD |
| | REVISIONS |

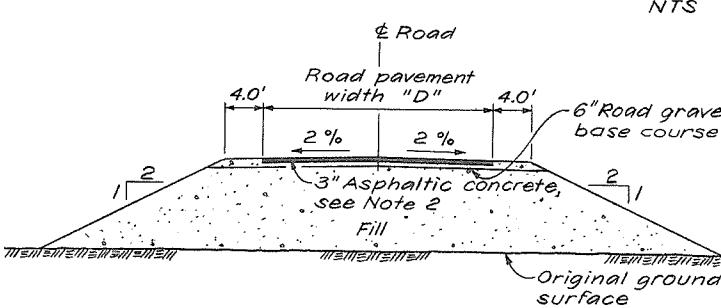
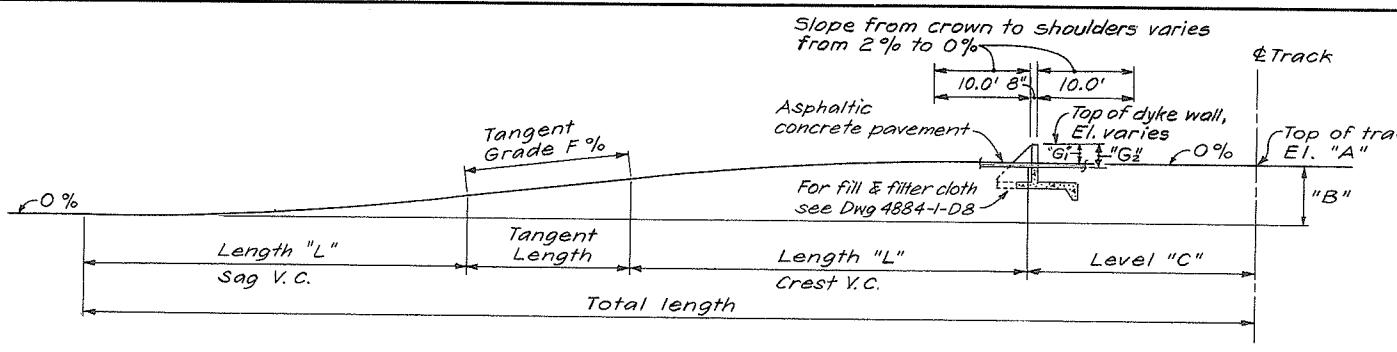
BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT
PROJECT 10.4 CONTRACT NO. 1
SOUTH WESTMINSTER FLOOD CONTROL WORKS
WESCO SPUR LINE - DYKE RETAINING WALLS
PLAN, SECTIONS & DETAILS

DESIGNED
DRAWN
CHECKED
SCALE
DWG. NO.

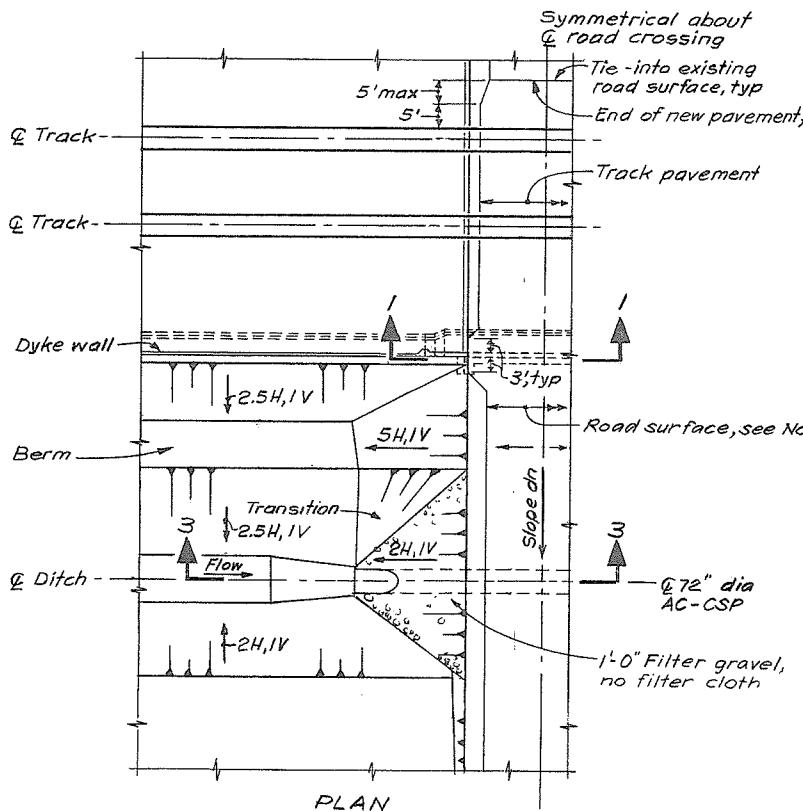
SURVEYED
DATE
FILE NO.
DATE
As shown
29 Aug
0281550
9 OF 21



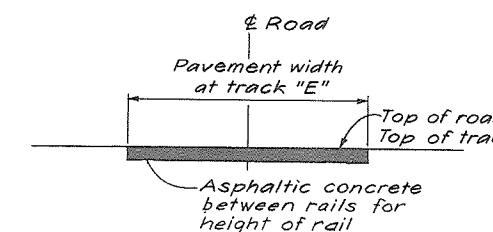




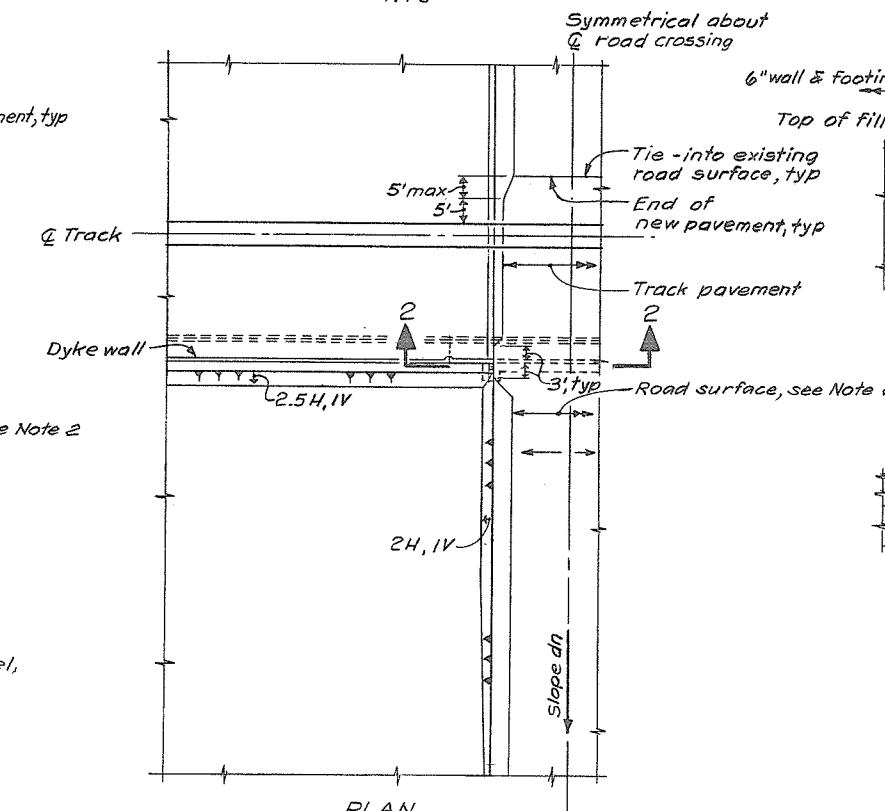
TYPICAL SECTION - ROAD CROSSING
Scale A



DOMTAR (W) ROAD CROSSING Shown
DOMTAR (E) ROAD CROSSING Similar
Scale C

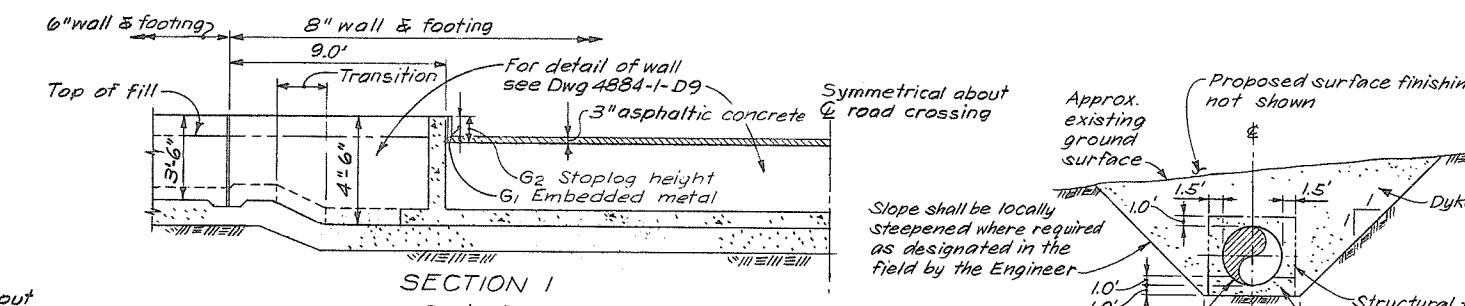


TYPICAL RAILWAY PROFILE
ROAD CROSSING
NTS

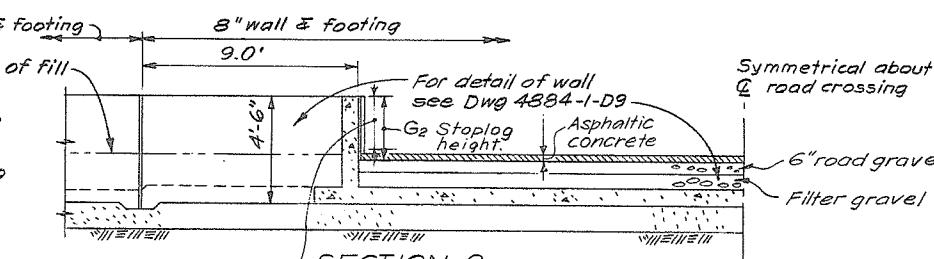


MUSQUEAM (W) ROAD CROSSING Shown
MUSQUEAM (E) " " Similar
130th ST. " " Similar
MILL & TIMBER " " Similar
Scale C

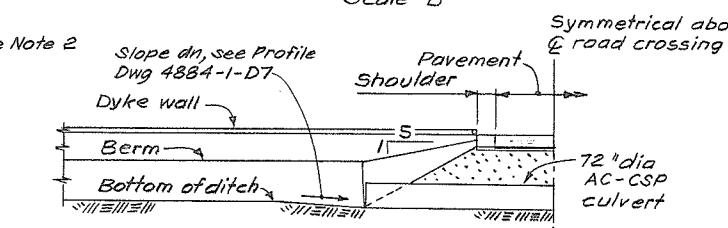
| ROAD CROSSING - TABLE OF VARIABLES | | | | | | | | | | | | |
|------------------------------------|--------------|----------------------|----------------------|---------------|-----------|------------|----------------|-------------------|--------------|-------------------------|-------------------|----------------------|
| Road Crossing | Dyke Station | Top of Dyke wall El. | Top of Track El. "A" | El. Diff. "B" | Level "C" | Length "L" | Tangent Length | Tangent Grade F % | Total Length | Road Pavement Width "D" | Pave't. Width "E" | Manuf. Embd. Mtl. G1 |
| 130 th St. | 34+75 | 15.20 | 12.39 | 4.66' | 50' | 100' | 0 | 4.7 | 250' | 24' | 28' | 2.35' |
| Mill & Timber | 52+25 | 15.20 | 13.13 | 5.68' | 27' | 49' | 17' | 8.6 | 142' | 12' | 16' | 2.15' |
| Domtar (E) | 69+95 | 15.07 | 14.03 | 6.56' | 27' | 40' | 17' | 11.5 | 124' | 12' | 16' | 1.32' |
| Domtar (W) | 75+42 | 15.01 | 14.02 | 5.40' | 35' | 60' | 10' | 7.7 | 165' | 24' | 28' | 1.06' |
| Musqueam(E) | 97+63 | 14.79 | 13.00 | 1.46' | 55' | 35' | 5' | 3.7 | 130' | 24' | 28' | 1.88' |
| Musqueam(W) | 106+23 | 14.70 | 12.09 | 1.20' | 70' | 40' | 0 | 3.0 | 150' | 24' | 28' | 2.65' |



SECTION 1
Scale B



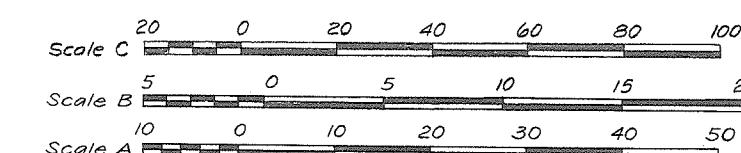
SECTION 2
Scale B



SECTION 3
Scale C

NOTES

1. For general notes see Dwg 4884-1-D4.
2. The Contractor shall pave the roads as shown on crossing plans for each location.



105



CRIPPEN ENGINEERING LTD.
NORTH VANCOUVER, B.C.
PROJECT NO. 1040
DEPARTMENT HEAD: M. A. Merlo
PROJECT ENGINEER: M. A. Merlo
CHIEF ENGINEER: M. A. Merlo

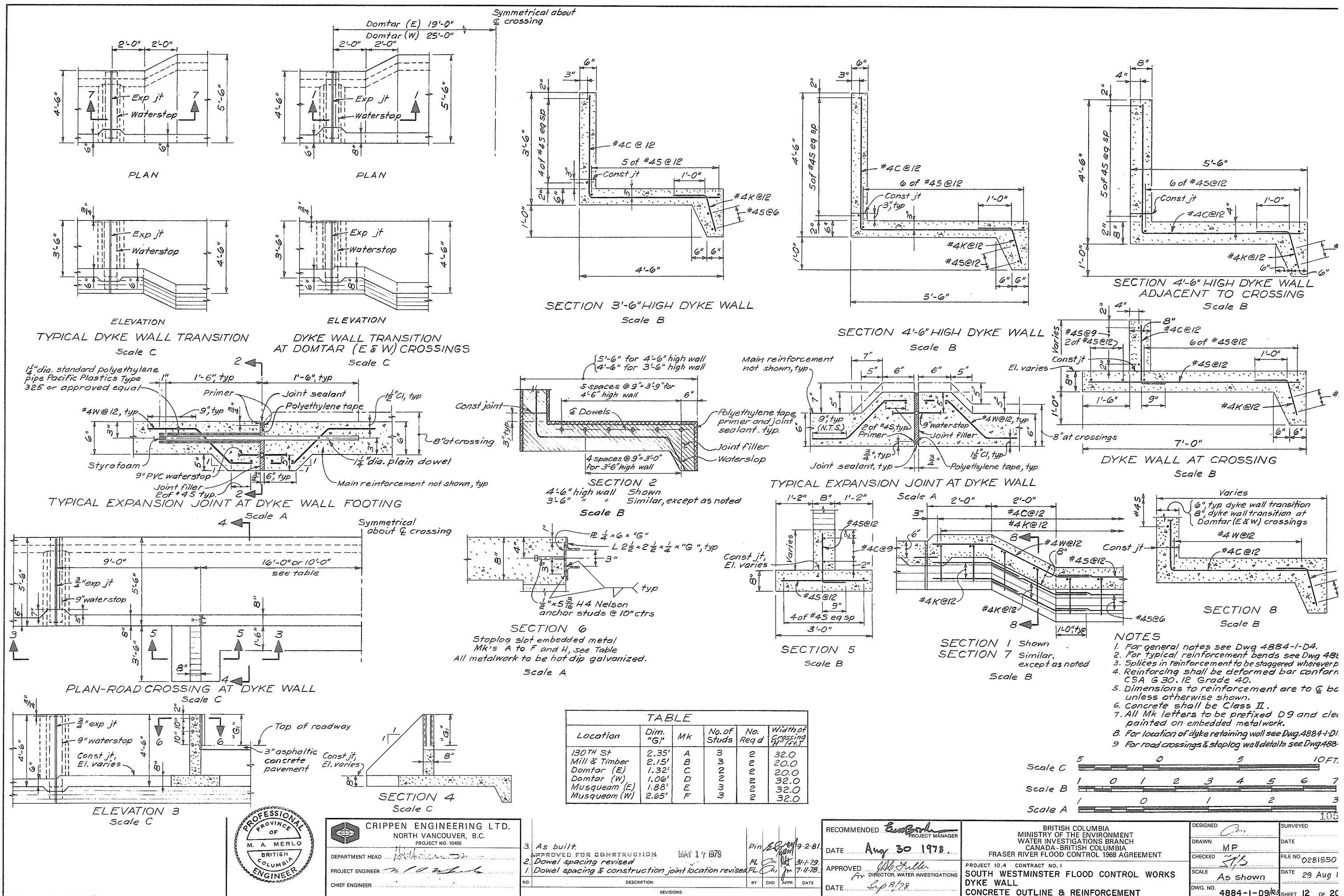
2. As built.
APPROVED FOR CONSTRUCTION MAY 19 1978
1. Typical detail of culvert excavation & fill added.
Minor revisions as shown

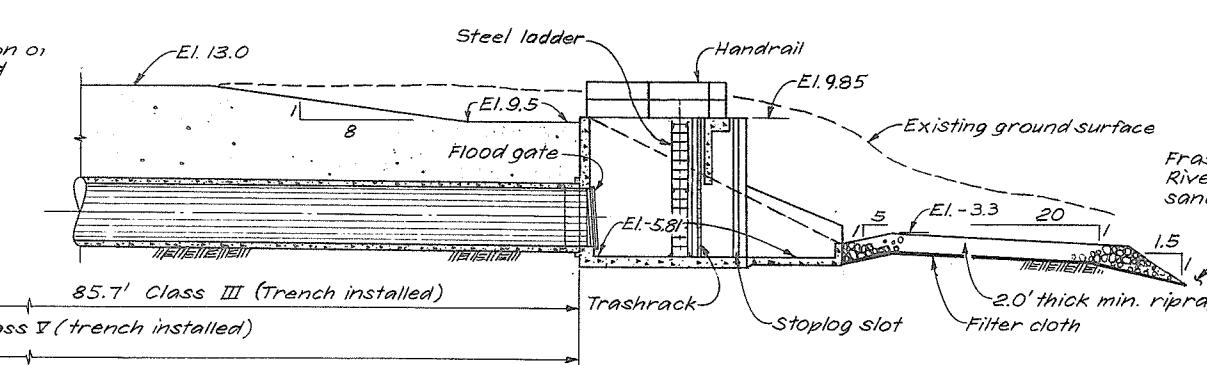
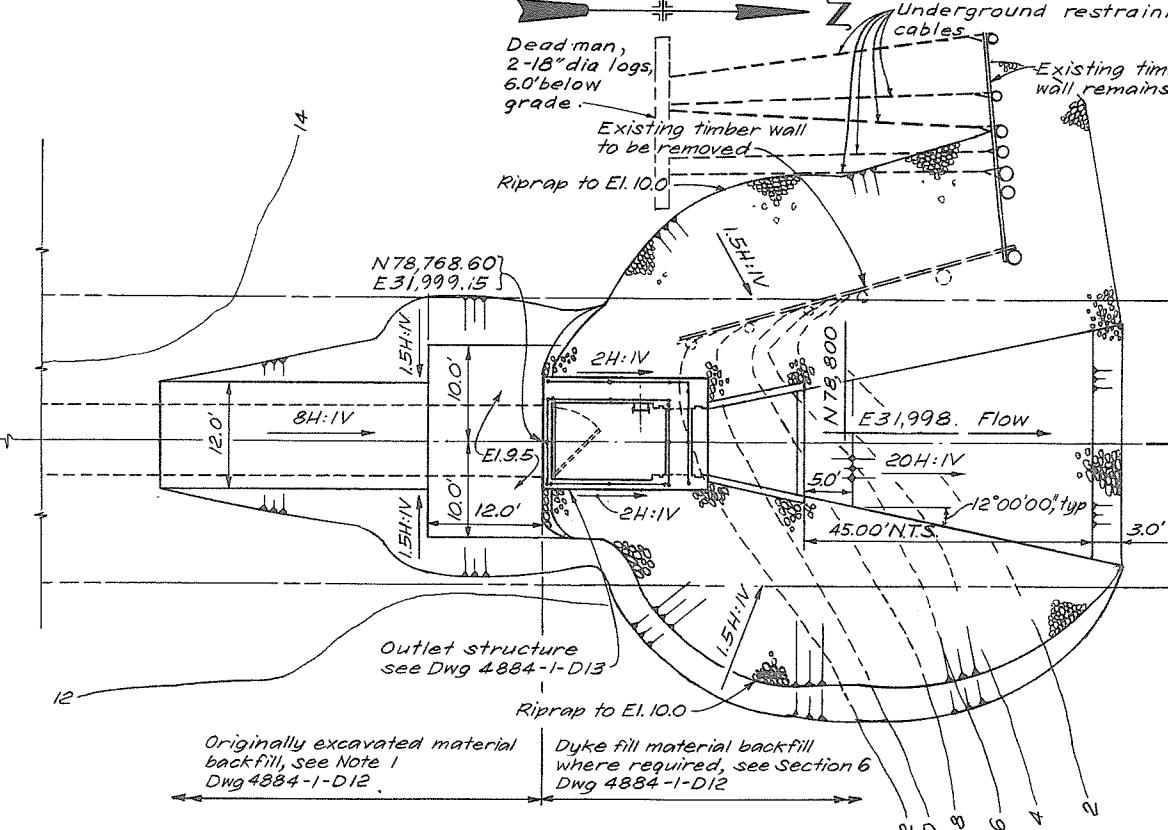
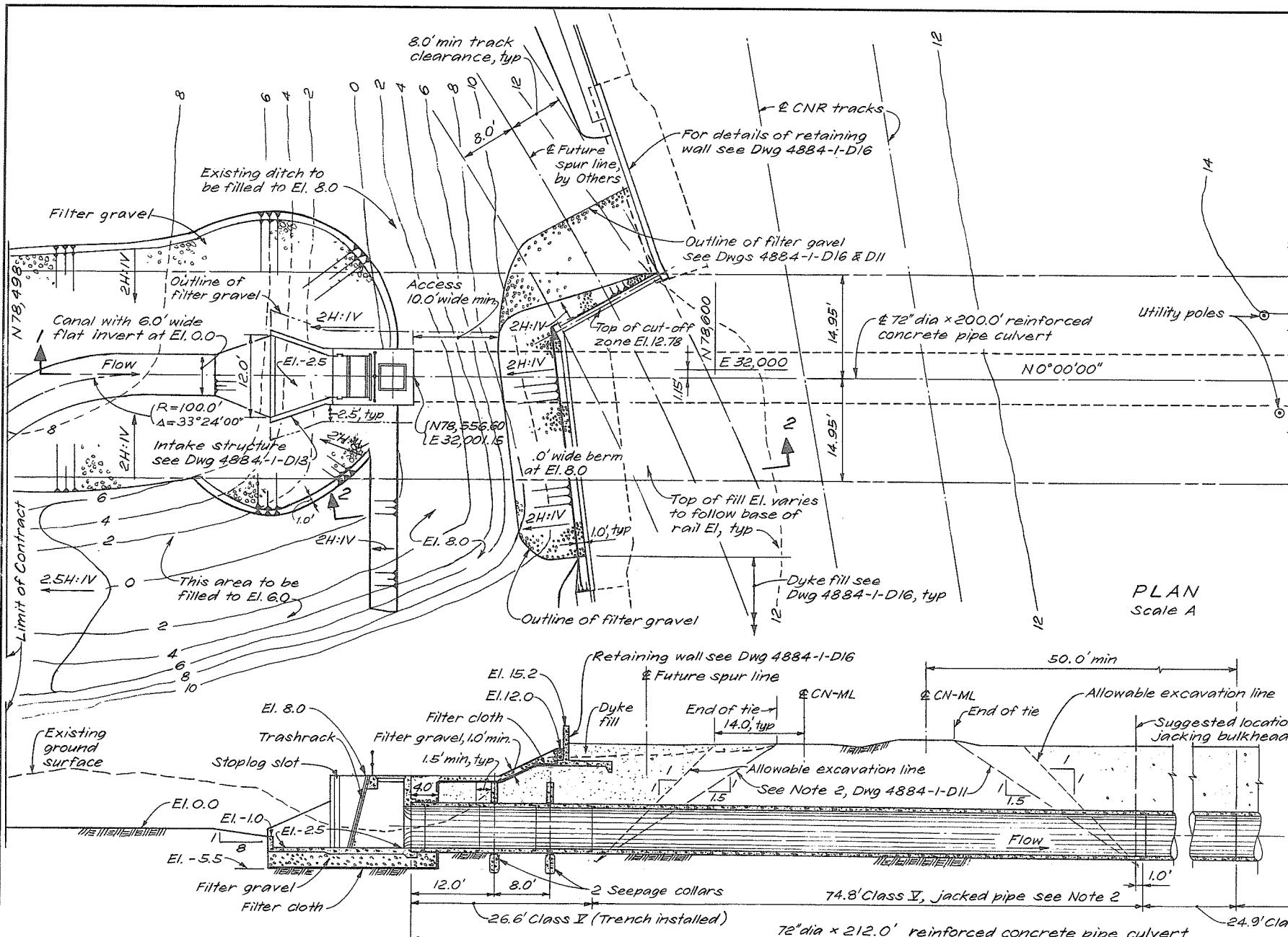
RECOMMENDED *Eduard B.*
PROJECT MANAGER
DATE Aug 30 1978
FL *BB* 20.2.78
APPR *MM* 20.2.78
DATE Aug 30 1978
APPROVED *John T. Fuller*
for DIRECTOR, WATER INVESTIGATIONS
DATE Sep 878

BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRAZER RIVER FLOOD CONTROL 1968 AGREEMENT
PROJECT 10.4 CONTRACT NO. 1
SOUTH WESTMINSTER FLOOD CONTROL WORKS
ROAD CROSSINGS & STOPLOG WALL DETAILS

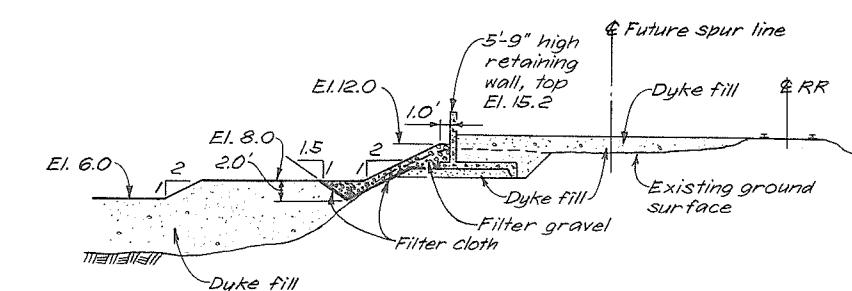
DESIGNED *John C.*
DRAWN *A. A. MP*
CHECKED *NAC*
SCALE *As shown*
DWG. NO. 4884-1-D18/R2
SHEET II OF 20

20





SECTION
Scale A



*SECTION
Scale A*



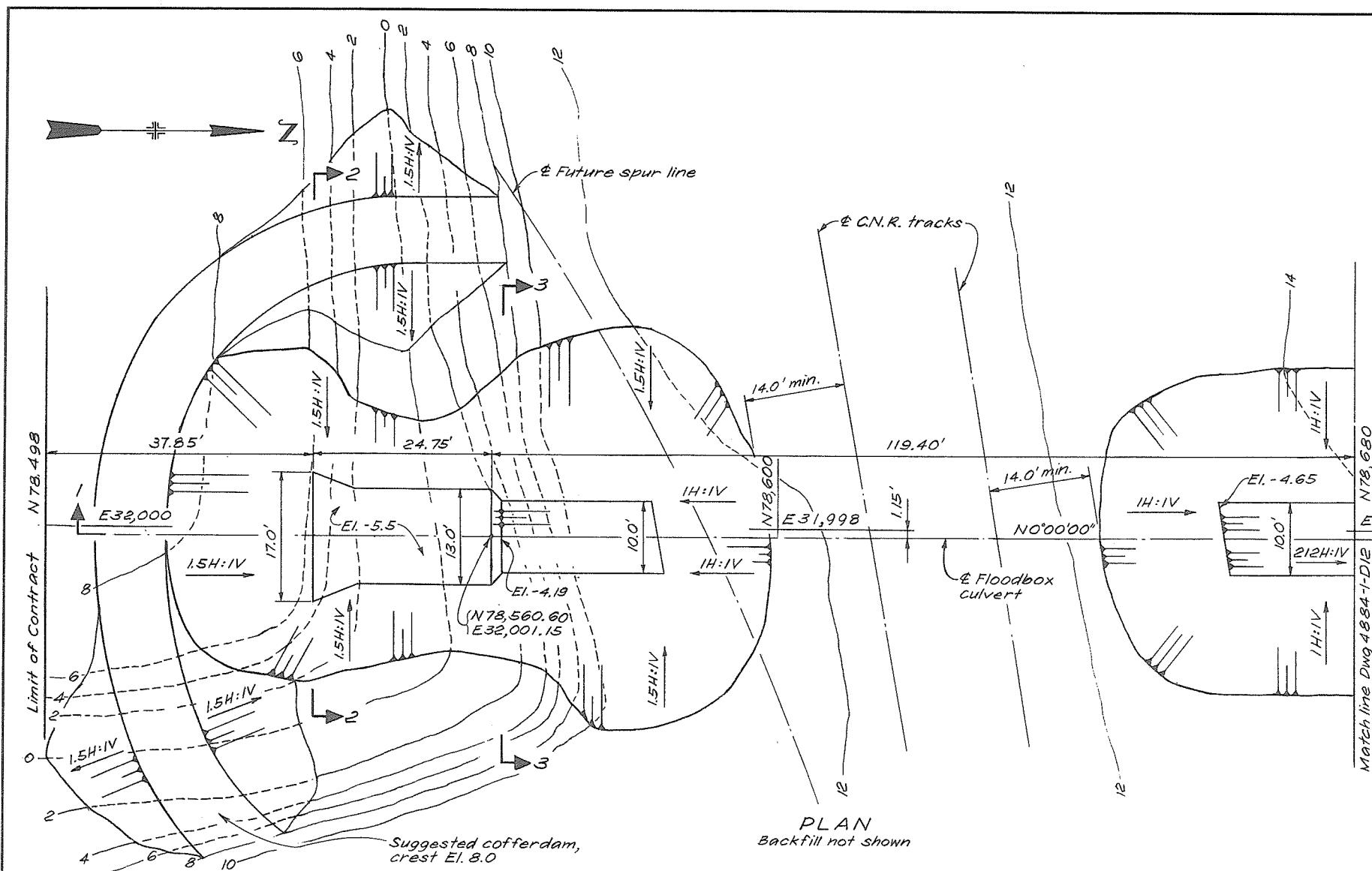
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|---|--|
|  | CRIPPEN ENGINEERING LTD. NORTH VANCOUVER, B.C. <small>PROJECT NO. 10405</small> |
| DEPARTMENT HEAD <i>R. G. L.</i> | |
| PROJECT ENGINEER <i>M. M. L.</i> | |
| CHIEF ENGINEER <i>J. A. R. S.</i> | |

3. As built.
2. Outlet structure invert El. revised
APPROVED FOR CONSTRUCTION MAY 17 1961
1. Filter cloth & filter gravel extended.

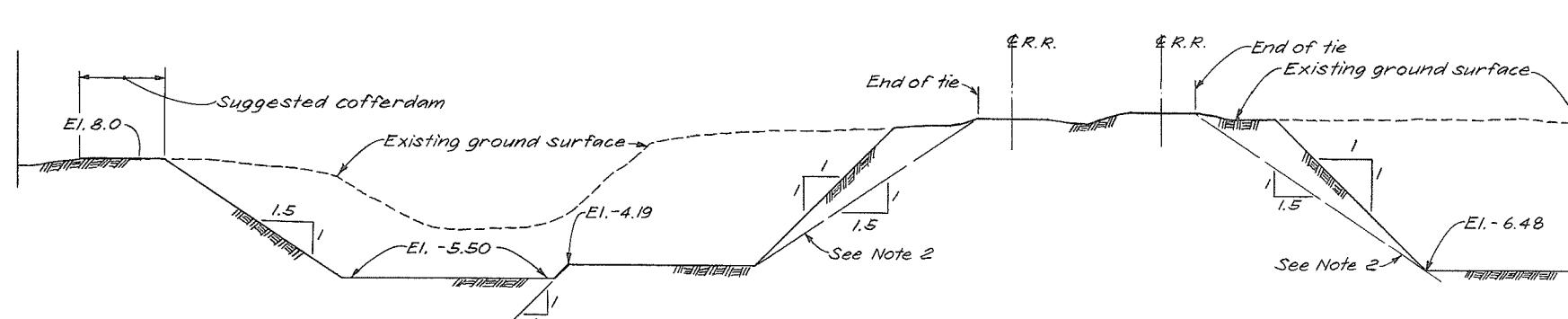
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|--------------------|----------------|---|-----------------|
| RECOMMENDED | | <i>Bob Ballo</i> | PROJECT MANAGER |
| 9-2-81 14-11-79 | DATE | <i>Aug 30 1978.</i> | |
| 20-279 | APPROVED | <i>M. Feltner</i> for DIRECTOR, WATER INVESTIGATIONS | |
| DATE | <i>See 848</i> | | |

**BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1988 AGREEMENT**

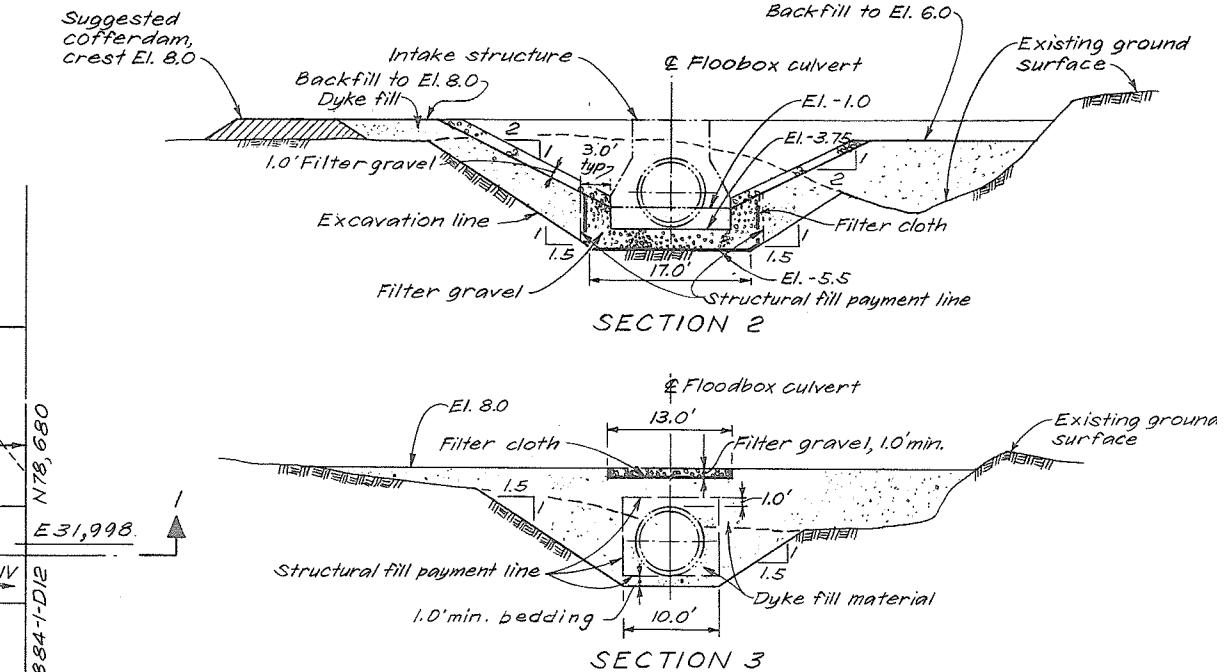
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|--|----------|---------------------|----------------|
| BRITISH COLUMBIA MINISTRY OF THE ENVIRONMENT WATER INVESTIGATIONS BRANCH CANADA-BRITISH COLUMBIA IVER FLOOD CONTROL 1968 AGREEMENT | DESIGNED | <i>N. McConnell</i> | SURVEYED |
| | DRAWN | FL | DATE |
| TRACT NO. 1 MINSTER FLOOD CONTROL WORKS ET FLOODBOX RANGEMENT & WORK AREAS | CHECKED | <i>ZPS</i> | FILE NO. |
| | SCALE | As shown | DATE |
| | DWG. NO. | 4884-1-DIO/R3 | SHEET 13 OF 26 |



PLAN
Backfill not shown



SECTION 1



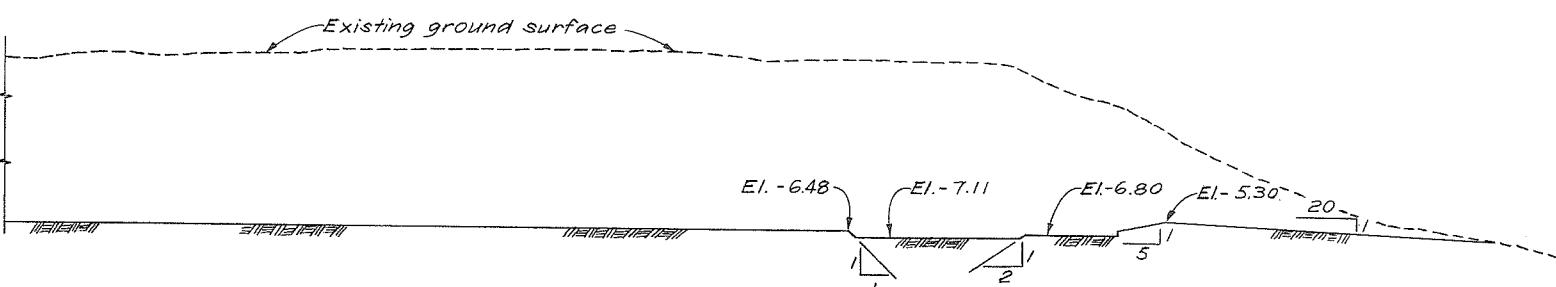
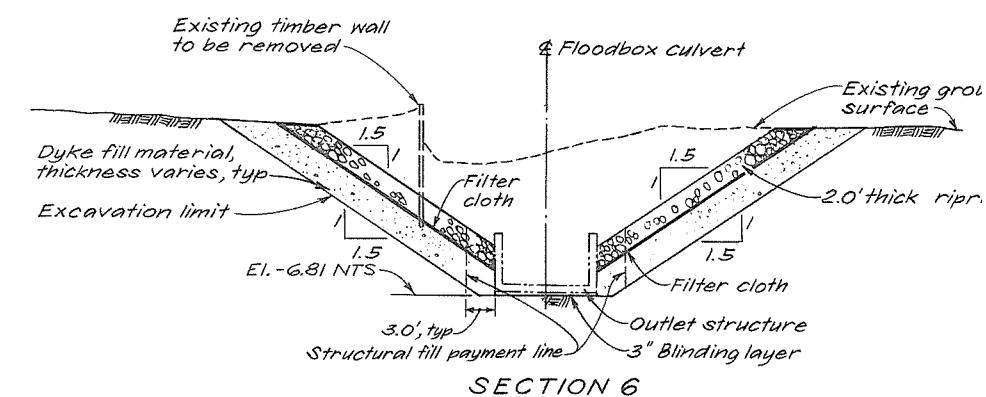
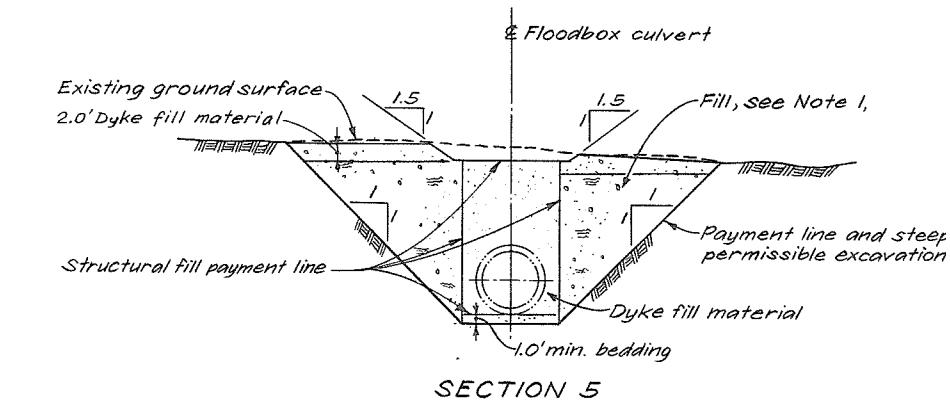
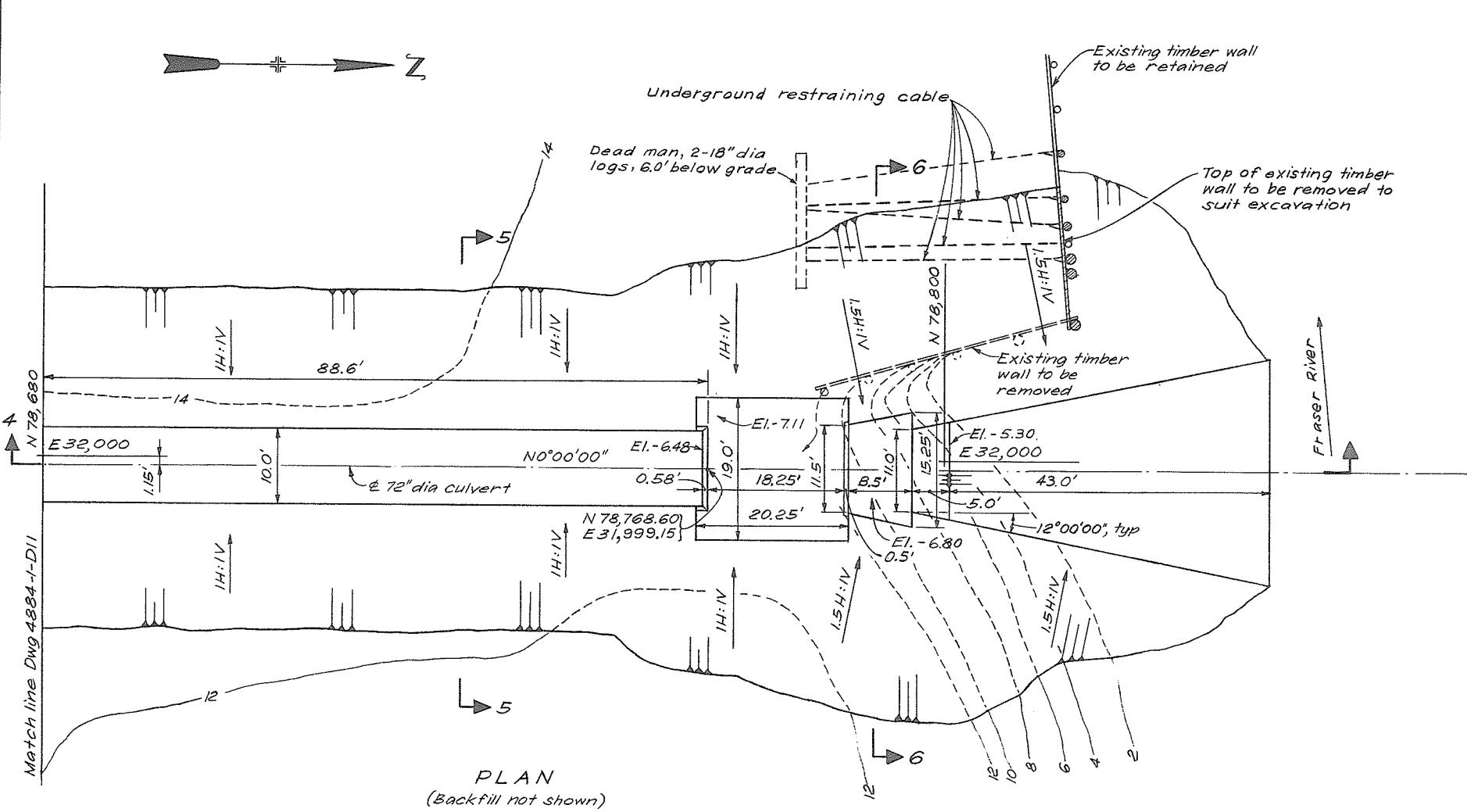
NOTE

1. Backfill outside of 10 ft wide dyke fill prism over the culvert and in the area north of the CNR rights-of-way shall be material originally excavated, except for top 2 ft.
2. Any excavation carried out within 14 ft. of the track centralline must have its design approved by the C.R.R.

10 0 10 20 30 40 50

105

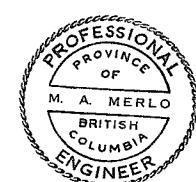
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|--|--|--|--|---|--|---|--|--|
| | | CRIPPEN ENGINEERING LTD. NORTH VANCOUVER, B.C. PROJECT NO. 10405 DEPARTMENT HEAD: <u>Herb Nussbaum</u> PROJECT ENGINEER: <u>M.A. Merlo</u> CHIEF ENGINEER: <u>J. C. L.</u> | | RECOMMENDED <u>L. G. Fuller</u> PROJECT MANAGER DATE <u>Aug 30 1978</u> APPROVED <u>L. G. Fuller</u> for DIRECTOR, WATER INVESTIGATIONS PROJECT 10.4 CONTRACT NO. 1 SOUTH WESTMINSTER FLOOD CONTROL WORKS 128 TH STREET FLOODBOX EXCAVATION & BACKFILL SHEET 1 OF 2 | | BRITISH COLUMBIA MINISTRY OF THE ENVIRONMENT WATER INVESTIGATIONS BRANCH CANADA-BRITISH COLUMBIA FRAZER RIVER FLOOD CONTROL 1968 AGREEMENT | | |
| | | | | DATE <u>Aug 28 1978</u> APPROVED FOR CONSTRUCTION <u>MAY 17 1978</u> 1. Structural fill payment line defined. APPROVED <u>L. G. Fuller</u> for DIRECTOR, WATER INVESTIGATIONS PROJECT 10.4 CONTRACT NO. 1 SOUTH WESTMINSTER FLOOD CONTROL WORKS 128 TH STREET FLOODBOX EXCAVATION & BACKFILL SHEET 1 OF 2 | | DESIGNED <u>N. McConnell</u> DRAWN <u>FL</u> CHECKED <u>1</u> SCALE <u>As shown</u> DWG. NO. <u>4884-1-D11/R2</u> SHEET <u>14</u> OF <u>20</u> | | |
| | | | | DESCRIPTION <u>REVISIONS</u> <u>BY</u> <u>CHO</u> <u>APPR</u> <u>DATE</u> <u>REVISIONS</u> <u>BY</u> <u>CHO</u> <u>APPR</u> <u>DATE</u> | | | | |



NOTE

- 1. Backfill outside of 10 ft wide dyke fill prism over 1 culvert and in the area north of the CNR right shall be material originally excavated except as indicated in Section 6 and on Plan Dwg 4884-1-D1C
- 2. Sheet piling was used in lieu of cofferdam during construction of floodbox.

| | | | | | | |
|-----|---|----|----|----|----|----|
| 10 | 0 | 10 | 20 | 30 | 40 | 50 |
| 105 | | | | | | |



CRIPPEN ENGINEERING LTD.
NORTH VANCOUVER, B.C.
PROJECT NO. 10405

DEPARTMENT HEAD: Harold J. Blasbom
PROJECT ENGINEER: M. A. Merlo
CHIEF ENGINEER:

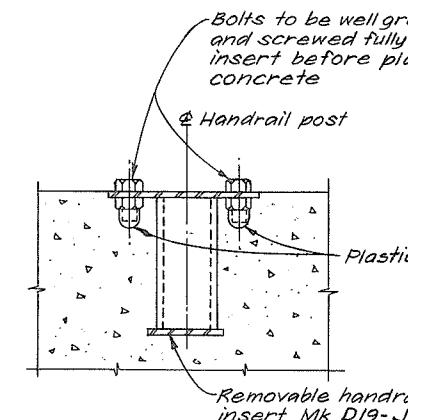
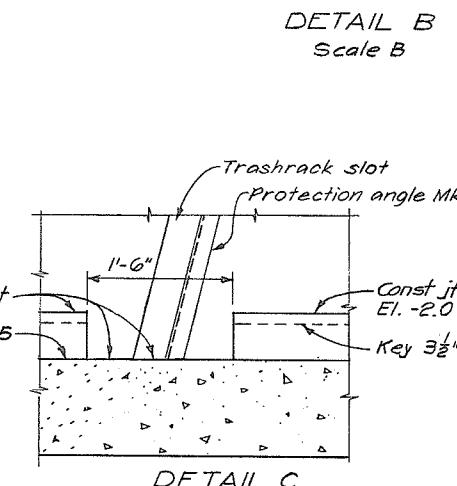
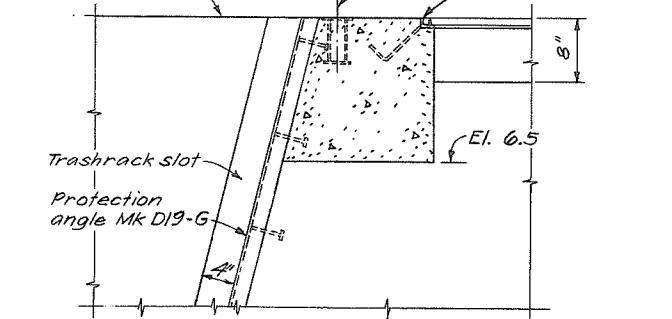
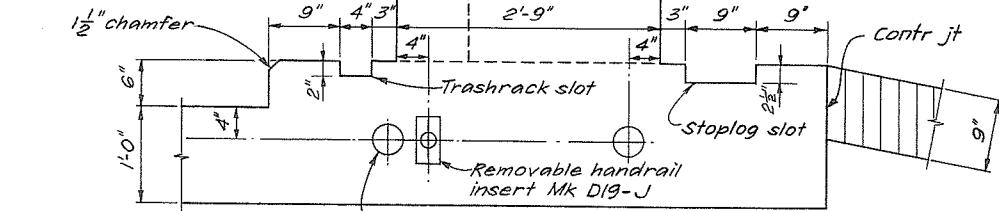
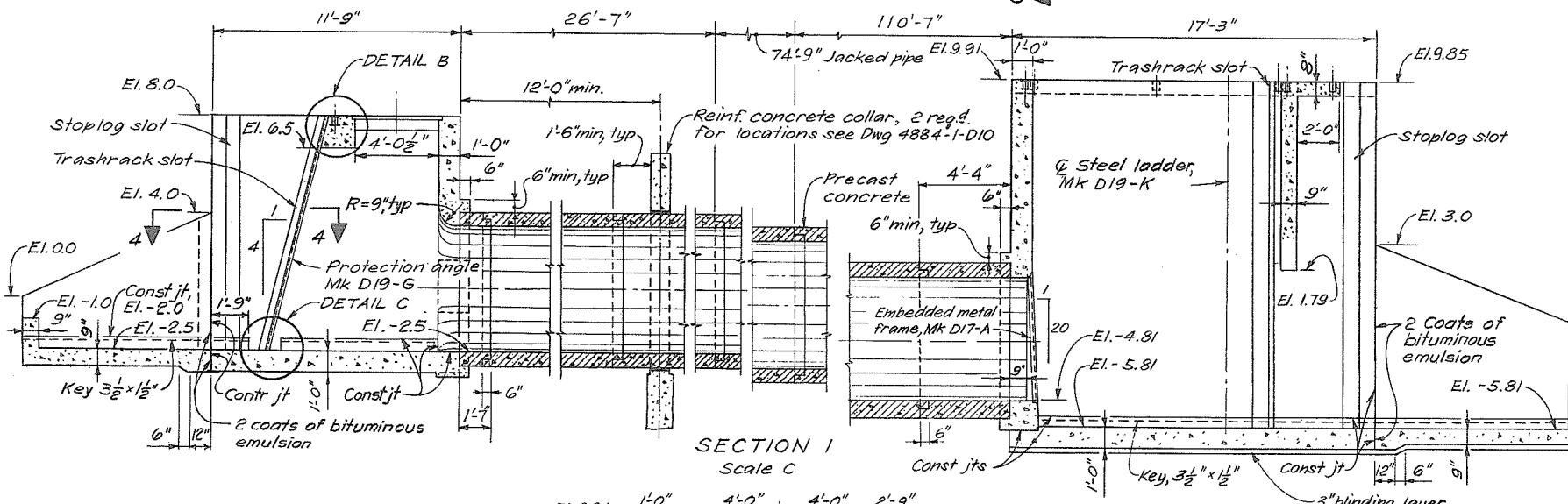
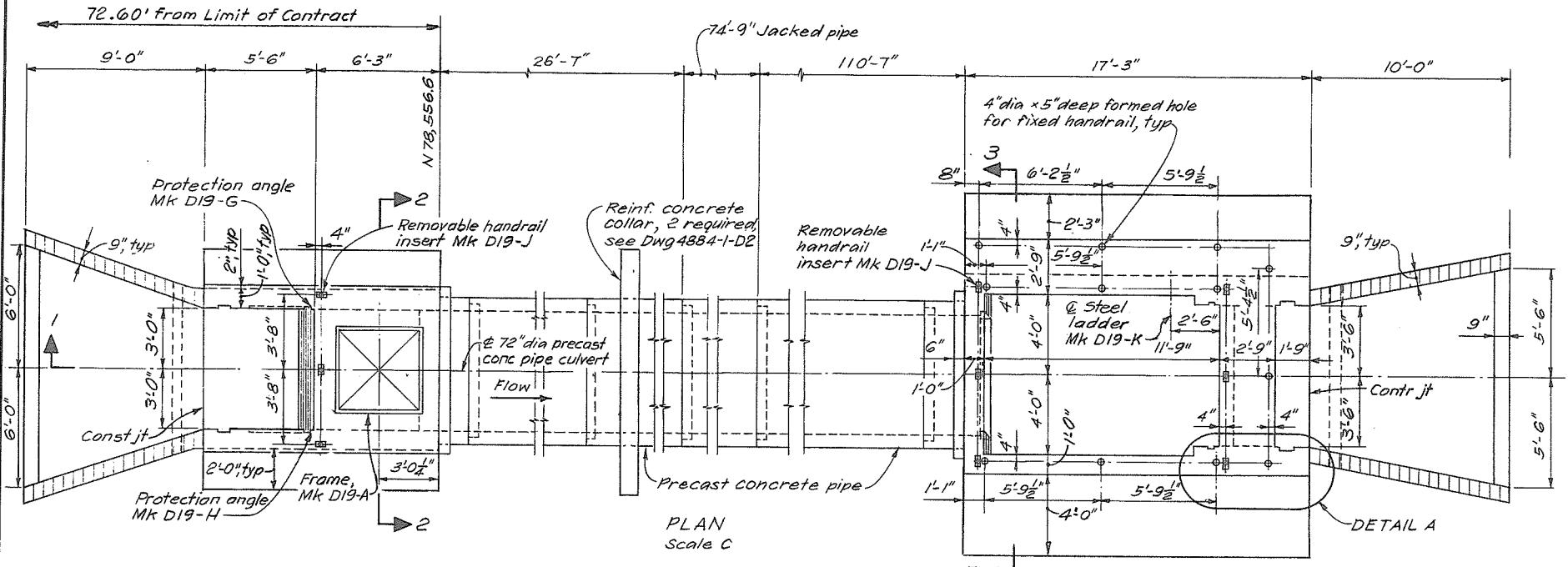
2 As built.
APPROVED FOR CONSTRUCTION MAY 17 1978
1. Structural fill payment line defined.

| | | |
|-------------|------------------|-----------------|
| RECOMMENDED | <u>Eric Bond</u> | PROJECT MANAGER |
| DATE | Aug 30 1978 | |
| FL | 20-279 | 9-2-81 |
| NO. | | |
| DESCRIPTION | BY | CHO APPR DATE |
| REVISIONS | | |

BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT

PROJECT 10.4 CONTRACT NO. I
SOUTH WESTMINSTER FLOOD CONTROL WORKS
128 TH STREET FLOODBOX
EXCAVATION & BACKFILL SHEET 2 OF 2

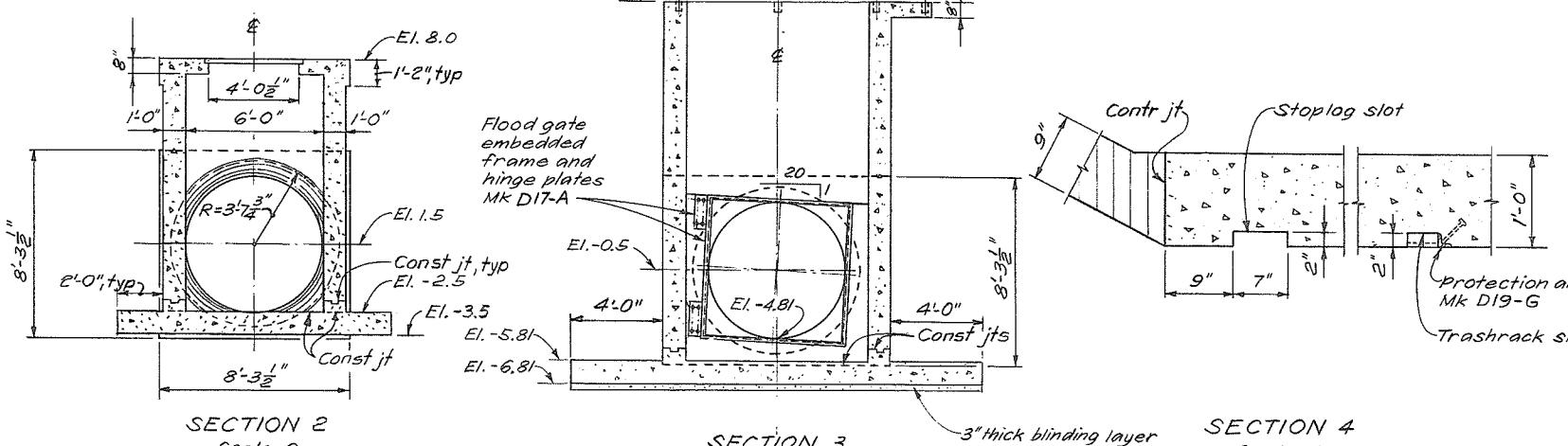
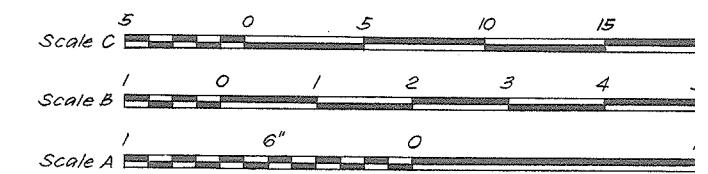
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| DRAWN | FL | DATE | |
| CHECKED | <u>R. A. ...</u> | FILE NO. | 0281550-1 |
| SCALE | As shown | DATE | 29 Aug 1 |
| DWG. NO. | 4884-1-D12/R2 | SHEET | 15 OF 20 |



*INSERT FOR
REMOVABLE HANDRAIL
Scale A*

NOTES

1. For general arrangement see Dwg D10.
 2. For seepage collars see Dwg D2.
 3. For reinforcement see Dwg D14.
 4. Concrete shall be Class I.
 5. Precast concrete 72" internal diameter culvert sections with exposed reinforcement to be embed in in-situ inlet and outlet as shown.



| | |
|---|---|
|  | CRIPPEN ENGINEERING LTD. NORTH VANCOUVER, B.C. PROJECT NO. 10405 |
| DEPARTMENT HEAD <i>E. F. K. R.</i> | |
| PROJECT ENGINEER <i>M. M. Johnson</i> | |
| CHIEF ENGINEER | |

2 As built.
1. Outlet structure inverts r

N 4
B

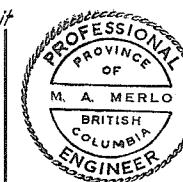
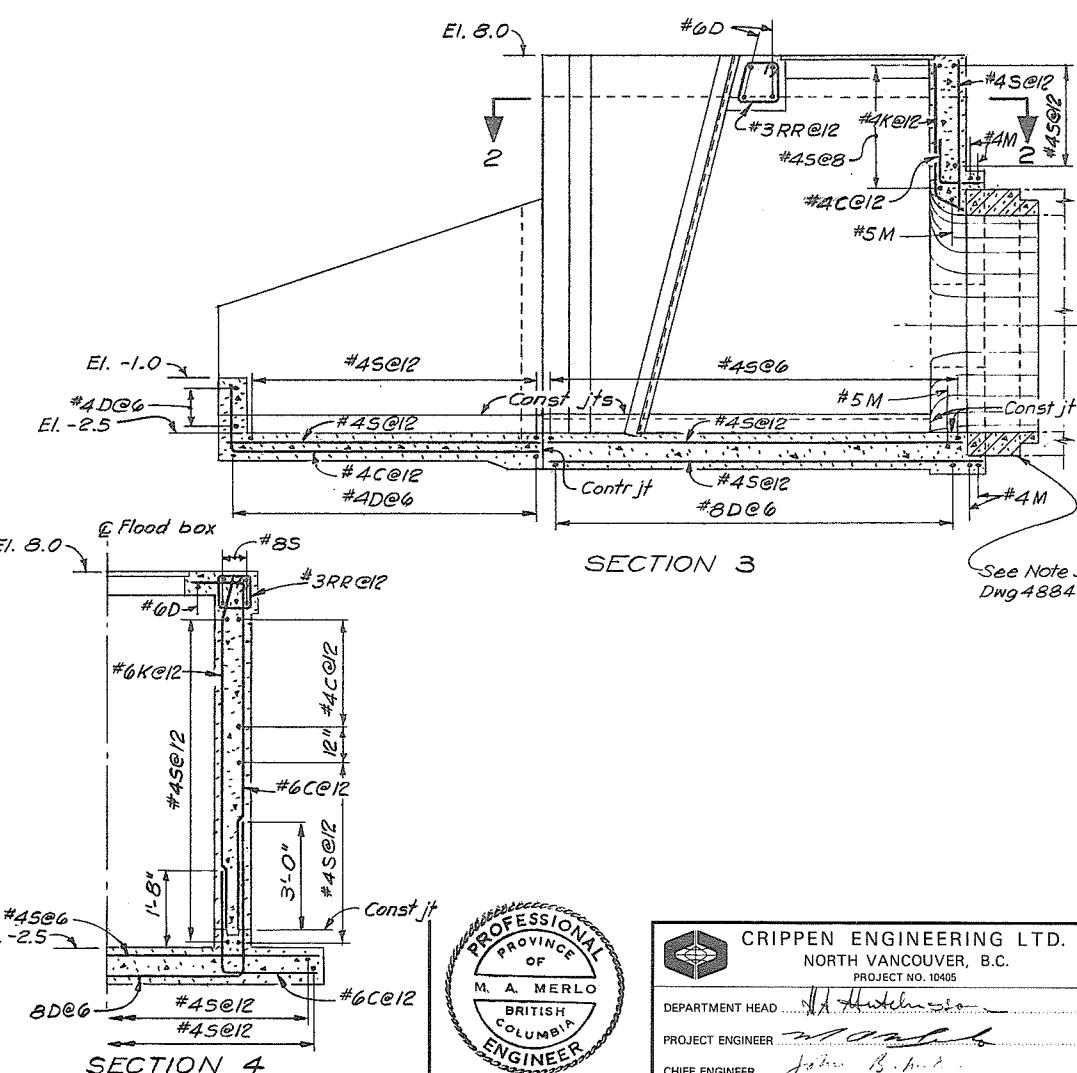
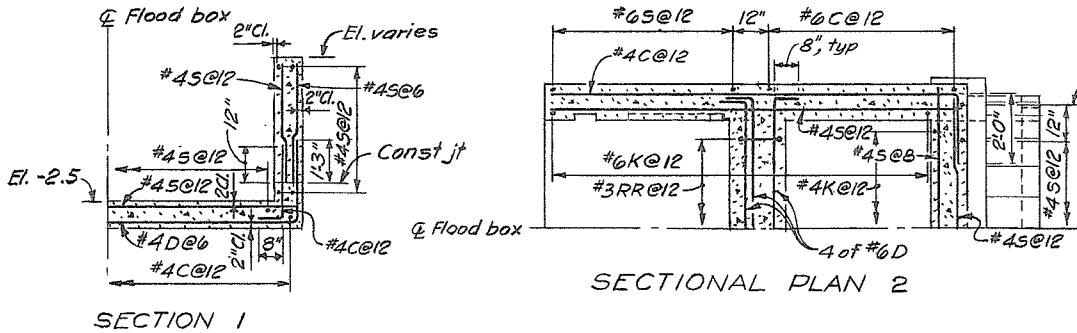
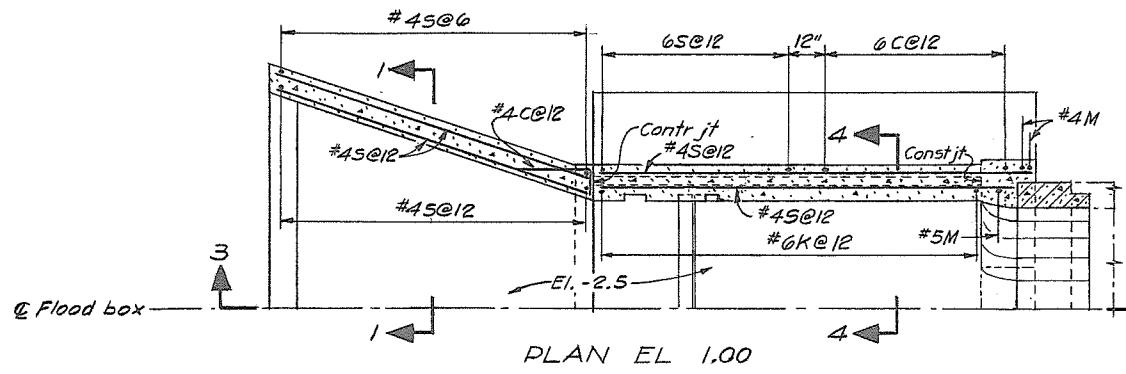
*SECTION
Scale B*

2 As built.
1. Outlet structure inverts revised.

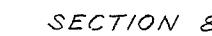
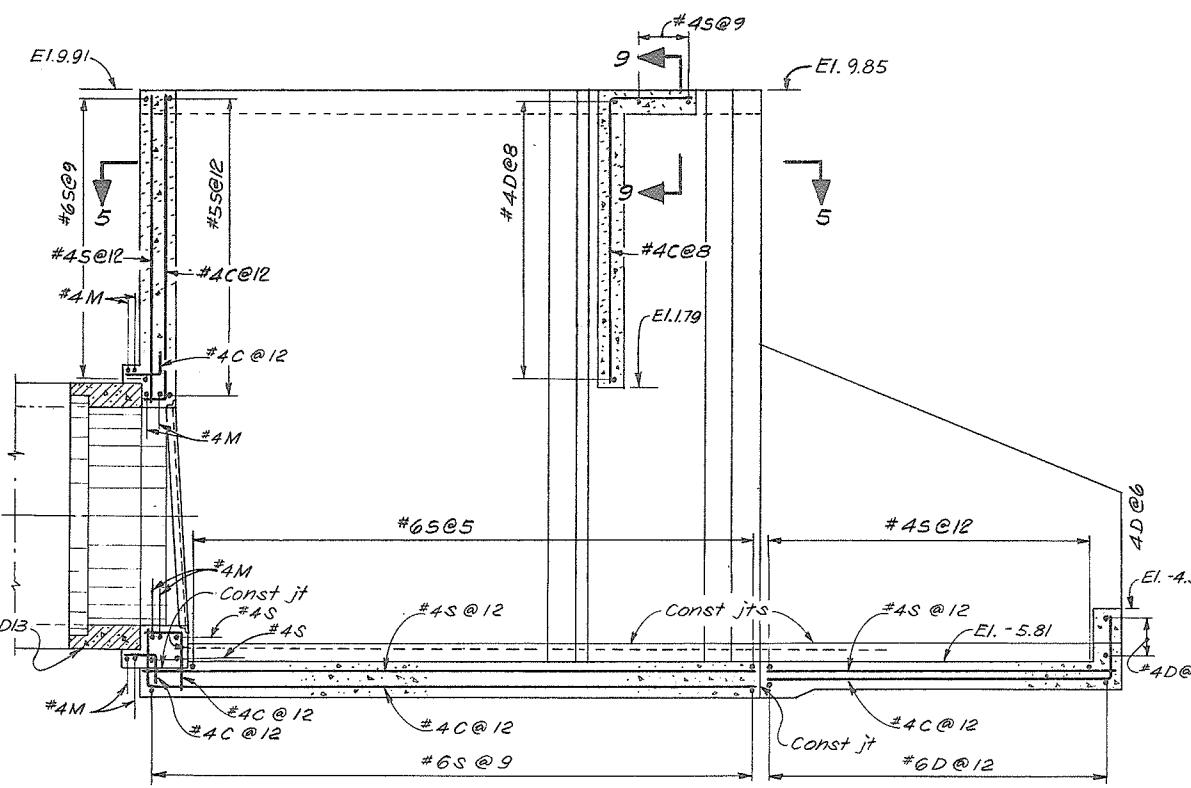
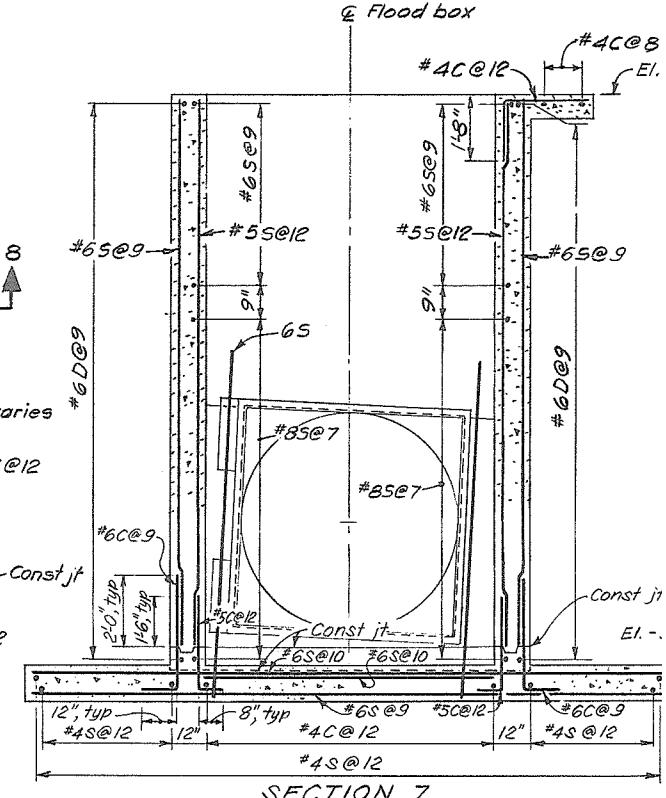
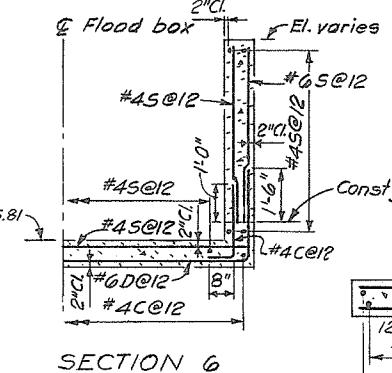
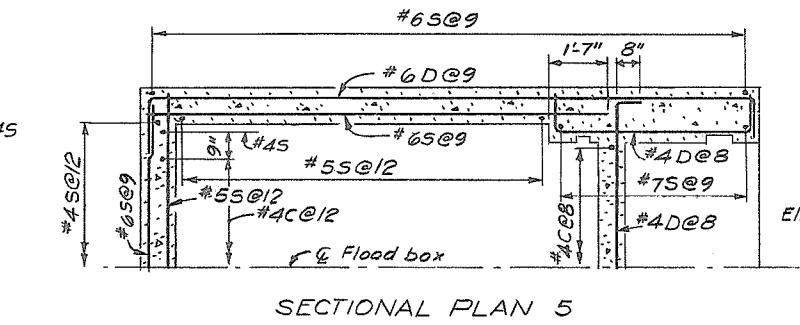
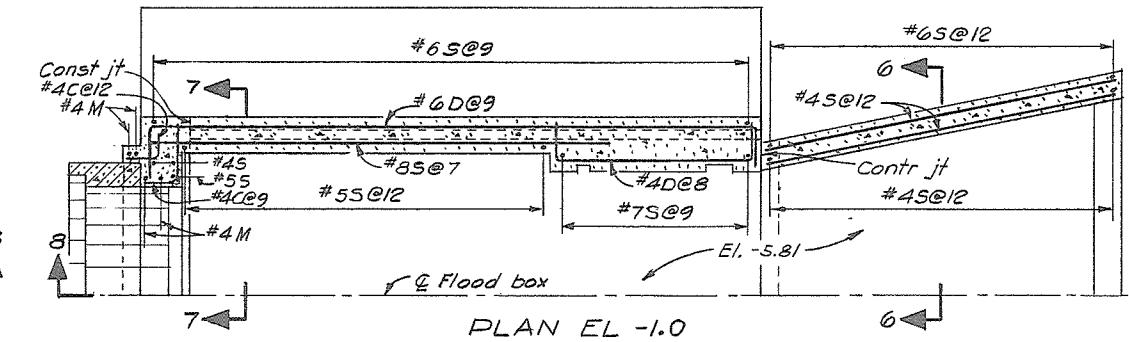
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|--------------------|----------|--|
| RECOMMENDED | | <i>Emerson De</i> |
| | | PROJECT M |
| 10-281 14-11-79 | DATE | Aug 30 1978 |
| | APPROVED | <i>W.B. Fuller</i> for DIRECTOR, WATER INVESTIGATIONS |
| DATE | Sep 8/18 | |

**BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT**

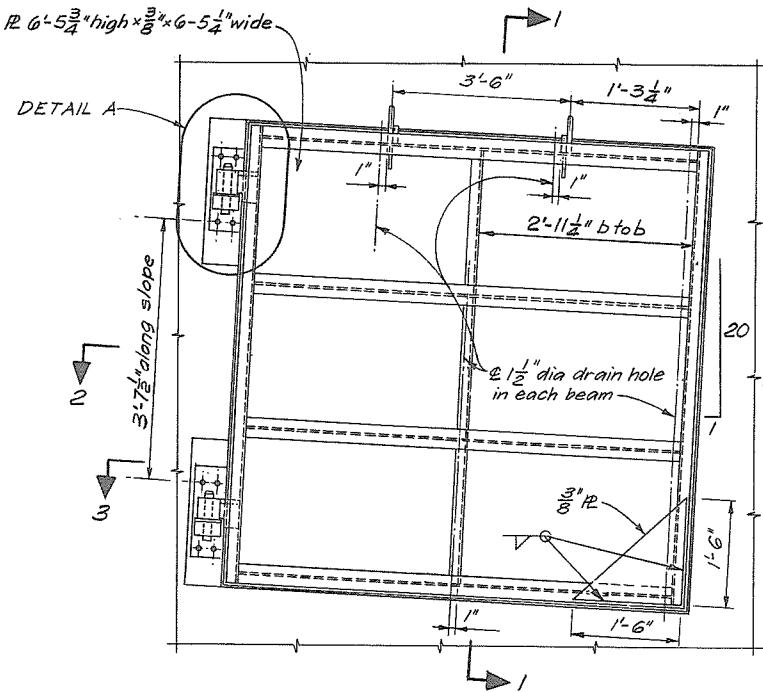
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| DESIGNED | <i>[Signature]</i> | SURVEYED |
| DRAWN | FL | DATE |
| CHECKED | <i>[Signature]</i> | FILE NO. |
| SCALE | As shown | DATE |
| DWG. NO. | 1001-L-B7-P2 | 10-10 |



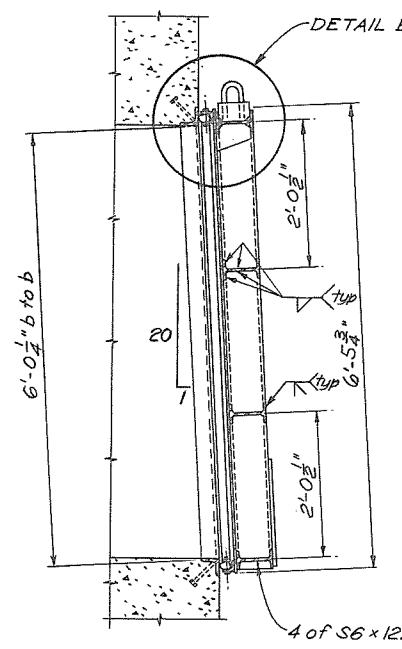
| | | |
|---|---------------------------|--|
|  | CRIPPEN ENGINEERING LTD. | |
| | NORTH VANCOUVER, B.C. | |
| | PROJECT NO. 10405 | |
| DEPARTMENT HEAD | <u>H. H. Strelakowski</u> | |
| PROJECT ENGINEER | <u>M. A. L. S.</u> | |
| CHIEF ENGINEER | <u>J. B. A. S.</u> | |



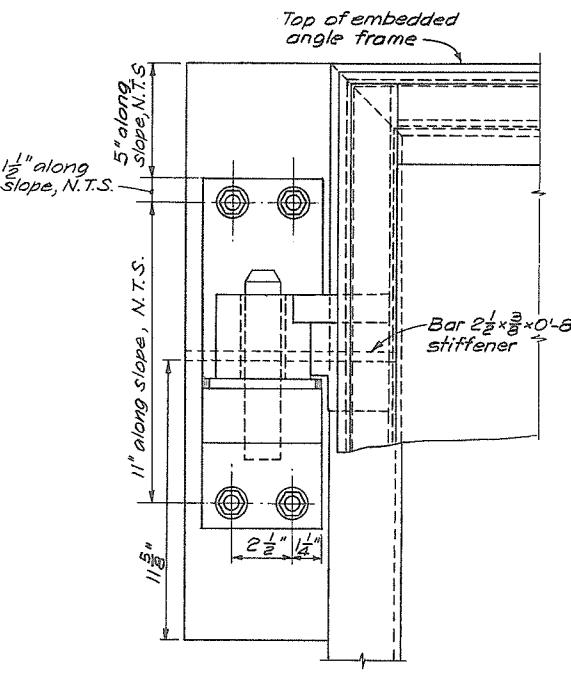
1. Reinforcement shown in sections to be symmetrical unless shown otherwise.
 2. For concrete outline, see Dwg. D2.
 3. Reinforcement to be Grade 40 deformed bars conforming to CSA G30.12.
 4. Min. lap length to be 30 bar diameters for #7 or smaller and 43 bar diameters for larger bars unless otherwise specified.
 5. Dowels to be located to splice with bars in subsequent concrete placement.
 6. Concrete cover to reinforcement to be 3", except as otherwise shown.
 7. Concrete to be Class I. See Detail Specification, Section 8.
 8. Refer to Detailed Specifications Section 9 for precast concrete pipe, reinforcement and concrete requirements.



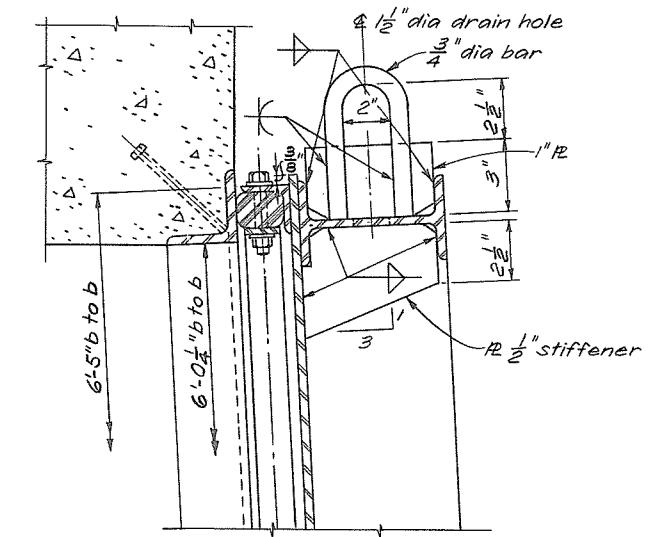
ELEVATION - FLOODGATE
Mk A, One required
Scale A



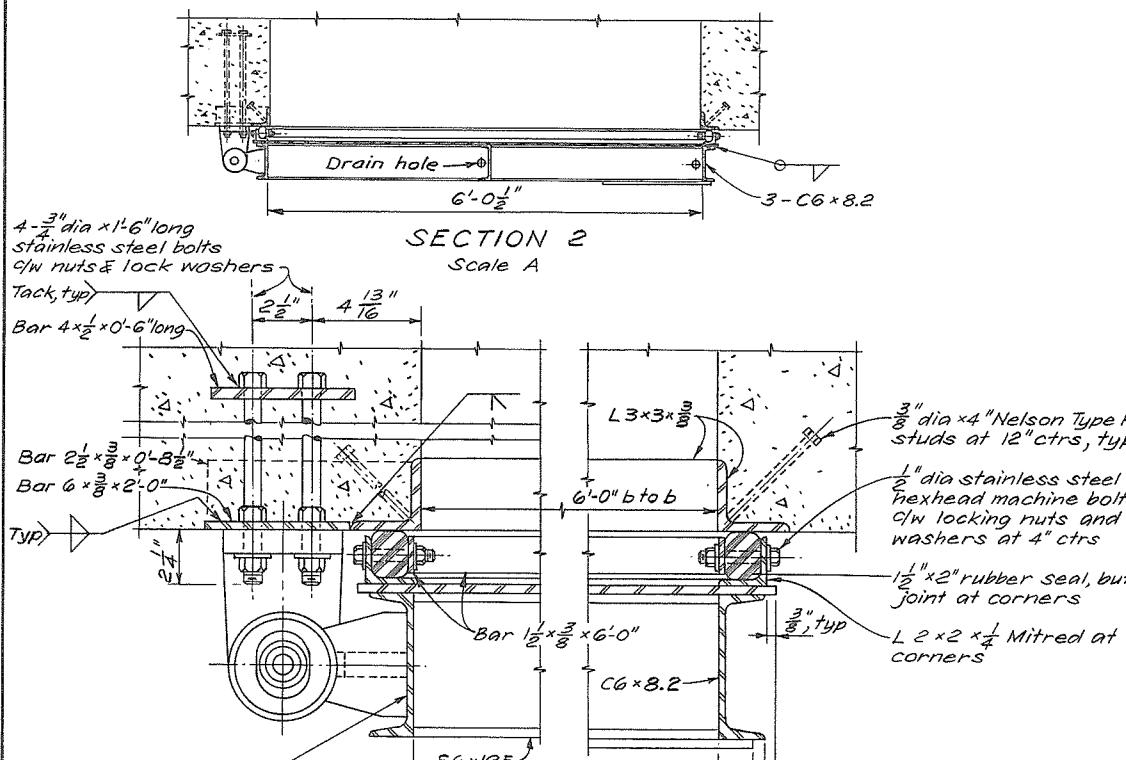
SECTION 1
Scale A



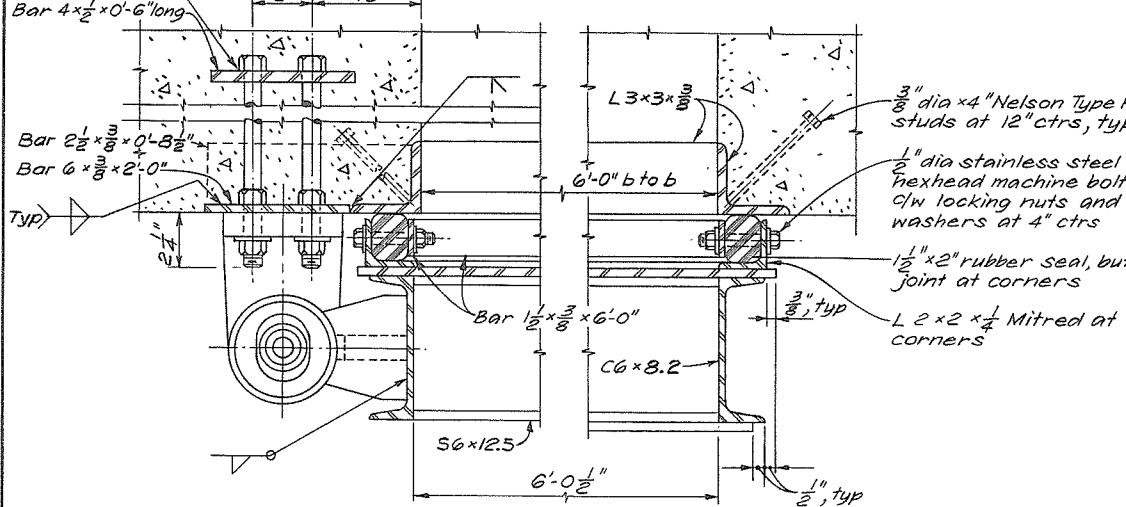
DETAIL A
Scale B



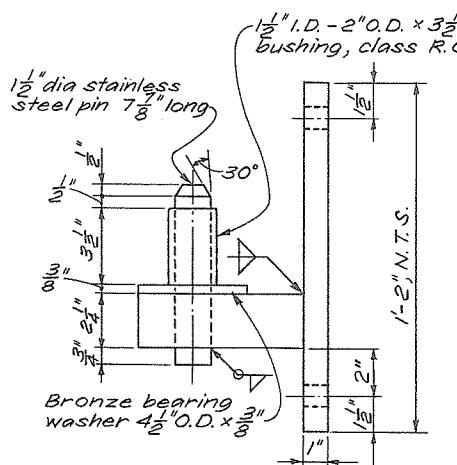
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Scale B



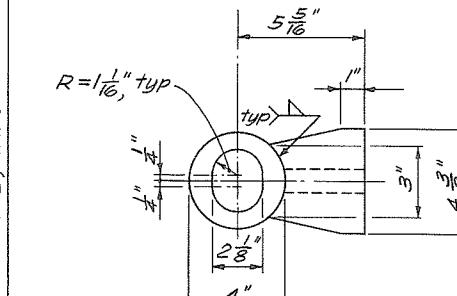
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Scale A



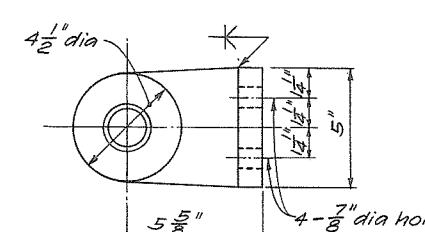
SECTION 3
Scale B



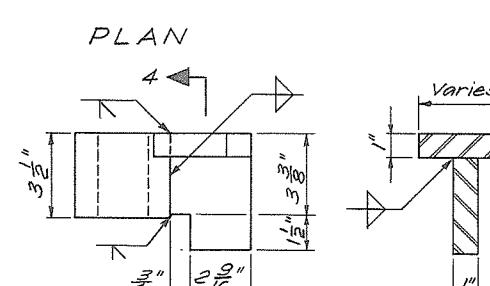
ELEVATION



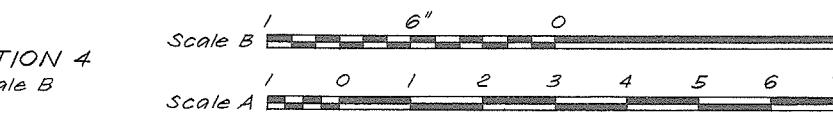
PLAN



PLAN
HINGE PINTLE
Scale B



ELEVATION
HINGE BRACKET
Scale B



NOTES

- Mk letters to be prefixed D17 and clearly paint on assemblies.
- Protective coatings to be in accordance with Clause 12.6 of the Specification.
- Floodgate & frame to be grade G40.21-44 except as otherwise noted.
- Reinforcement shall be adjusted to clear stud anchors.

NOTE ON DESIGN

Design adapted from Dwg 4944-1-D125

By: M.A.N.

Checked: N.A.C.

Original designed by A.D. checked by M.G.B., 6 Aug



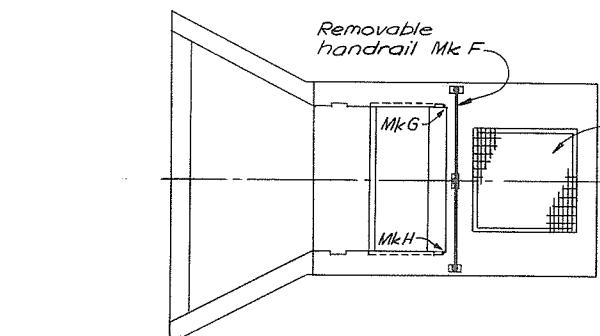
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|--|------------|
| CRIPPEN ENGINEERING LTD. NORTH VANCOUVER, B.C. PROJECT NO. 10405 | |
| DEPARTMENT HEAD | C.C. Blair |
| PROJECT ENGINEER | M.G.B. |
| CHIEF ENGINEER | |

As built.
APPROVED FOR CONSTRUCTION MAY 17 1978
NO. 102-81
DESCRIPTION BY CHD APPR DATE
REVISIONS

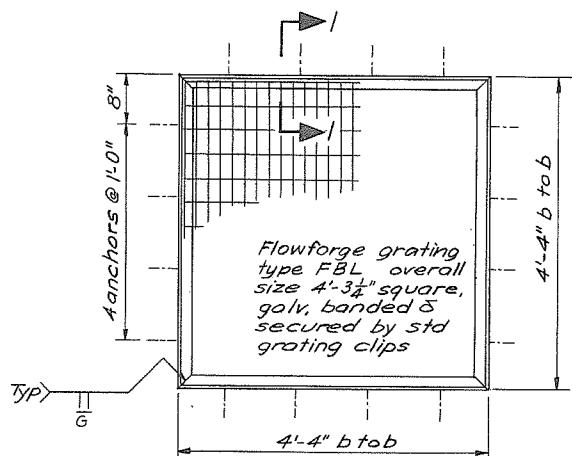
| | |
|-------------|------------------------------------|
| RECOMMENDED | Aug. 30 1978 |
| DATE | Pin # 102-81 |
| APPROVED | for DIRECTOR, WATER INVESTIGATIONS |
| DATE | Sep. 8/78 |

BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT
PROJECT IO.4 CONTRACT NO.1
SOUTH WESTMINSTER FLOOD CONTROL WORKS
128 TH STREET FLOODBOX
FLOODGATE

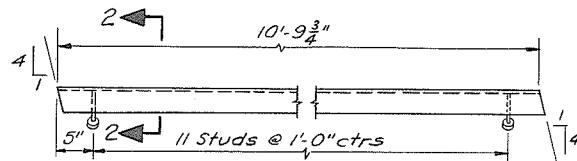
| | | |
|----------|--------------|-------------------|
| DESIGNED | See Note | SURVEYED |
| DRAWN | FL | DATE |
| CHECKED | See Note | FILE NO. 0281550- |
| SCALE | As shown | DATE 29 Aug 1. |
| DWG. NO. | 4884-1-D17/1 | HEET 18 OF 20 |



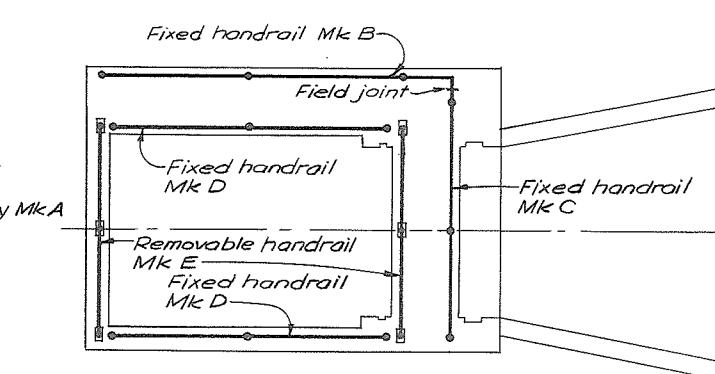
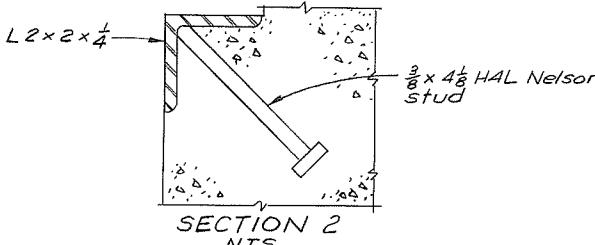
PLAN - INTAKE
Scale A



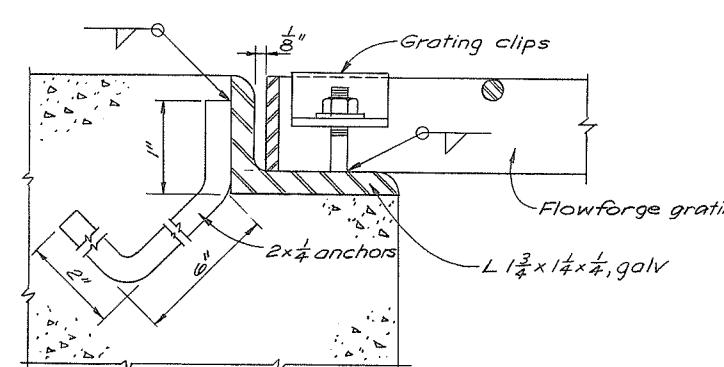
FRAME & GRATING ASSEMBLY
MK A - One reqd
Scale B



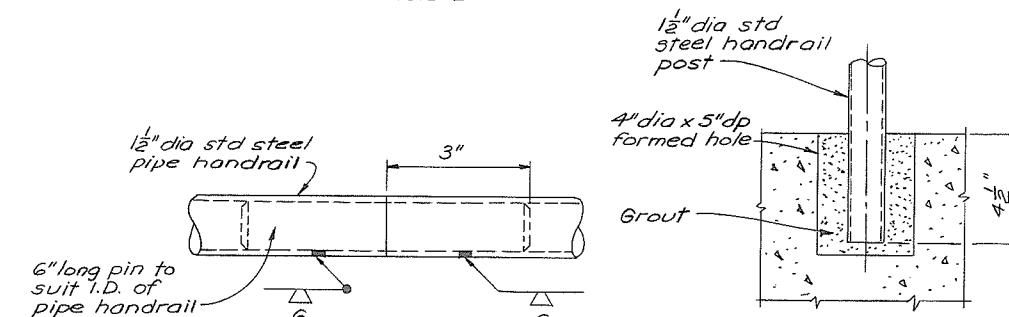
INTAKE TRASHRACK SLOT PROTECTION ANGLE
MKG Shown - one reqd
MKH Similar opp hand - one reqd
N.T.S.



PLAN - OUTLET
Scale A

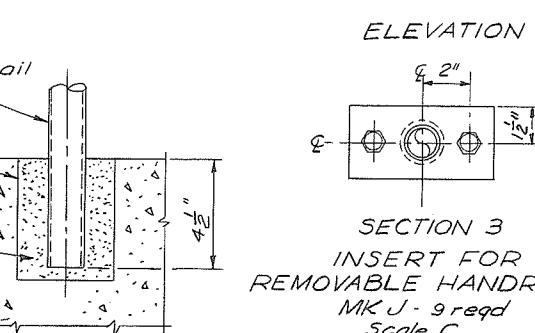


SECTION 1
scale D

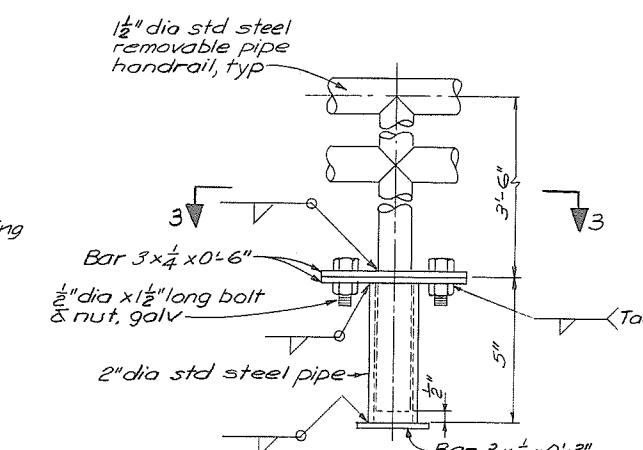


FIELD JOINT DETAIL
N.T.S.

FIXED HANDRAIL
INSTALLATION
Scale C

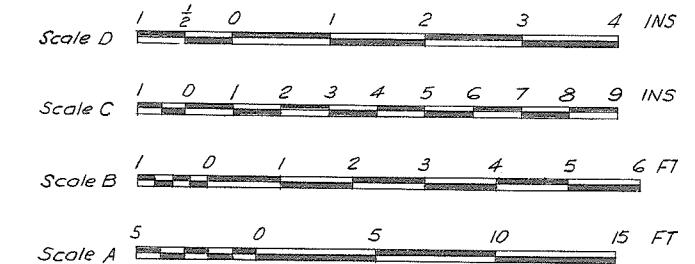


SECTION 3
INSERT FOR
REMOVABLE HANDRAIL
MK J - 9 reqd
Scale C



NOTES

- All Mk letters to be prefixed D19 and clear, painted on assemblies.
- Pipe for handrails to be Sch 40 carbon pipe & conform to CSA B63, Grade B.
- Vent holes to be drilled at underside of handrail piping as required.
- Metalwork to be hot dip galv. in accord with CSA G164.
- Areas damaged by shipping or installation to be painted with a prime coat of 2. minimum dry film thickness of Internat Paints Zinc Rich Paint 31 or equal.



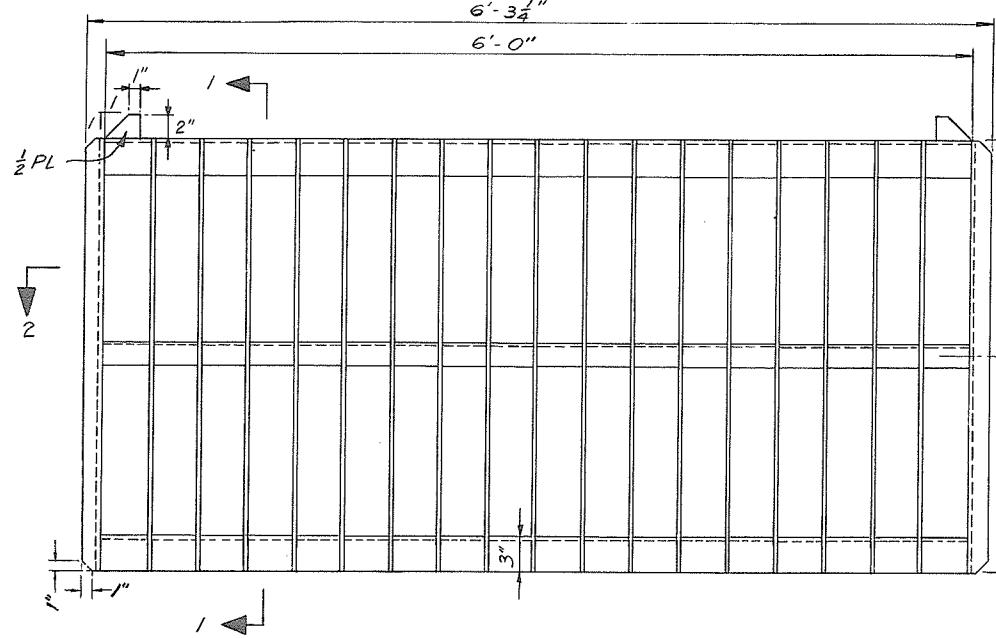
CRIPPEN ENGINEERING LTD.
NORTH VANCOUVER, B.C.
PROJECT NO. 10405
DEPARTMENT HEAD: C.P. Blane
PROJECT ENGINEER: M.A. Merlo
CHIEF ENGINEER: J.J. ...

RECOMMENDED *E. Boles*
PROJECT MANAGER
DATE Aug 30 1978
APPROVED *J.W. Fuller*
DIRECTOR, WATER INVESTIGATIONS
DATE Sep 8/78

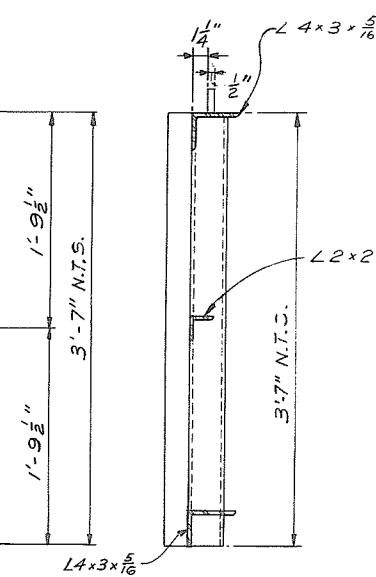
Pin 32 MM 10281
NO. APPROV'D BY CHD APPR DATE
DESCRIPTION
REVISIONS

BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT
PROJECT 10.4 CONTRACT NO. 1
SOUTH WESTMINSTER FLOOD CONTROL WORKS
128 TH STREET FLOODBOX
MISCELLANEOUS METALWORK

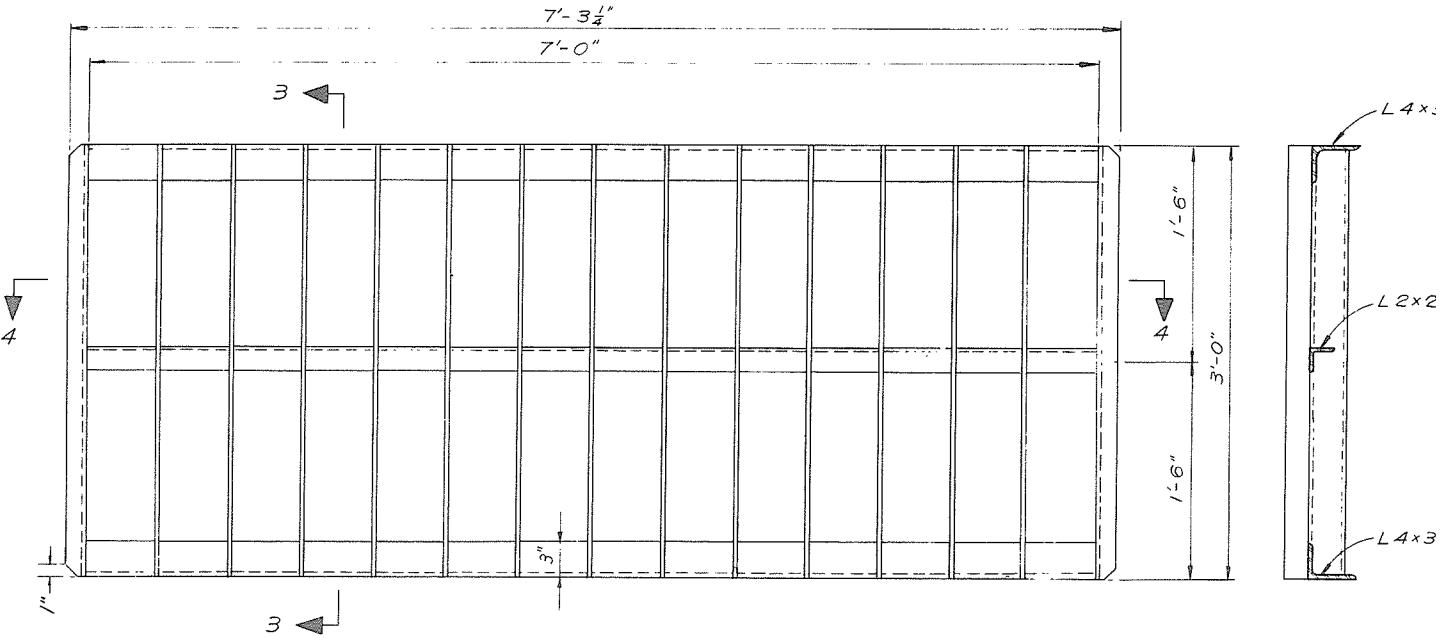
DESIGNED *M. Merlo*
DRAWN *M. Merlo*
CHECKED *N. McConnell*
SCALE As shown
DWG. NO. 4884-1-D19/A
SHEET 19 OF 20



ELEVATION - INTAKE TRASHRACK
MK A 3 required

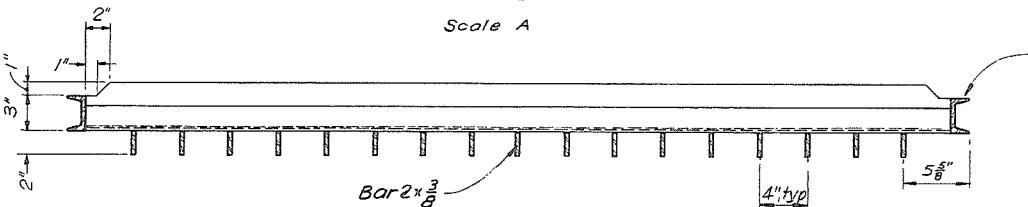


SECTION 1
Scale A

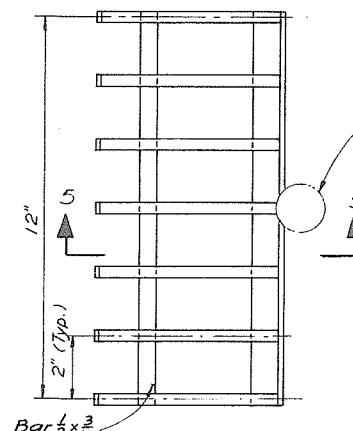


ELEVATION - OUTLET TRASHRACK
MK B 3 required

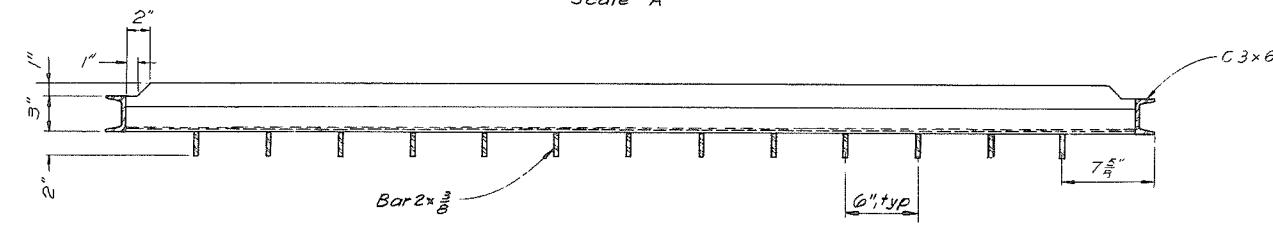
SECTION 3
Scale A



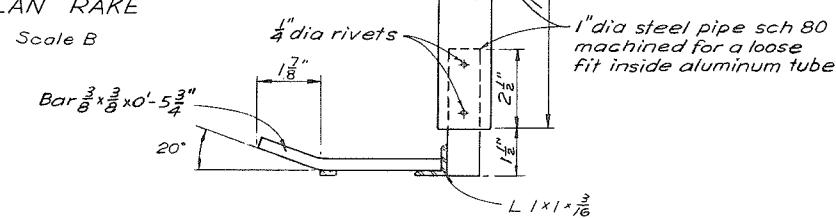
SECTION 2
Scale A



PLAN RAKE
Scale B



SECTION 4
Scale A



SECTION 5
Scale B

RAKE

NOTE ON DESIGN
Design adapted from Dwg 4944-1-D127
By: AmhP
Checked: NAC
Original designed by N. vdG. and W.K.C., checked by M.G.B., 6 Aug 1976.

NOTES

- Protective coatings to be in accordance with Clause 11.6 of the Specification.
- Full strength continuous welds throughout.
- Trashracks & rake to be grade G 40.21-44W steel except as otherwise noted



CRIPPEN ENGINEERING LTD.
NORTH VANCOUVER, B.C.
PROJECT NO. 10415
DEPARTMENT HEAD C.R. Black
PROJECT ENGINEER M.A. Merlo
CHIEF ENGINEER John B. Wilson

1 As built
APPROVED FOR CONSTRUCTION MAY 1979
PIN 88 WDM 102-81
NO. DESCRIPTION BY CHD APPR DATE
REVISIONS

RECOMMENDED PROJECT MANAGER
DATE Aug 30 1978
APPROVED DIRECTOR, WATER INVESTIGATIONS
DATE

BRITISH COLUMBIA
MINISTRY OF THE ENVIRONMENT
WATER INVESTIGATIONS BRANCH
CANADA-BRITISH COLUMBIA
FRASER RIVER FLOOD CONTROL 1968 AGREEMENT
PROJECT 10.4 CONTRACT NO. 1
SOUTH WESTMINSTER FLOOD CONTROL WORKS
128 TH STREET FLOODBOX
TRASHRACKS

DESIGNED See Note
DRAWN F.L.
CHECKED See Note
SCALE As shown
DWG. NO. 4884-1-D20/R1
SURVEYED DATE 0281550-
FILE NO. 0281550-
DATE 29 Aug 1
DWG. NO. 4884-1-D20/R1
SHEET 20 OF 20

CONCRETE TEST RESULTS

APPENDIX I

CONCRETE TEST RESULTS

CONCRETE TEST RESULTS

MIX DESIGN

| Test No. | Date Cost | Age days | Strength | | Air* | Slump* inches | Admixtures | Remarks |
|---------------|-----------|----------|----------|------------|------|---------------|----------------------------|-----------|
| | | | Test psi | Design psi | | | | |
| 1A B C | 29 Aug 79 | 7 | 2557 | | 5.0 | 2.75 | Pozzolith (300N) Flyash | Class I |
| | | 14 | 3486 | | | | | |
| | | 28 | 4043 | 4000 | | | | |
| 2A B C | 30 Aug 79 | 7 | 2086 | | 4.8 | 2.36 | Pozzolith (300N) Flyash | Class II |
| | | 14 | 2886 | | | | | |
| | | 28 | 3657 | 3000 | | | | |
| 3A B C | 30 Aug 79 | 7 | 1186 | | 5.0 | 5.51 | Pozzolith (300N) | Class III |
| | | 14 | 1529 | | | | | |
| | | 28 | 2229 | 2000 | | | | |
| 8A B C | 19 Oct 79 | 7 | 2329 | | 5.5 | 3.54 | | Class II |
| | | 28 | 3657 | | | | | |
| | | 28 | 3643 | 3000 | | | | |
| 37A B C | 6 Dec 79 | 7 | | | 3.3 | 3.0 | | |
| | | 28 | | | | | | |
| | | 28 | | 4000 | | | | |

* For Class I, II & III Specifications require 5% \pm 1%

** For Class I & II Specifications require 3 in. \pm 1 in.

*** For Class III Specifications require 6 in. \pm 1 $\frac{1}{2}$ in.

CONCRETE TEST RESULTS

Concrete Wall

| Test No. | Date Cost | Location | Air % | Slump inches | 7 day psi | 28 day psi | Remarks |
|----------|------------|--|-------|--------------|-----------|------------|---------|
| 4 | 26 Sept 79 | Footing 108+75 to 109+25 107+75 to 108+25 and 106+75 to 107+25 | 5.0 | 2.76 | 1843 | 3543 | |
| 5 | 4 Oct 79 | Footing - Musqueam (W) Road Crossing | 4.2 | 2.0 | 2514 | 3957 | |
| 6 | 12 Oct 79 | Wall 104+15 to 103+65 | 5.9 | 3.35 | 1772 | 2943 | |
| 7 | 19 Oct 79 | Wall 100+15 to 100+65 | 5.4 | 3.74 | 2343 | 3671 | |
| 9 | 23 Oct 79 | Footing 92+70 to 92+20 | - | 3.35 | 2157 | 4000 | |
| 10 | 23 Oct 79 | Wall 96+70 to 97+20 | 4.1 | 3.74 | 2100 | 3943 | |
| 11 | 24 Oct 79 | Wall 95+20 to 95+70 | 3.4 | 2.76 | 2929 | 4507 | |
| 12 | 25 Oct 79 | Wall 95+70 to 96+20 | 4.8 | 4.72 | 2214 | 3700 | |
| 13 | 26 Oct 79 | Wall 94+20 to 94+70 | 4.8 | 2.76 | 2515 | 4086 | |
| 14 | 29 Oct 79 | Wall 94+00 | 4.3 | 3.94 | 1914 | 3686 | |
| 15 | 31 Oct 79 | Wall 90+70 to 91+20 | 5.5 | 3.94 | 1986 | 3829 | |

CONCRETE TEST RESULTS

Concrete Wall

| Test No. | Date Cost | Location | Air % | Slump inches | 7 day psi | 28 day psi | Remarks |
|----------|-----------|----------------------------------|-------|--------------|-----------|------------|---------|
| 16 | 2 Nov 79 | Wall 88+70 to 89+20 | 5.3 | 3.94 | 1800 | 3614 | |
| 17 | 6 Nov 79 | Footing 86+40 to 86+90 | 5.0 | 1.97 | 2843 | 4500 | |
| 18 | 7 Nov 79 | Footing 87+90 to 88+40 | 4.3 | 1.97 | 2829 | 4371 | |
| 19 | 8 Nov 79 | Wall 105+75 to 105+25 | 5.2 | .76 | 2086 | 3414 | |
| 20 | 9 Nov 79 | Wall 100+02 to 100+45 | 5.4 | 2.56 | 2129 | 3671 | |
| 21 | 14 Nov 79 | Footing #11 | 4.8 | 2.36 | 3200 | 4200 | |
| 22 | 15 Nov 79 | Wall 89+20 to 88+57 | 5.6 | 3.94 | 2371 | 3600 | |
| 23 | 16 Nov 79 | Footing #23 | 4.8 | 2.76 | 3471 | 4957 | |
| 24 | 19 Nov 79 | Musqueam West Road Crossing | 4.6 | 2.17 | 4357 | 5743 | |
| 25 | 20 Nov 79 | Footing 78+40 to 77+90 | 6.1 | 2.36 | 2800 | 2737 | |
| 26 | 21 Nov 79 | Musqueam West Road Crossing Wall | 5.0 | 3.54 | 3386 | 5236 | |
| 27 | 22 Nov 79 | Wall 83+06 to 83+56 | 4.5 | 4.33 | 2343 | 3307 | |

CONCRETE TEST RESULTS

128 th Street Floodbox

| Test No. | Date Cast | Location | Air % | Slump Inches | Strength | | Remarks |
|-------------|--------------|--|----------|-----------------|----------|--------|---------|
| | | | | | 7 Day | 28 Day | |
| 28 | 23 Nov 79 | 128th St. Flood Box Seepage collar ftg. | 4.1 | 3.35 | 3857 | 4915 | |
| 35 | 5 Dec 79 | 128th St. Flood Box & Seepage collars | 3.8 | 3.35 | 3100 | 4615 | |
| 49 | 15 Jan 80 | Flood Box slab-north of 128th F.B. | 4.0 | 2.56 | 3186 | 4250 | |
| 52 | 22 Jan 80 | Inlet Box Walls, 128th F.B. | 5.0 | 4.33 | 3043 | 4136 | |
| 56 | 4 Feb 80 | Flood Box walls at outlet | 3.6 | 3.35 | 3100 | 4207 | |
| 79 | 13 Mar 80 | 128 F.B. Head Wall & Apron walls | 4.6 | 2.56 | 4043 | 5672 | |

CONCRETE TEST RESULTS

Concrete Wall

| Test No. | Date Cast | Location | Air % | Slump inches | 7 day psi | 28 day psi | Remarks |
|----------|-----------|--------------------------------------|-------|--------------|-----------|------------|---------|
| 29 | 26 Nov 79 | Wall 81+56 to 82+06 | 4.8 | 2.56 | 3514 | 4000 | |
| 30 | 27 Nov 79 | Wall 81+06 to 81+56 & 80+06 to 80+56 | 4.4 | 2.76 | 2629 | 3729 | |
| 31 | 28 Nov 79 | Wall 78+40 to 78+90 | 4.5 | 2.95 | 2629 | 4171 | |
| 32 | 29 Nov 79 | Wall 78+40 to 77+90 | 7.4 | 3.74 | 1229 | 2971 | |
| 33 | 30 Nov 79 | Wall 77+90 to 77+40 | 5.9 | 3.15 | 2371 | 4114 | |
| 34 | 4 Dec 79 | Wall 75+00 to 74+50 | 4.8 | 2.95 | 2414 | 4093 | |
| 36 | 6 Dec 79 | Wall 72+63 to 73+13 & 71+63 to 72+13 | 5.3 | 3.94 | 2071 | 3600 | |
| 38 | 7 Dec 79 | Footing Musqueam East Road Crossing | 6.0 | 3.54 | 1843 | 3686 | |
| 39 | 10 Dec 79 | Wall 98+00 to 98+50 | 5.0 | | 2400 | | |
| 40 | 11 Dec 79 | Wall 99+00 to 99+50 | 3.8 | 2.95 | 2943 | 4093 | |
| 41 | 12 Dec 79 | Wall 67+00 to 67+50 | 5.9 | 3.35 | 2443 | 3629 | |

CONCRETE TEST RESULTS

Dyke Wall

| Test No. | Date Cast | Location | Air % | Slump Inches | Strength 7 Day | Strength 28 Day | Remarks |
|----------|-----------|---|-------|--------------|-------------------|--------------------|---------|
| psi | | | | | | | |
| 42 | 13 Dec 79 | Walls Sta 65+00 to 66+00 | 6.0 | 3.35 | 2529 | 3686 | |
| 43 | 14 Dec 79 | Sta 65+293 to 65+793 66+293 to 66+793 Walls | 6.4 | 5.51 | 2057 | 3415 | |
| 44 | 19 Dec 79 | Footing Sta 54+00 to 54+50 | 6.0 | 6.30 | 2200 | 3057 | |
| 45 | 20 Dec 79 | Footing Sta 50+50 to 51+00 | 5.4 | 1.97 | 2600 | 3715 | |
| 46 | 21 Dec 79 | Footing Sta 52+50 to 53+00 | 5.6 | 2.76 | 2329 | 3643 | |
| 47 | 3 Jan 80 | 2nd slab East of M & T Crossing | 6.4 | 2.36 | 2000 | 3343 | |
| 48 | 4 Jan 80 | Wall Sta 60+50 to 61+00 | 4.2 | 2.76 | 2443 | 3657 | |
| 49 | 15 Jan 80 | Flood Box Slab North of 128th | 4.0 | 2.56 | 3186 | 4257 | |
| 50 | 16 Jan 80 | Domtar E Crossing Slab | - | 1.77 | 2972 | 4000 | |
| 51 | 18 Jan 80 | Sta 60+00 to 60+50 Wall | 4.9 | 2.95 | 2729 | 3957 | |

CONCRETE TEST RESULTS

Dyke Wall

| Test No. | Date Cast | Location | Air % | Slump Inches | Strength 7 Day | Strength 28 Day | Remarks |
|----------|-----------|--------------------------------------|-------|--------------|-------------------|--------------------|---------|
| 53 | 23 Jan 80 | Sta 59+50 to 60+00 Wall | ~ 5.0 | 2.76 | 2672 | 3786 | |
| 54 | 24 Jan 80 | Sta 35+05 to 35+55 Footing | 4.3 | 2.36 | 2843 | 3907 | |
| 55 | 25 Jan 80 | Wall @ Sta 58+00 to 58+50 | 5.3 | 2.76 | 2629 | 3900 | |
| 57 | 6 Feb 80 | Slab @ Sta 41+005 to 41+55 | 5.4 | 2.76 | 2843 | 4072 | |
| 58 | 7 Feb 80 | Wall @ Sta 53+44 to 53+04 | 4.9 | 2.76 | 2886 | 4029 | |
| 59 | 8 Feb 80 | Slab @ West of Domtar X-ing | 5.2 | 3.15 | 3315 | 5129 | |
| 60 | 11 Feb 80 | Wall @ East of M & T X-ing | 5.5 | 3.75 | 2429 | 3686 | |
| 61 | 12 Feb 80 | Slab @ Sta 45+06 to 44+50 | 4.7 | 2.56 | 2429 | 3786 | |
| 62 | 12 Feb 80 | Wall @ Domtar E X-ing | 4.5 | 2.95 | 3586 | 5215 | |
| 63 | 13 Feb 80 | Wall @ ½ Musqueam W X-ing | 4.3 | 2.36 | 4657 | 5715 | |
| 64 | 14 Feb 80 | Wall @ 3rd Section West of 130th St. | 3.15 | 5.7 | 2714 | 4143 | |

CONCRETE TEST RESULTS

Dyke Wall

| Test No. | Date Cast | Location | Air % | Slump Inches | Strength 7 Day | Strength 28 Day | Remarks |
|----------|-----------|--|-------|--------------|----------------|-----------------|---------|
| psi | | | | | | | |
| 65 | 15 Feb 80 | Wall @ 1st Section, West of 130th St. | 3.35 | 5.4 | 1957 | 3672 | |
| 66 | 19 Feb 80 | Wall @ Sta 37+51 to 38+01 | 3.54 | 4.6 | 2686 | 4072 | |
| 67 | 20 Feb 80 | Wall @ Sta 38+01 to 38+51 | 3.94 | 5.2 | 2472 | 3657 | |
| 68 | 21 Feb 80 | Slab @ Sta 45+01 West | 3.54 | 4.0 | 2843 | 4015 | |
| 69 | 22 Feb 80 | Wall @ Sta 40+50 to 41+00 | 3.35 | 5.6 | 2672 | 4372 | |
| 70 | 26 Feb 80 | Footing @ Wesco Area by Spur Line | 5.12 | 3.1 | 3786 | 4843 | |
| 71 | 27 Feb 80 | - | - | - | 4186 | 4843 | |
| 72 | 28 Feb 80 | Footing @ Sta 45+39 to 45+83 | 4.2 | 2.56 | 3200 | 4357 | |
| 73 | 29 Feb 80 | Footing @ Sta 43+43 to 42+99 | 4.7 | 1.77 | 2915 | 3986 | |
| 74 | 4 Mar 80 | Wall @ Sta 43+63 to 44+00 | 4.9 | 2.76 | 2757 | 3686 | |
| 75 | 5 Mar 80 | Wall @ Sta 44+00 to 44+50 | 5.8 | 2.95 | 2357 | 3600 | |
| 76 | 7 Mar 80 | Wall @ Sta 43+41 to 43+91 5'-0" High Wall | 4.7 | 3.15 | 2600 | 4243 | |

CONCRETE TEST RESULTS

Dyke Wall

| Test No. | Date Cast | Location | Air % | Slump Inches | Strength 7 Day | Strength 28 Day | Remarks |
|----------|-----------|--|-------|--------------|-------------------|--------------------|---------|
| psi | | | | | | | |
| 77 | 11 Mar 80 | Wall @ Sta 45+90 to 46+30 5'-9" High Wall | 5.3 | 3.74 | 2600 | 3757 | |
| 78 | 12 Mar 80 | Wall @ Sta 43+01 to 43+41 7'-0" High Wall | 4.5 | 2.17 | 2786 | 4043 | |
| 80 | 14 Mar 80 | Wall @ Sta 45+50 to 45+90 5'-9" High Wall | 5.7 | 3.15 | 2271 | 3543 | |
| 81 | 18 Mar 80 | Wall @ Sta 42+22 to 42+62 | 4.7 | 2.95 | 2114 | 3586 | |
| 82 | 19 Mar 80 | Footing @ Sta 21+50 to 22+00 22+50 to 23+00 | 3.7 | 2.17 | 2286 | 3900 | |
| 83 | 20 Mar 80 | Wall @ Sta 42+62 to 42+02 | 5.0 | 2.76 | 2957 | 3715 | |
| 84 | 24 Mar 80 | Footing @ Sta 21+00 to 21+50 Sta 22+00 to 22+50 | 4.6 | 1.97 | 3272 | 4500 | |
| 85 | 25 Mar 80 | Wall @ Sta 48+41 to 48+69 | 5.6 | 2.56 | 2543 | 4036 | |
| 86 | 25 Mar 80 | Footing @ Sta 19+82 to 20+00 & Sta 10+50 to 21+00 | 4.5 | 2.17 | 2572 | 4329 | |
| 87 | 26 Mar 80 | Footing @ Sta 20+00 to 20+50 | 4.8 | 1.77 | 3000 | 4457 | |
| 88 | 27 Mar 80 | Footing @ 1/2 130 St X-ing | 5.0 | 2.36 | 2886 | 4086 | |

CONCRETE TEST RESULTS

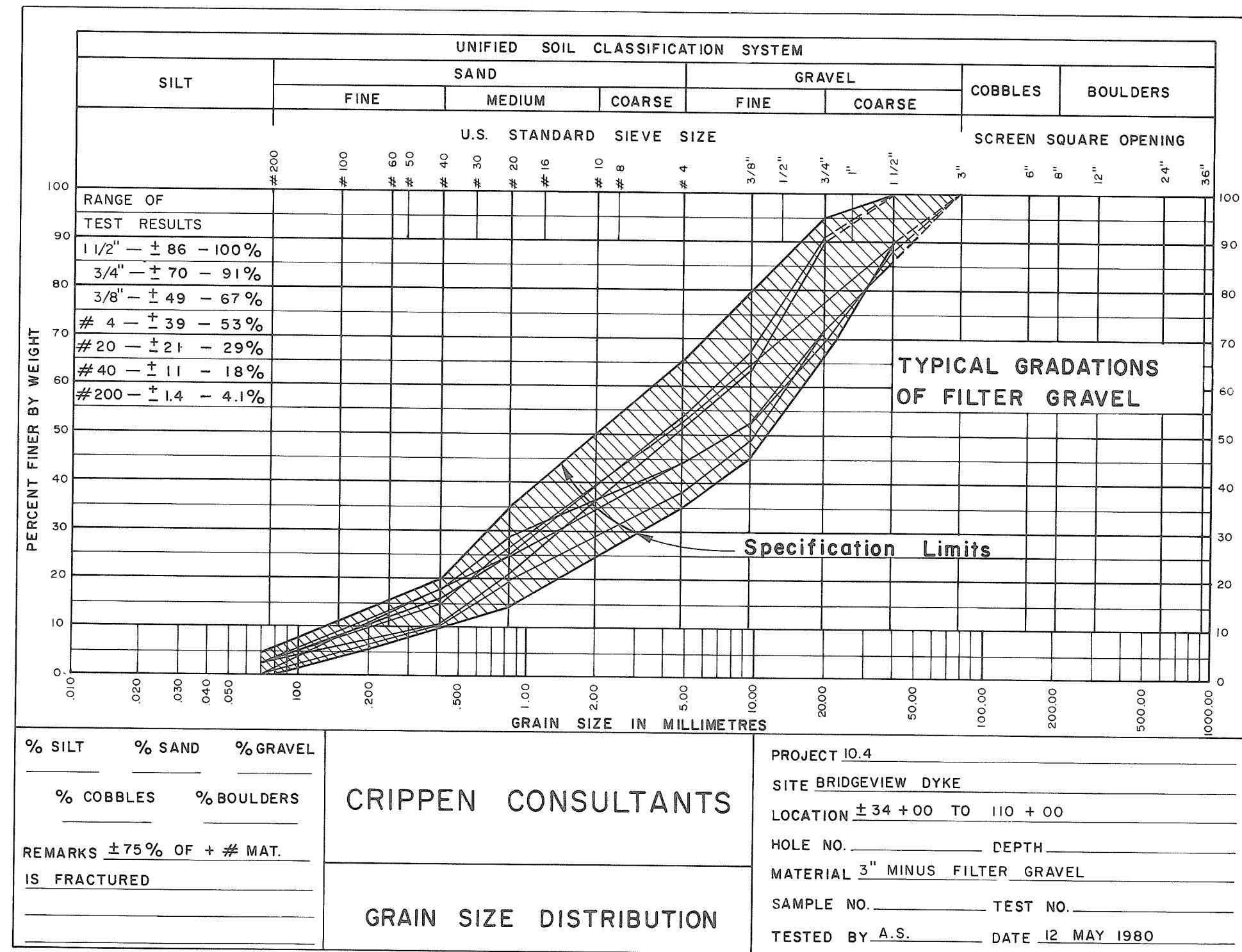
Dyke Wall

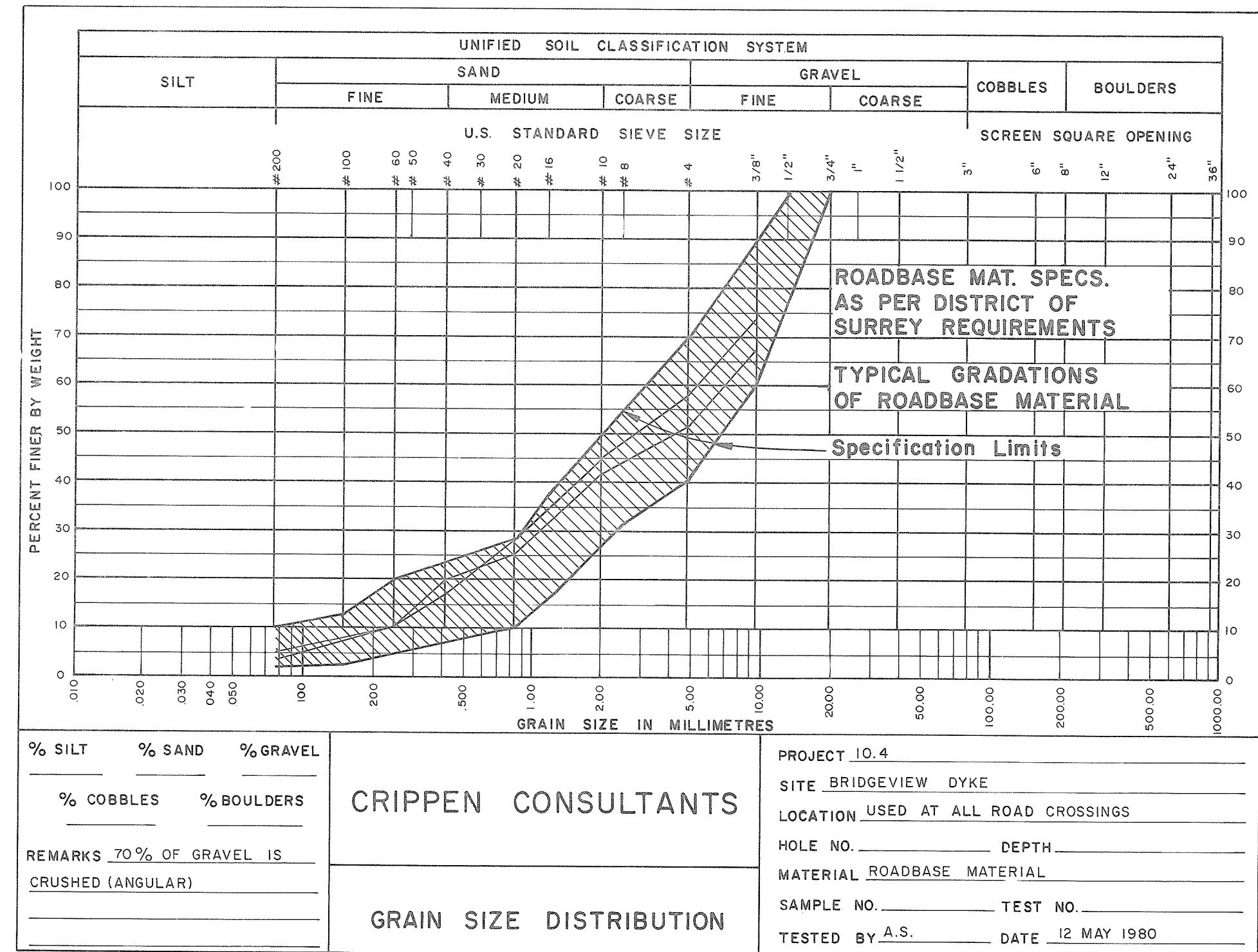
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|----------|-----------|--|-------|--------------|-------------------|--------------------|---------|
| psi | | | | | | | |
| 89 | 28 Mar 80 | Footing @ ½ Domtar W X-ing | 3.8 | 2.36 | 4043 | 4857 | |
| 90 | 28 Mar 80 | Wall @ Sta 22+50 to 23+00 | 4.5 | 3.15 | 3257 | 4300 | |
| 91 | 28 Mar 80 | Wall @ ½ 130 St. X-ing | 4.5 | 2.36 | 3386 | 4186 | |
| 92 | 1 Apr 80 | Wall @ ½ Domtar W X-ing | 5.0 | 3.54 | 2643 | 3906 | |
| 93 | 1 Apr 80 | Wall @ Sta 22+00 to 22+50 | 4.7 | 2.95 | 2572 | 3928 | |
| 94 | 3 Apr 80 | Wall @ Sta 21+00 to 21+50 | 6.2 | 3.74 | 1829 | 3486 | |
| 95 | 8 Apr 80 | Wall @ Sta 20+50 to 21+00 | 5.1 | 2.95 | 2157 | 3715 | |
| 96 | 9 Apr 80 | Wall @ Sta 20+00 to 20+50 | 4.8 | 3.74 | 2143 | 3557 | |
| 97 | 10 Apr 80 | Footing @ Sta 55+14 to 55+64 | 5.1 | 4.53 | 2086 | 3629 | |
| 98 | 10 Apr 80 | Wall @ Sta 19+80 to 20+00 | 4.6 | 2.56 | 2472 | 3886 | |
| 99 | 11 Apr 80 | Footing @ 27' W.O. East of 126A St. | 4.9 | 3.35 | 1986 | 3600 | |
| 100 | 11 Apr 80 | Footing @ ½ 130 St. X-ing | 3.4 | 2.56 | 2929 | 5100 | |
| 101 | 15 Apr 80 | Wall @ ½ 130 St. X-ing | 3.4 | 2.56 | 3657 | 4657 | |
| 103 | 18 Apr 80 | Footing @ Sta 48+41 & 48+69 | 5.3 | 2.76 | 2400 | 3609 | |

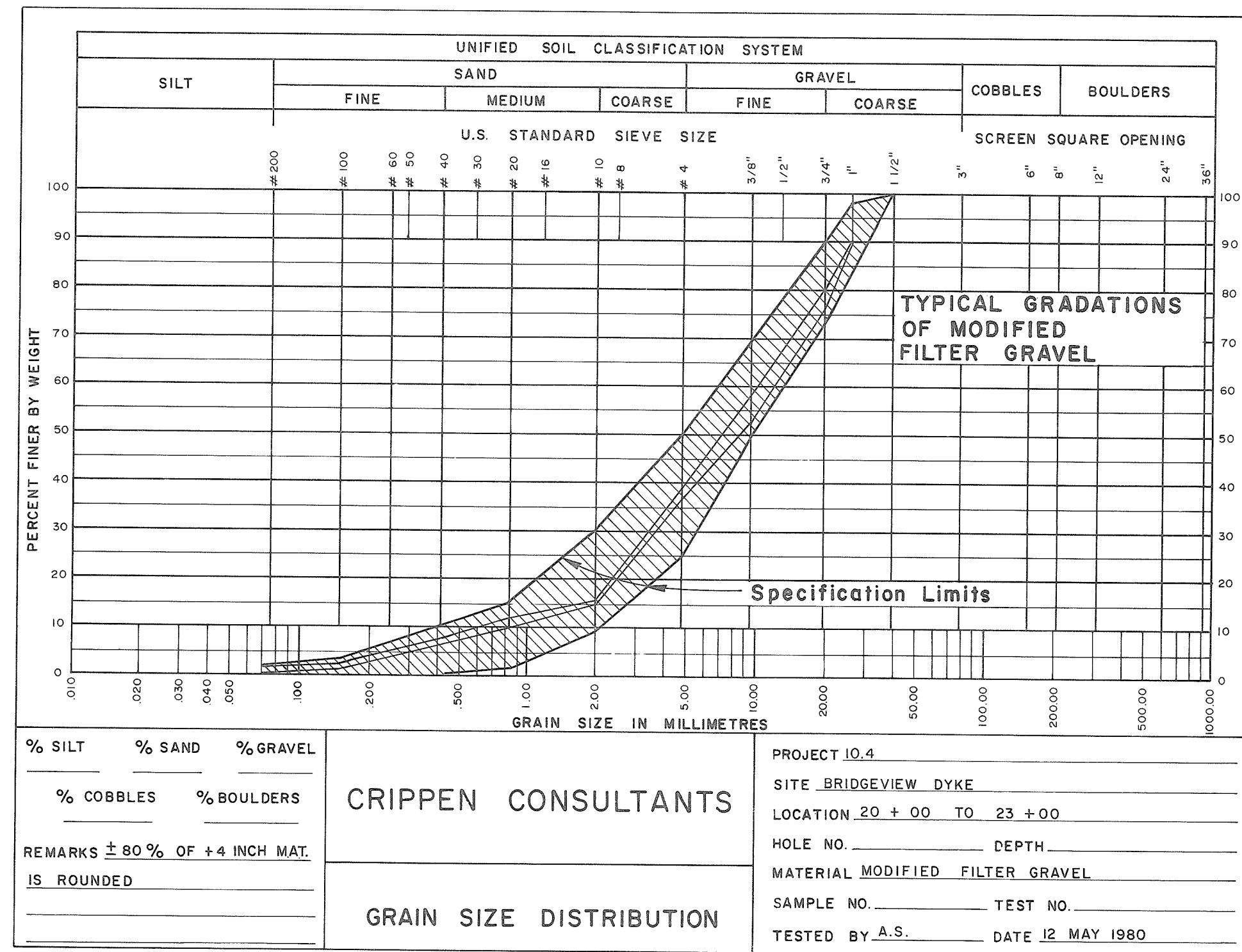
TYPICAL MATERIAL GRADATION

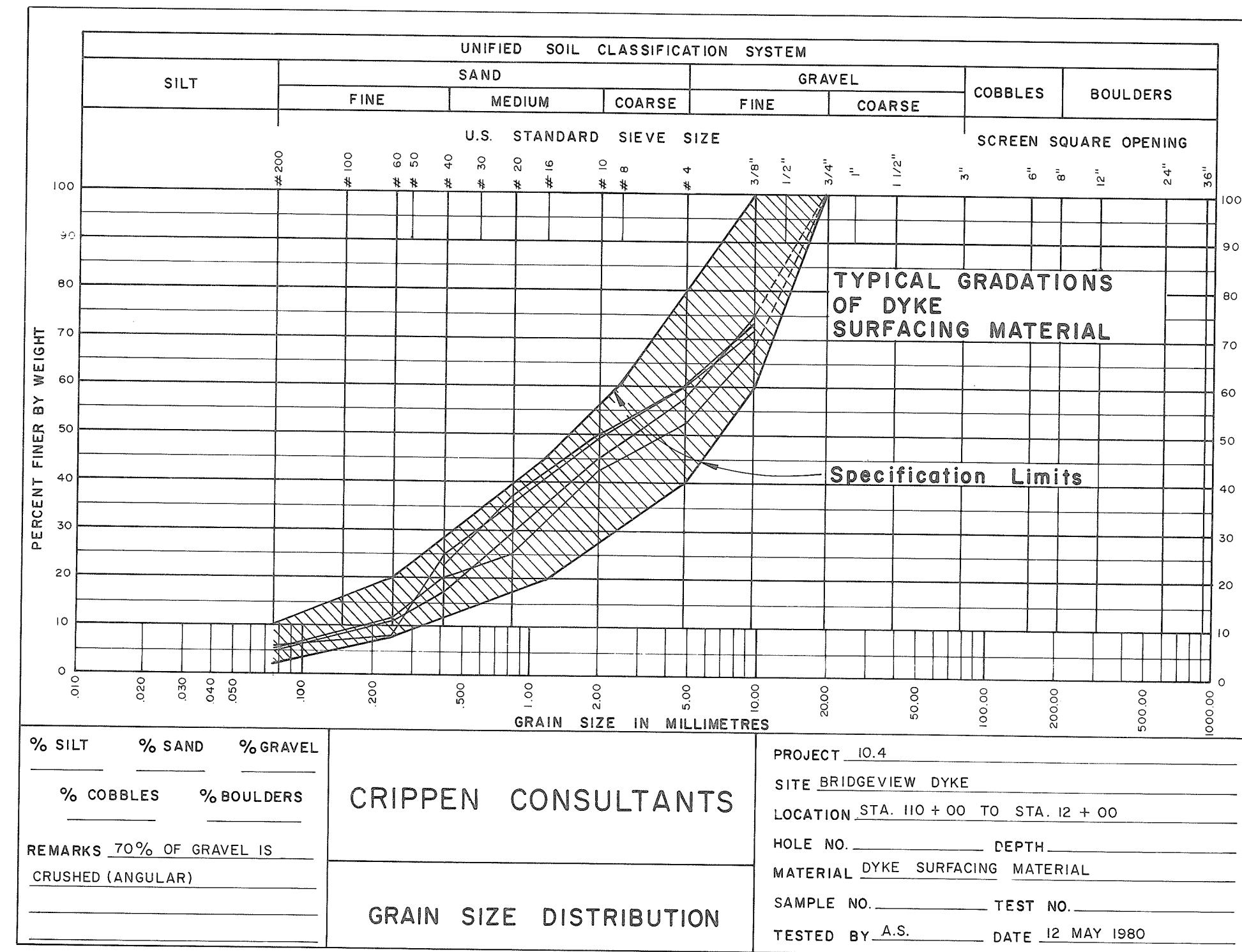
APPENDIX 2

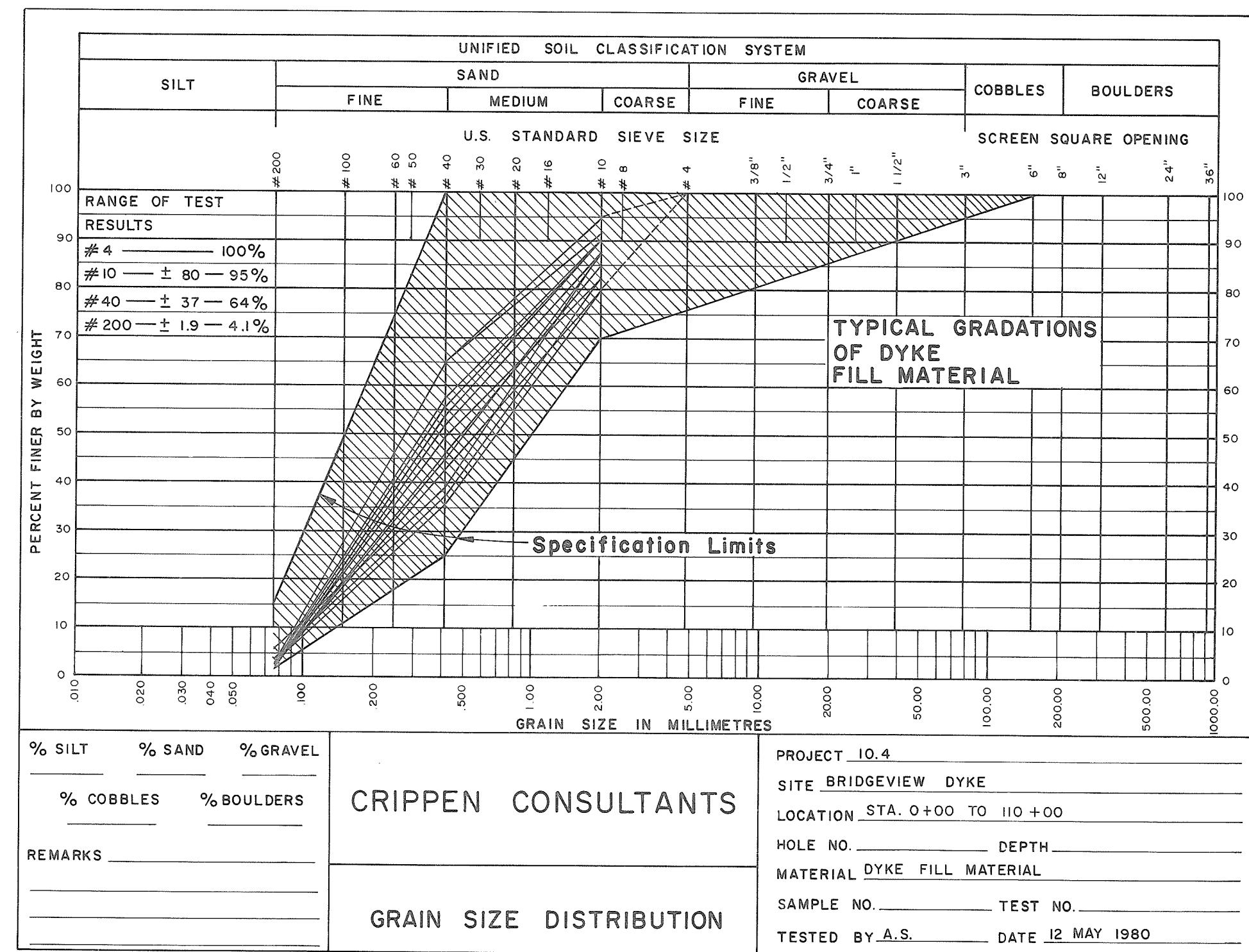
DYKE MATERIALS







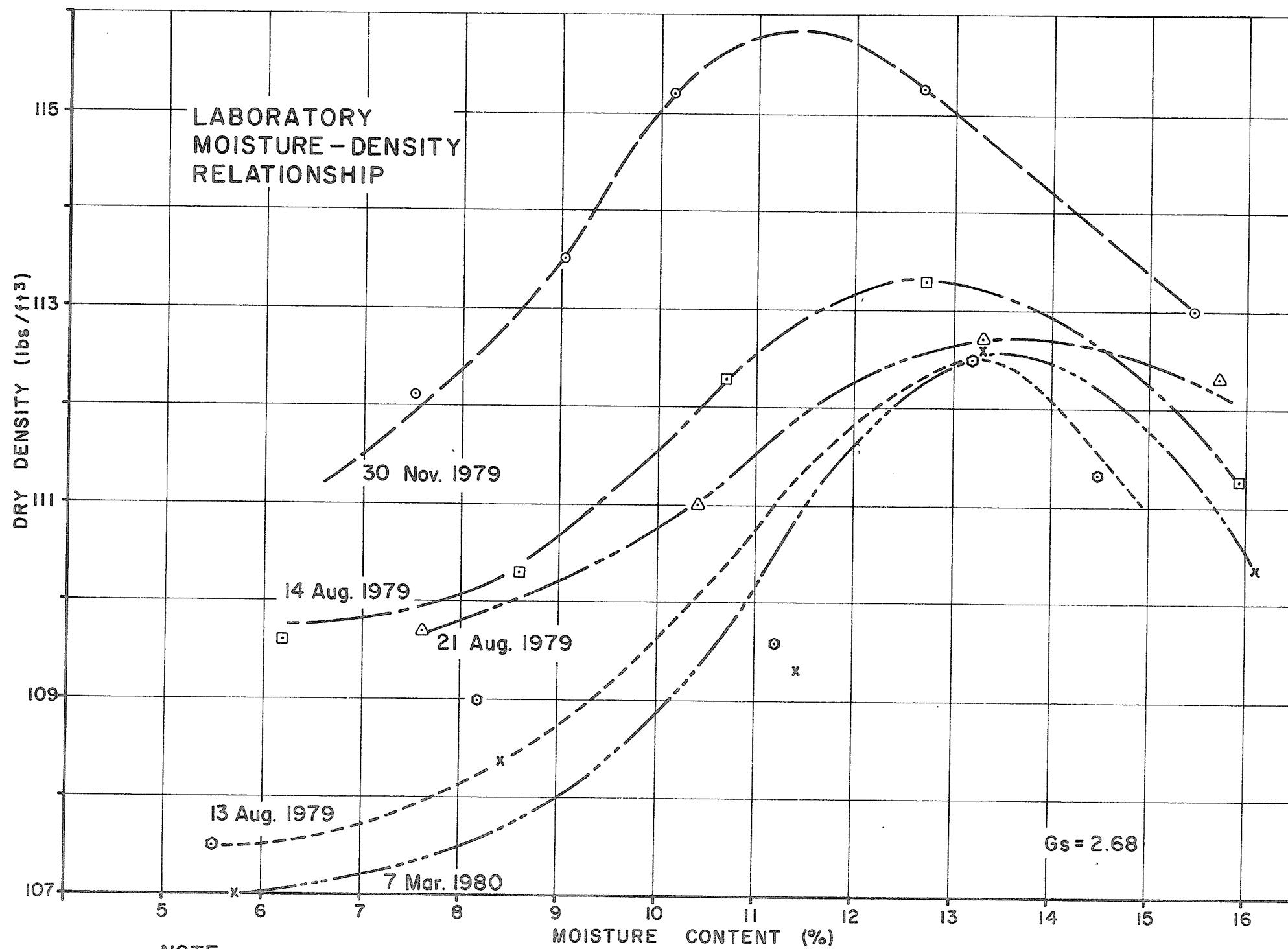




APPENDIX 3

INSTRUMENTATION

DYKE FILL MATERIAL

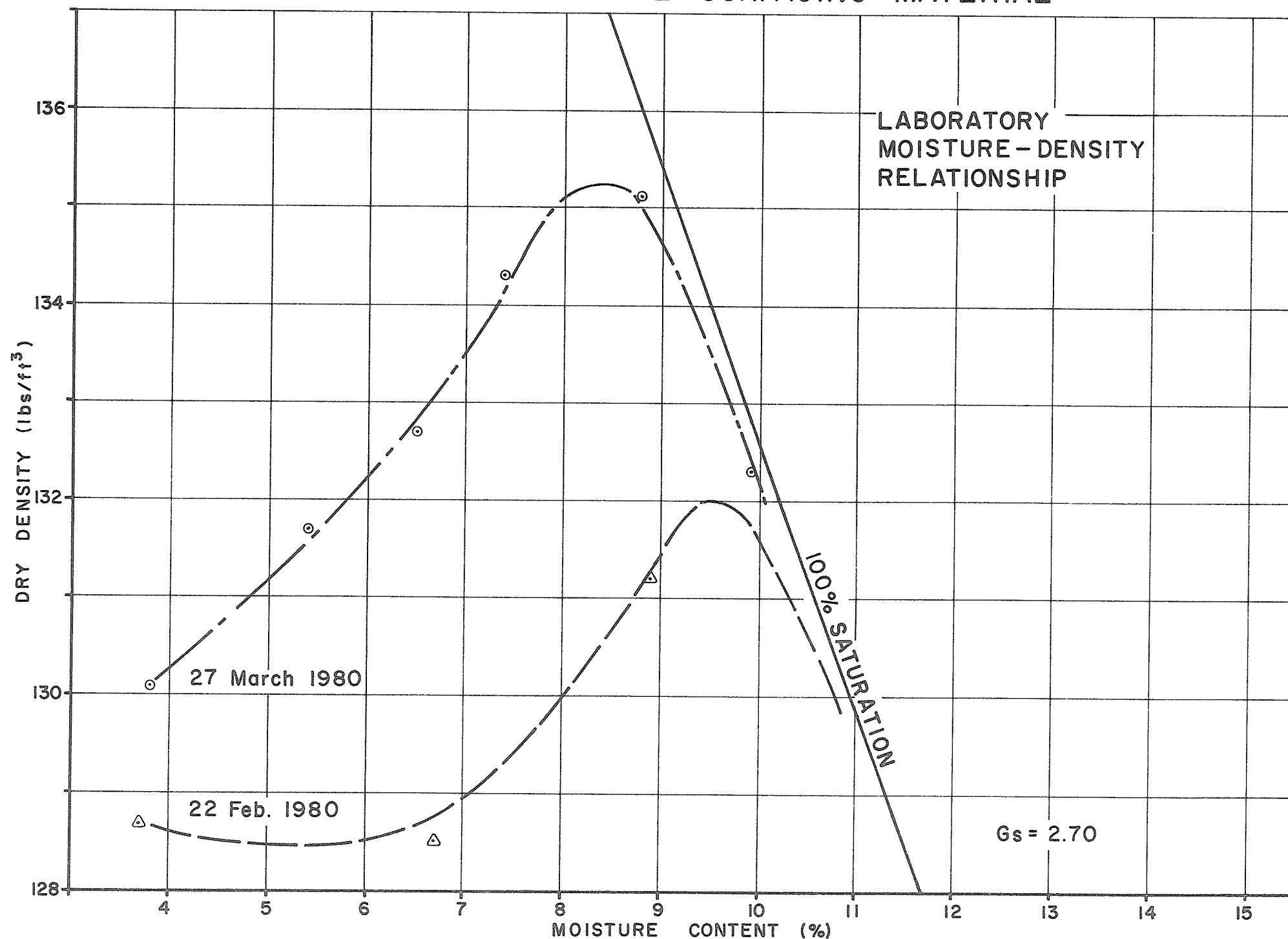


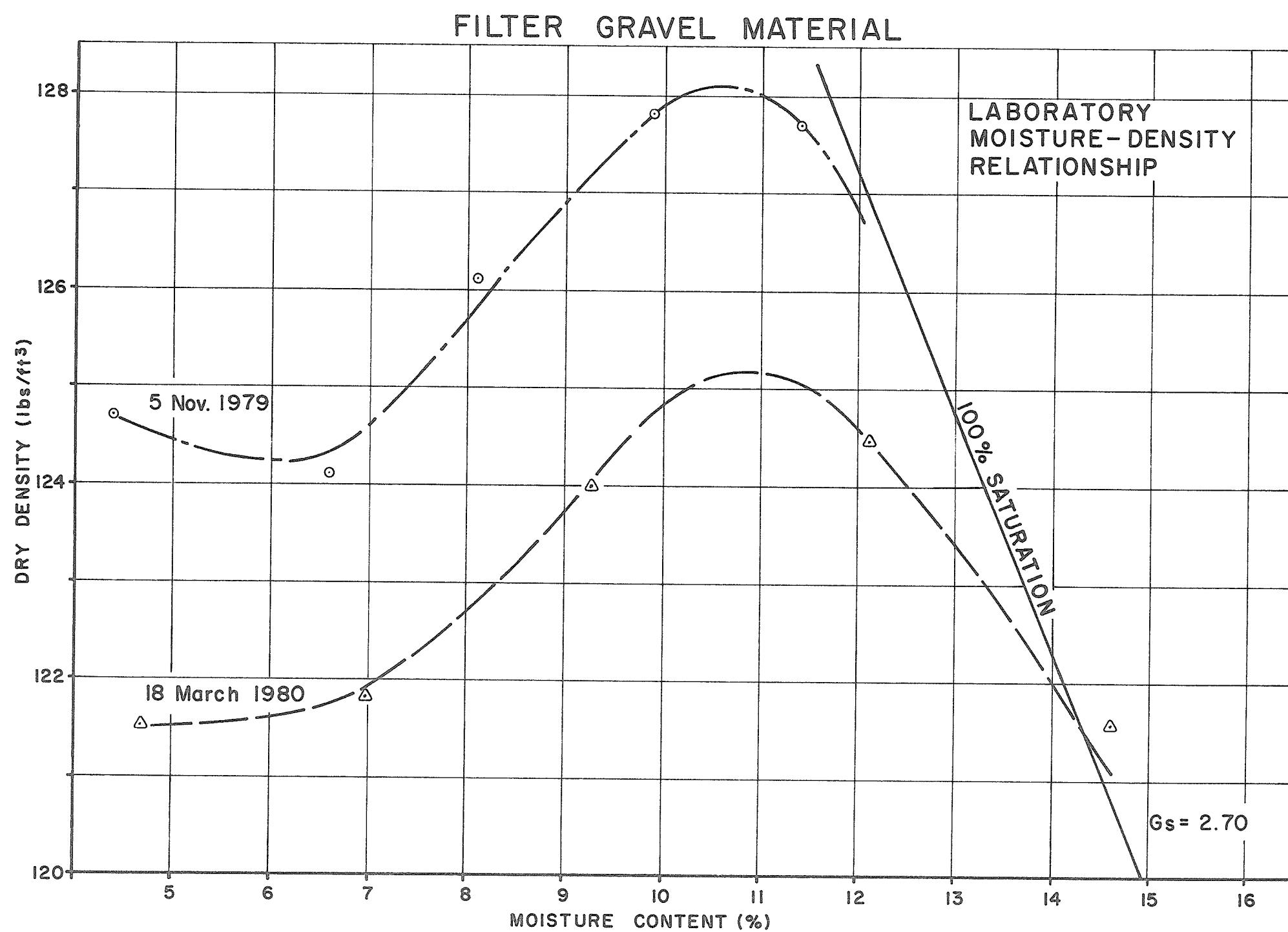
NOTE

LABORATORY TEST RESULTS UNRELIABLE AT HIGHER MOISTURE CONTENTS DUE TO LEAKAGE
OF WATER FROM SAMPLE

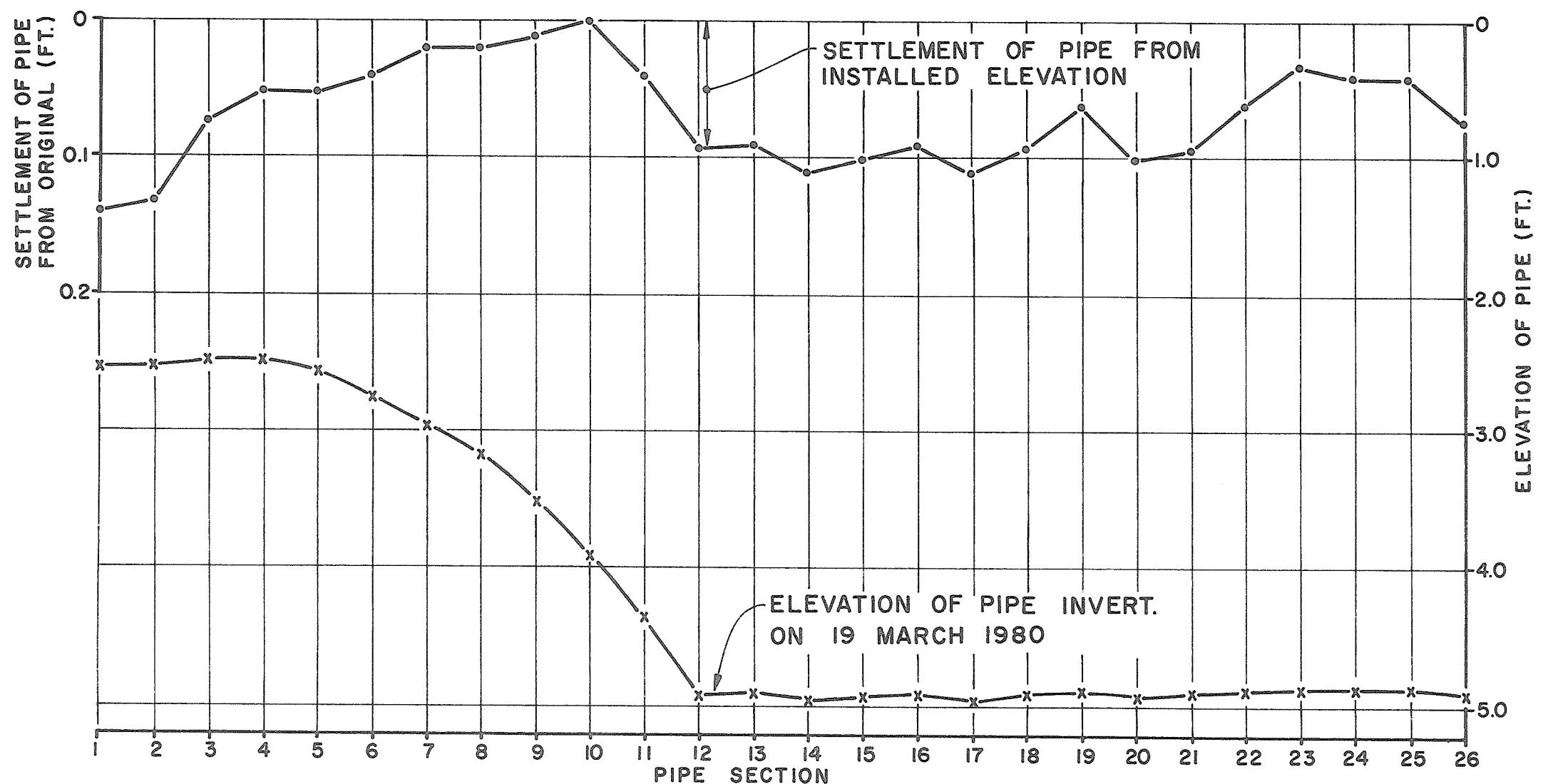
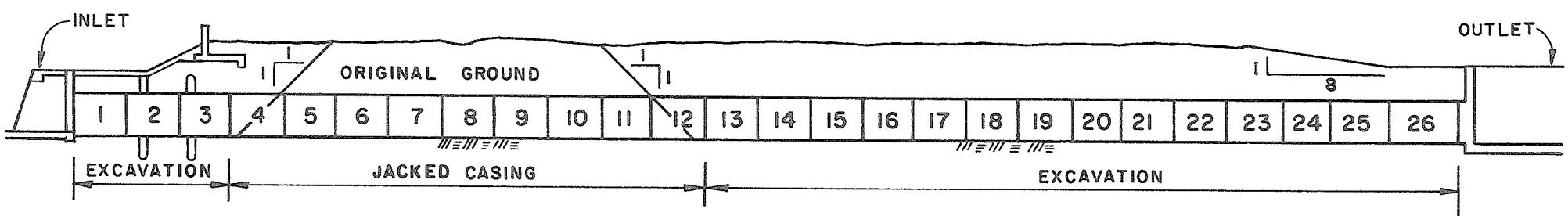
MAXIMUM DRY DENSITY ESTIMATED FROM LOWER PORTION OF CURVES AND FROM FIELD COMPARISON TESTS OF COMPACTION VS NUMBER OF ROLLER PASSES.

BASECOURSE / DYKE SURFACING MATERIAL

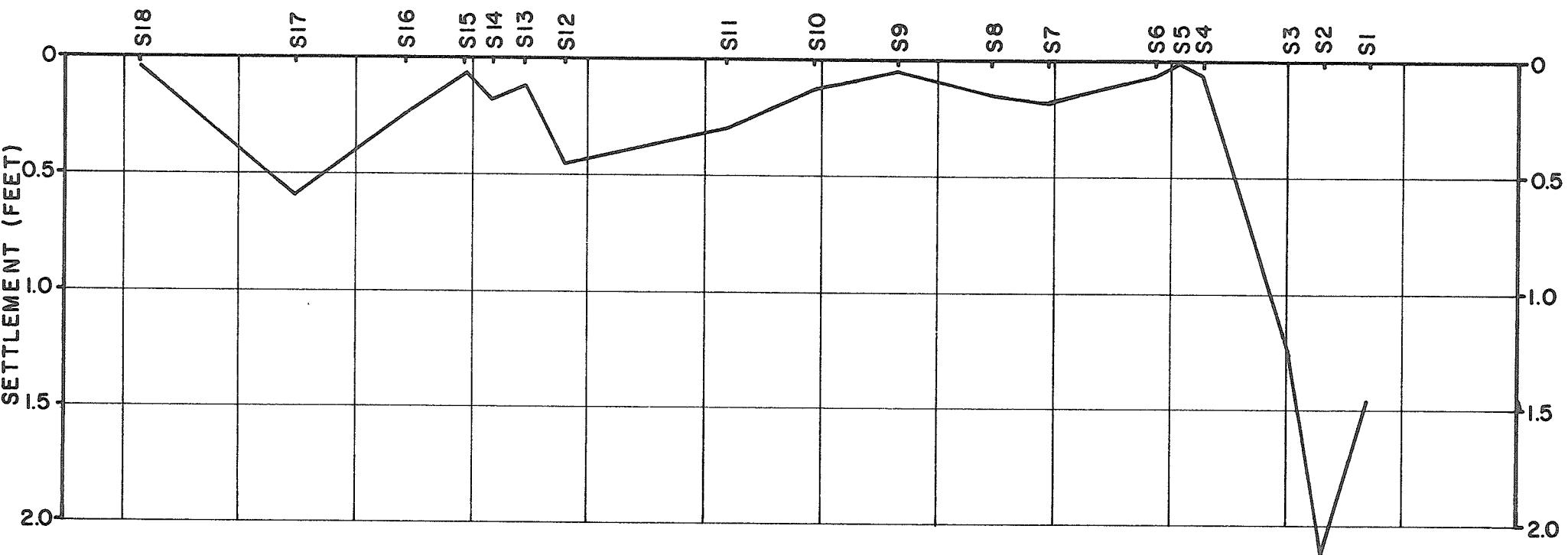




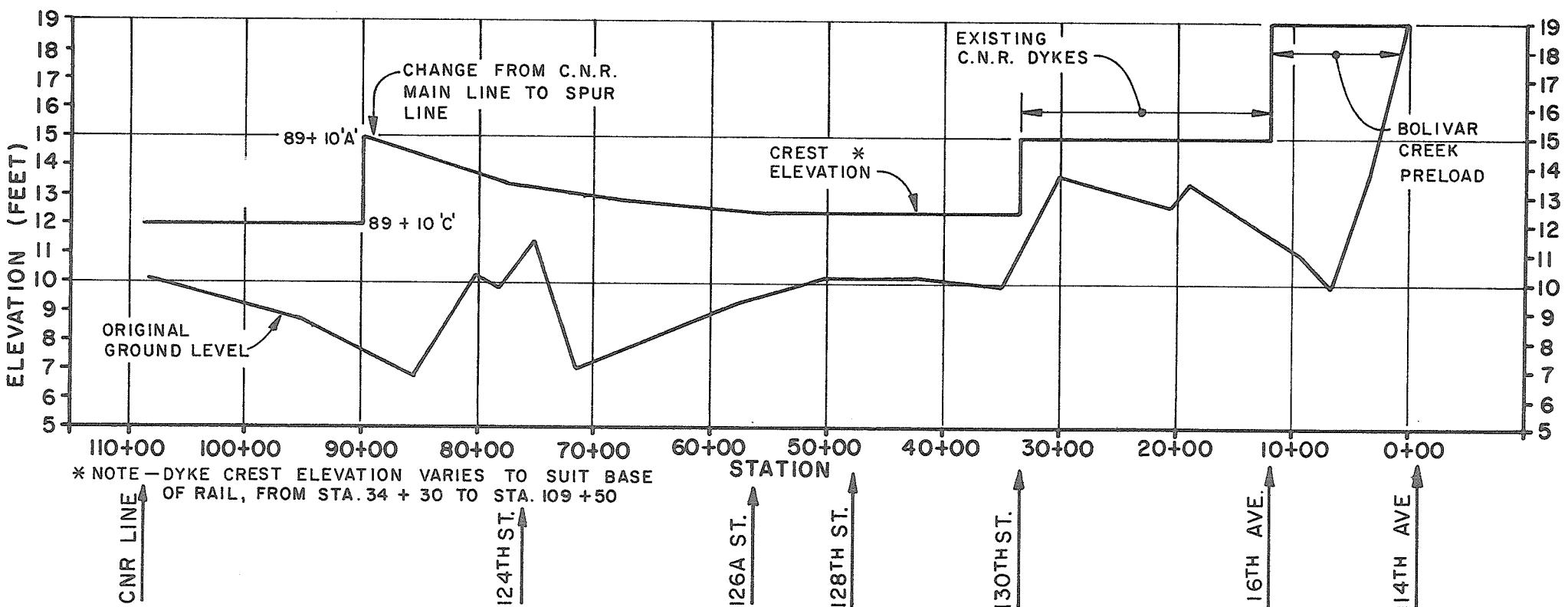
SETTLEMENT PROFILE OF 72" Ø PIPE AT 128TH ST. OUTLET WORKS

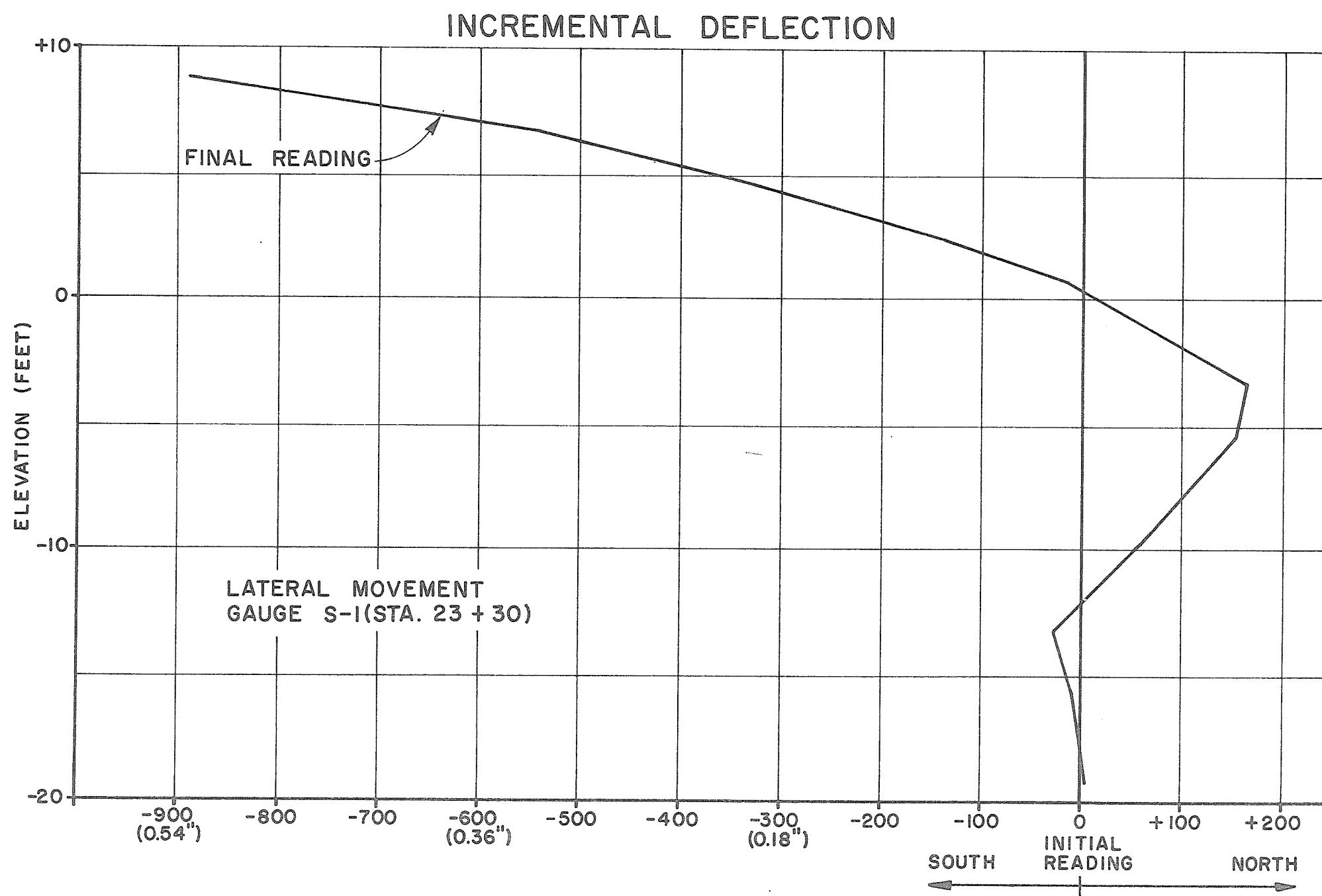


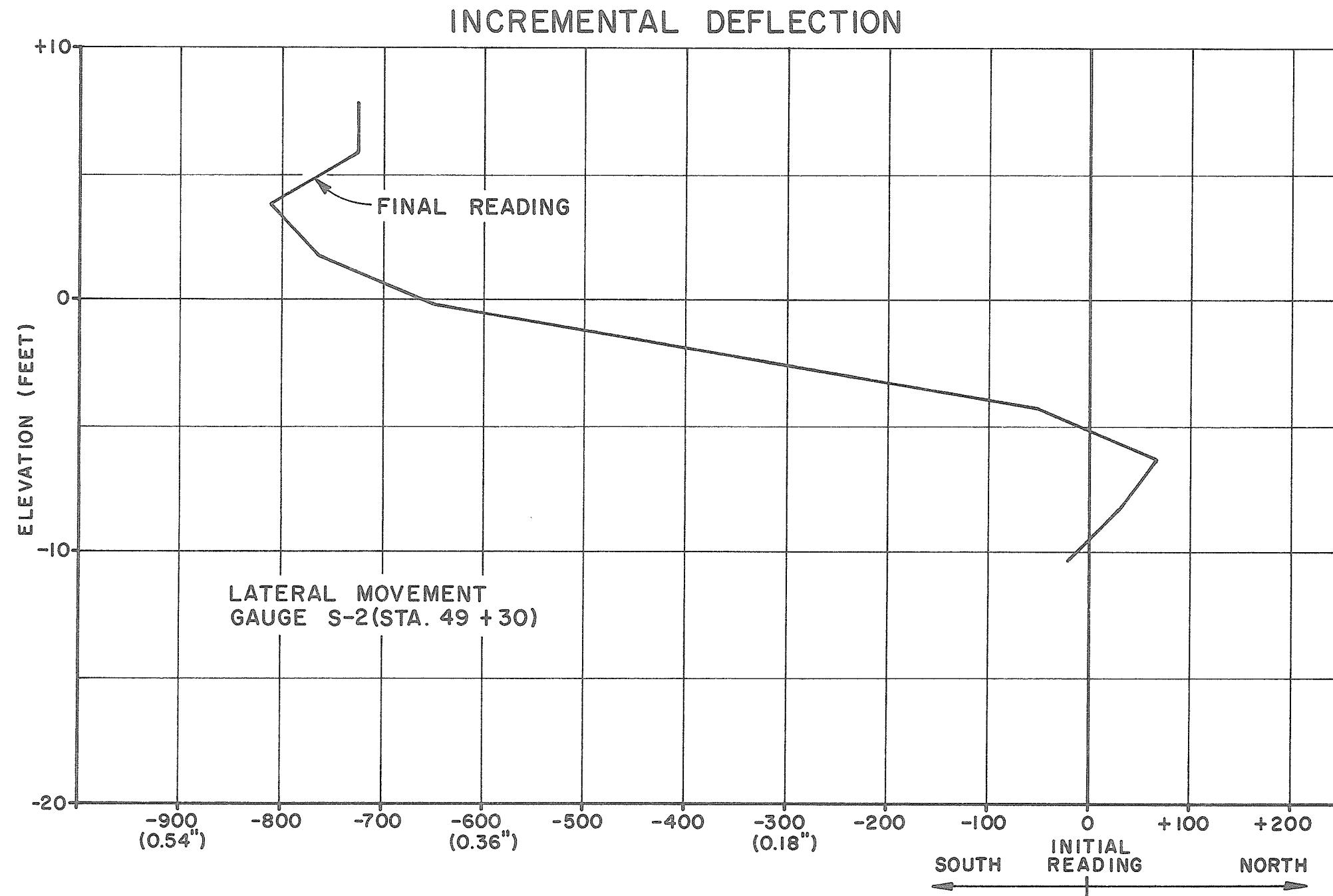
DYKE CENTRE LINE PROFILES
SETTLEMENT PROFILE

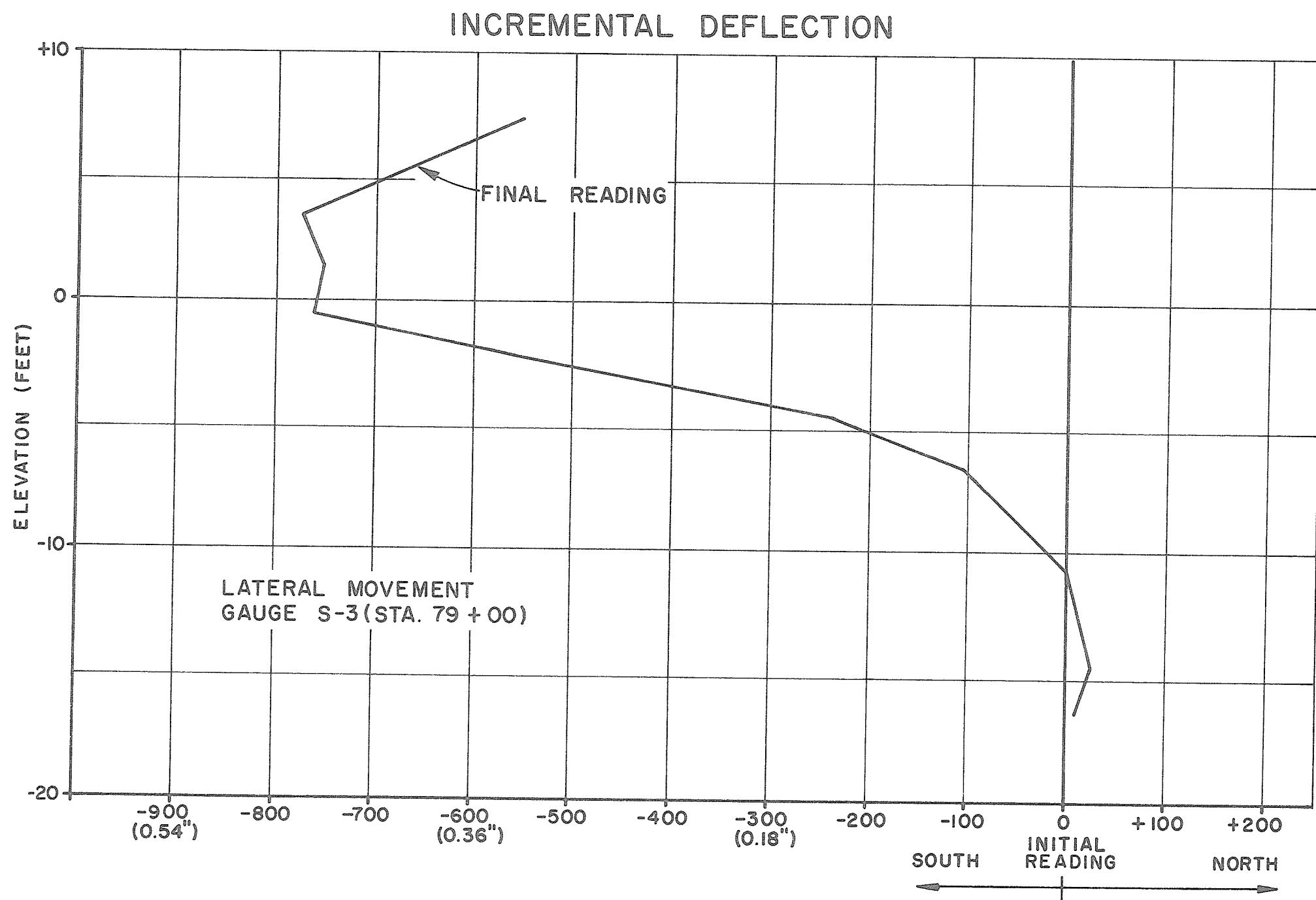


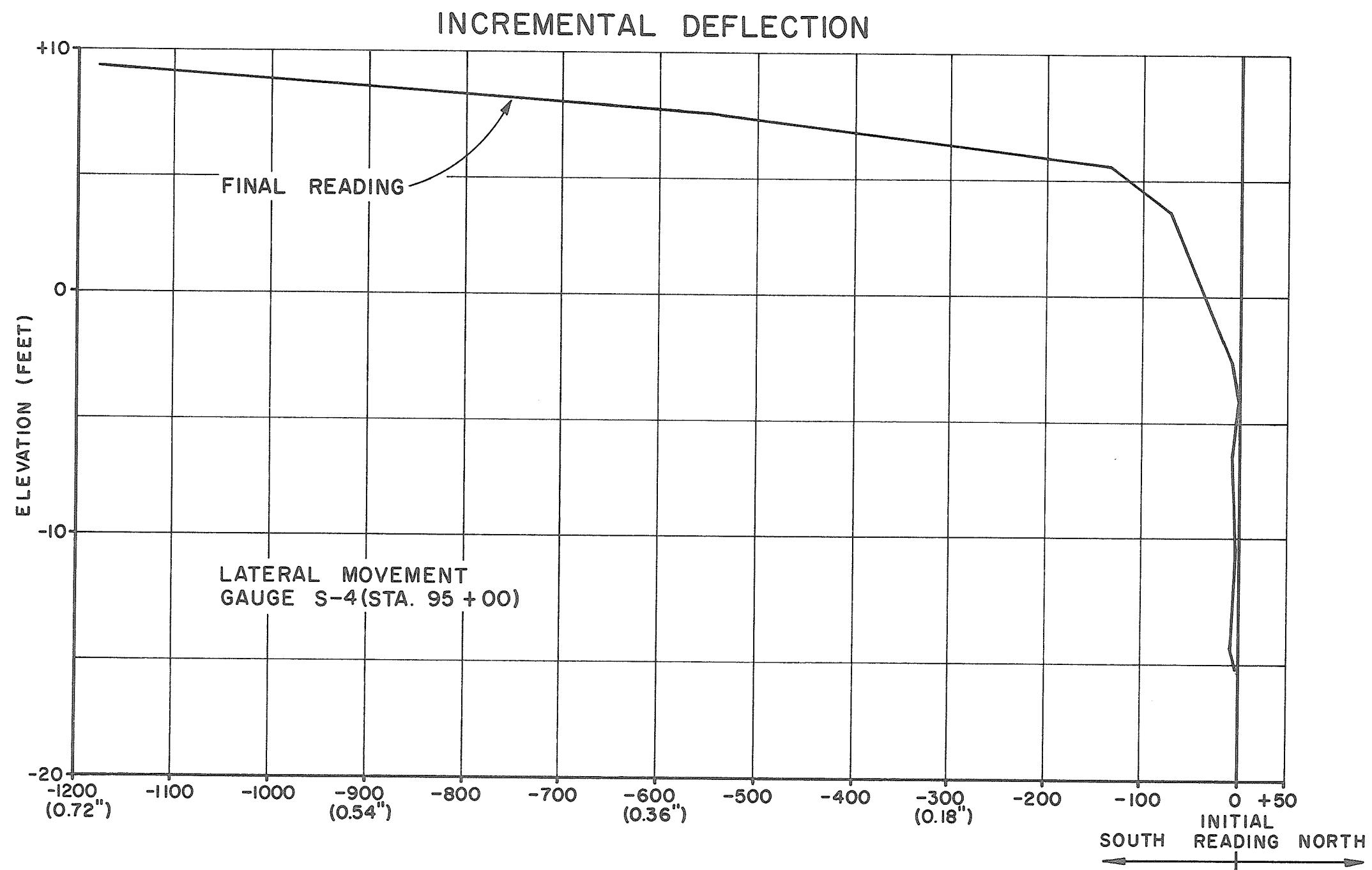
LONGITUDINAL PROFILE











FIELD TRIALS - COMPACTION vs NO. OF ROLLER PASSES

